

1873



# EUGENE DIETZGEN CO.

DRAWING MATERIALS, MATHEMATICAL and  
SURVEYING INSTRUMENTS

Chicago New York San Francisco New Orleans Pittsburg Toronto

**MICROFILMED**  
Distances from Center of Roadway for Cross-Sectioning  
Roadway 16 feet wide. Side Slopes 1 on 1.  
For Single Track Embankment.

H	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	H
0	8.0	8.1	8.2	8.3	8.4	8.5	8.6	8.7	8.8	8.9	0
1	9.0	9.1	9.2	9.3	9.4	9.5	9.6	9.7	9.8	9.9	1
2	10.0	10.1	10.2	10.3	10.4	10.5	10.6	10.7	10.8	10.9	2
3	11.0	11.1	11.2	11.3	11.4	11.5	11.6	11.7	11.8	11.9	3
4	12.0	12.1	12.2	12.3	12.4	12.5	12.6	12.7	12.8	12.9	4
5	13.0	13.1	13.2	13.3	13.4	13.5	13.6	13.7	13.8	13.9	5
6	14.0	14.1	14.2	14.3	14.4	14.5	14.6	14.7	14.8	14.9	6
7	15.0	15.1	15.2	15.3	15.4	15.5	15.6	15.7	15.8	15.9	7
8	16.0	16.1	16.2	16.3	16.4	16.5	16.6	16.7	16.8	16.9	8
9	17.0	17.1	17.2	17.3	17.4	17.5	17.6	17.7	17.8	17.9	9
10	18.0	18.1	18.2	18.3	18.4	18.5	18.6	18.7	18.8	18.9	10
11	19.0	19.1	19.2	19.3	19.4	19.5	19.6	19.7	19.8	19.9	11
12	20.0	20.1	20.2	20.3	20.4	20.5	20.6	20.7	20.8	20.9	12
13	21.0	21.1	21.2	21.3	21.4	21.5	21.6	21.7	21.8	21.9	13
14	22.0	22.1	22.2	22.3	22.4	22.5	22.6	22.7	22.8	22.9	14
15	23.0	23.1	23.2	23.3	23.4	23.5	23.6	23.7	23.8	23.9	15
16	24.0	24.1	24.2	24.3	24.4	24.5	24.6	24.7	24.8	24.9	16
17	25.0	25.1	25.2	25.3	25.4	25.5	25.6	25.7	25.8	25.9	17
18	26.0	26.1	26.2	26.3	26.4	26.5	26.6	26.7	26.8	26.9	18
19	27.0	27.1	27.2	27.3	27.4	27.5	27.6	27.7	27.8	27.9	19
20	28.0	28.1	28.2	28.3	28.4	28.5	28.6	28.7	28.8	28.9	20
21	29.0	29.1	29.2	29.3	29.4	29.5	29.6	29.7	29.8	29.9	21
22	30.0	30.1	30.2	30.3	30.4	30.5	30.6	30.7	30.8	30.9	22
23	31.0	31.1	31.2	31.3	31.4	31.5	31.6	31.7	31.8	31.9	23
24	32.0	32.1	32.2	32.3	32.4	32.5	32.6	32.7	32.8	32.9	24
25	33.0	33.1	33.2	33.3	33.4	33.5	33.6	33.7	33.8	33.9	25
26	34.0	34.1	34.2	34.3	34.4	34.5	34.6	34.7	34.8	34.9	26
27	35.0	35.1	35.2	35.3	35.4	35.5	35.6	35.7	35.8	35.9	27
28	36.0	36.1	36.2	36.3	36.4	36.5	36.6	36.7	36.8	36.9	28
29	37.0	37.1	37.2	37.3	37.4	37.5	37.6	37.7	37.8	37.9	29
30	38.0	38.1	38.2	38.3	38.4	38.5	38.6	38.7	38.8	38.9	30
31	39.0	39.1	39.2	39.3	39.4	39.5	39.6	39.7	39.8	39.9	31
32	40.0	40.1	40.2	40.3	40.4	40.5	40.6	40.7	40.8	40.9	32
33	41.0	41.1	41.2	41.3	41.4	41.5	41.6	41.7	41.8	41.9	33
34	42.0	42.1	42.2	42.3	42.4	42.5	42.6	42.7	42.8	42.9	34
35	43.0	43.1	43.2	43.3	43.4	43.5	43.6	43.7	43.8	43.9	35
36	44.0	44.1	44.2	44.3	44.4	44.5	44.6	44.7	44.8	44.9	36
37	45.0	45.1	45.2	45.3	45.4	45.5	45.6	45.7	45.8	45.9	37
38	46.0	46.1	46.2	46.3	46.4	46.5	46.6	46.7	46.8	46.9	38
39	47.0	47.1	47.2	47.3	47.4	47.5	47.6	47.7	47.8	47.9	39
40	48.0	48.1	48.2	48.3	48.4	48.5	48.6	48.7	48.8	48.9	40

Example—If point is 22.6 ft. above grade, how far should it be from center line to be a slope stake point? Ans. from Table 30.6. For same slopes but other widths of roadbed, correct above figures by one-half difference in width of roadbed; thus in example above, for 20 ft. roadbed distance will be  $30.6 + (20 - 16) \div 2$  or 2 ft. added to  $30.6 = 32.6$ . For slopes of 1 on 1½ see inside of back cover.  
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INDEXED  
to page 175

0008/62

This Field Book is manufactured of a High Grade 50% Rag Paper having a WATER RESISTING SURFACE, and is sewed with Bing Special Enamel Waterproof thread.

Made in U. S. A.



2- Alvarado Sewer. Above Adobe Falls  
 25 change v " 402+00 to 407+77<sup>Ac</sup>  
 27 v " 425+45<sup>00</sup> to 468+67<sup>49</sup>  
 29 v " " 376+17<sup>08</sup> to 381+82<sup>20</sup>  
 42 v " " 508+15<sup>20</sup> to  $\left. \begin{array}{l} = 538+10.30 \\ = 537+65.30 \end{array} \right\}$   
 48 Ties at = 6+74<sup>00</sup> New Sta.  
 49 " " = 409+03.27 New Sta.  
 50 " " 457+59.79 old Sta.  
 51 " " 512+60.09 " "

For final run see F.B 2040

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Also see F.B 1629  
 L 1631  
 L 1703  
 L 2003  
 L 2054



Prelim. Alvarado Canyon  
 Sewer. Ely. from state  
 Highway multiplate culvert  
 State Station 103+  
 Above Adobe Falls

1-5-49

Sommermeier  
 McCoy  
 Jones

INDEXED

WK

JUL 1 1949

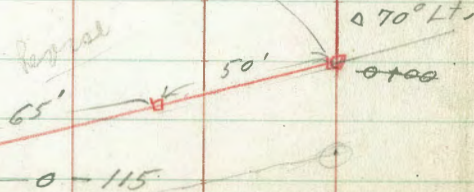
Revised  
 $\frac{2054}{5}$

From  $\frac{2003}{59}$

Corrected station  
 = 488+60<sup>24</sup>

Exist. 21" sewer  
 (under) Multi-plate  
 culvert.

(State Hq. Sta.  
 103+)



cont. P. 11

2

$505+86.52 = P.O.T.$   
 $17+26.28 = P.O.T.$

Revised  
 $\frac{2054}{5}$

$\Delta 45^\circ - 09'$

$17+30 = 502+90^{24}$

49A+93<sup>24</sup>  
 6+33<sup>00</sup>



Sketch - P. 2

490+10  
~~1450~~

489+60  
~~1400~~

489+10  
~~0450~~

488+60<sup>24</sup>  
~~1400~~

0+00 = Δ 70° Lt.

B.M. #1

0-50 ← these 2 shots are  
old line. Not to be  
used 3/3/49

0-115' end of pipe under Const.

R.P. #1

FB 2003

17

7.63 291.01

283.38

City  
Data.

(289.50)  
State Data

2/18/49  
Rec'd. 3

284.7 ✓  
6.3

284.8 ✓  
6.2

285.0 ✓  
6.0

284.38 ✓  
6.63  
Hub

283.38 ✓  
7.63  
Hub

City Returns  
El. 289.9  
INVERT

PI 2003  
17

291.01 City



492+60  
~~4+00~~

T.P.

7.12

~~294.61~~

3.52

~~287.49~~

492+10  
~~3+50~~

491+60  
~~3+00~~

491+10  
~~2+50~~

490+60  
~~2+00~~

490+35  
~~1+75~~

291.01

4

4

288.44

6.2

~~294.61~~

287.00

4.0

285.84

5.2

285.00

6.0

284.94

6.1

283.54

7.5

~~291.01~~



495+10  
6+50

T.P. 6.93  $\langle 298.23 \rangle$  3.31  $\langle 291.30 \rangle$  B.H.#2

<sup>Stub</sup> 24  
494+93  
6+33  $\Delta$  46°-48' LT.

494+60  
6+00

494+23<sup>24</sup>  
5+63 IXI P.O.T. (P.21)

494+10  
5+50

493+60  
5+00

493+10  
4+50

$\langle 294.61 \rangle$

291.1

7.1

$\langle 298.23 \rangle$

291.30 ✓

3.31

Stub

290.8 ✓

3.8

290.37 ✓

4.24

Stub

289.6 ✓

5.0

289.8 ✓

4.8

289.2 ✓

5.4

$\langle 294.61 \rangle$



497+10  
8+50

496+60  
8+00

496+30  
7+70

496+10  
7+50

495+60  
7+00

495+35  
6+75

495+25  
6+65

298.23

4

292.1  
6.1

290.5  
7.7

291.8  
6.7

289.9  
8.3

289.8  
8.4

289.8  
8.4

291.1  
7.1

298.23

5



T.P. 10.86  $\langle 306.55 \rangle$  2.54  $\langle 295.69 \rangle$

500+10  
H+50

199+60  
H+100

499+10  
H+50

498+60  
H+100

498+35  
H+75

498+10  
H+50

497+60  
H+100

$\langle 298.23 \rangle$

4

295.3 ✓  
2.9

294.7 ✓  
3.5

293.9 ✓  
4.3

293.8 ✓  
4.4

292.7 ✓  
5.5

293.1 ✓  
5.1

292.7 ✓  
5.5

$\langle 298.23 \rangle$

7



502 TIP. 11.34  $\langle 315.35 \rangle$  2.54  $\langle 304.01 \rangle$  B.M.#3  
~~462+90.24~~  
~~14+30.00~~  $\Delta 452.09!$  RT. (on hub)

502+60  
~~14+00~~

502+35  
~~13+75~~

502+10  
~~12+50~~

501+60  
~~13+00~~

501+10  
~~12+50~~

500+60  
~~14+00~~

$\langle 306.55 \rangle$

4  
302.34  
1.21

299.8  
6.8

297.8  
8.8

297.6  
9.0

297.3  
9.3

296.6  
10.0

296.0  
10.6

$\langle 306.55 \rangle$



T.P. 10.66  $\langle 325.35 \rangle$  0.66  $\langle 314.67 \rangle$   
~~505+35~~  
~~16+75~~

505+10  
~~16+50~~

504+60  
~~16+00~~

504+40  
~~15+80~~

504+35  
~~15+75~~

504+05  
~~15+25~~

503+60  
~~15+00~~

$\langle 315.35 \rangle$

9

313.4 ✓  
1.9

312.3 ✓  
30

310.1 ✓  
5.2

304.1 ✓  
11.2

306.4 ✓  
8.9

306.4 ✓  
8.9

303.2 ✓  
12.1

$\langle 315.35 \rangle$



Levels Cont  $\frac{2003}{69}$

Chiseled square - Top of dam  
0.5' North of spillway

5.24  $\left\langle \begin{array}{c} 320.11 \\ \downarrow \end{array} \right\rangle$  BM #4

506+10  
17+50

322.0 ✓  
3.4

505+86 <sup>52</sup>

17+26 <sup>28</sup> - P.O.T. on stub. ixl stub. on stub.

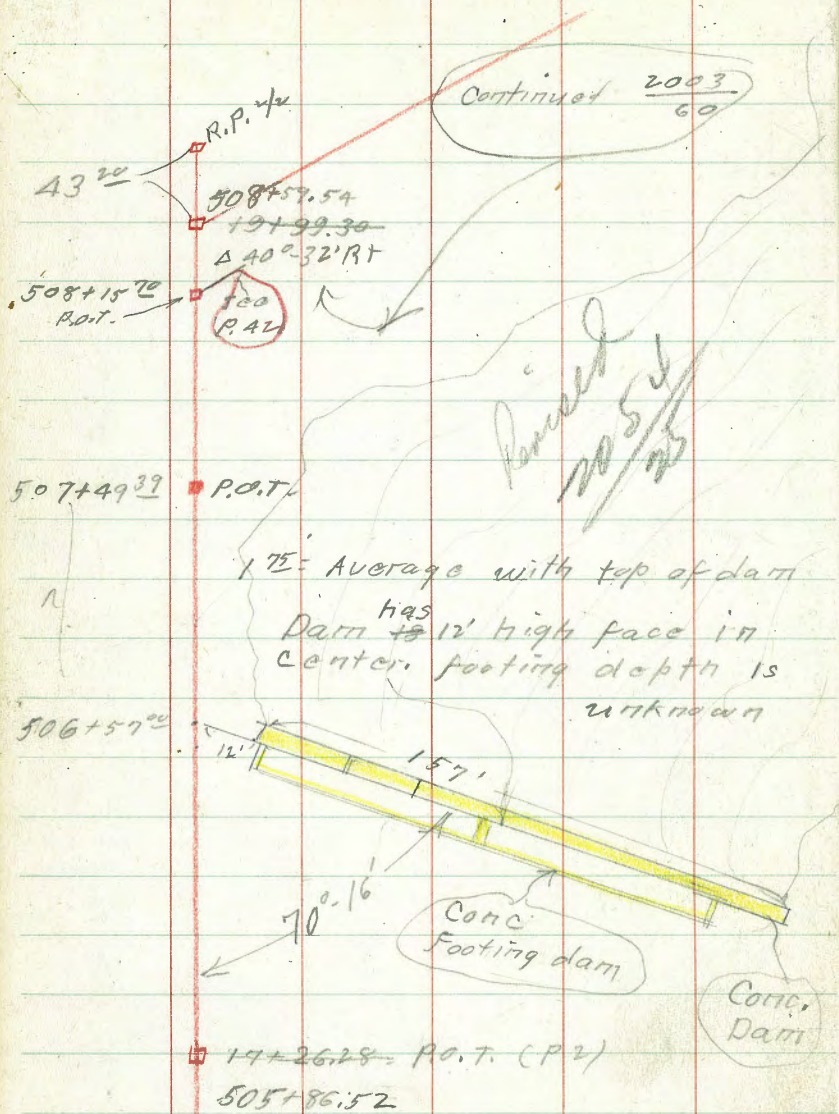
321.44 ✓  
3.91

505+60  
17+20

$\left\langle \begin{array}{c} 325.35 \\ \downarrow \end{array} \right\rangle$

318.4 ✓  
6.9  
 $\left\langle \begin{array}{c} 325.35 \\ \downarrow \end{array} \right\rangle$





From  
Page 2



1/9/49

Tie in line Crossing # 5  $\frac{2003}{16+25}$

INDEXED  
WK  
JUL 1 1949

Transit at "X" on  
all shots.  
Pt. "A" from X  
192' stadia vert: 2° 32'  
No Boot

Pt. B.  
214' stadia 2° 40' vert.  
5' Boot.

Pt. "C" stadia 53' Level  
K=428 Rod=9.2 "

Pt. "D"  
526' stadia  
vert 2° 49' - 10' Boot.

$\Delta = X$	$\pi = A$	Stadia	Vert $\Delta$	Ver Dif	El.
Pt. A	191.8	192.	2° 32'		
Pt. B.	213.8	214.	5' Boot 2° 40'		
Pt. C.	53.0	53'	Rod 9.2 0° 00'		
Pt. "D"	525.4	526.	10' Boot 2° 49'		120.09

state Hy  
Station = 81+30±

12

EL: 186.58  
USGS.  $\frac{2003}{16}$

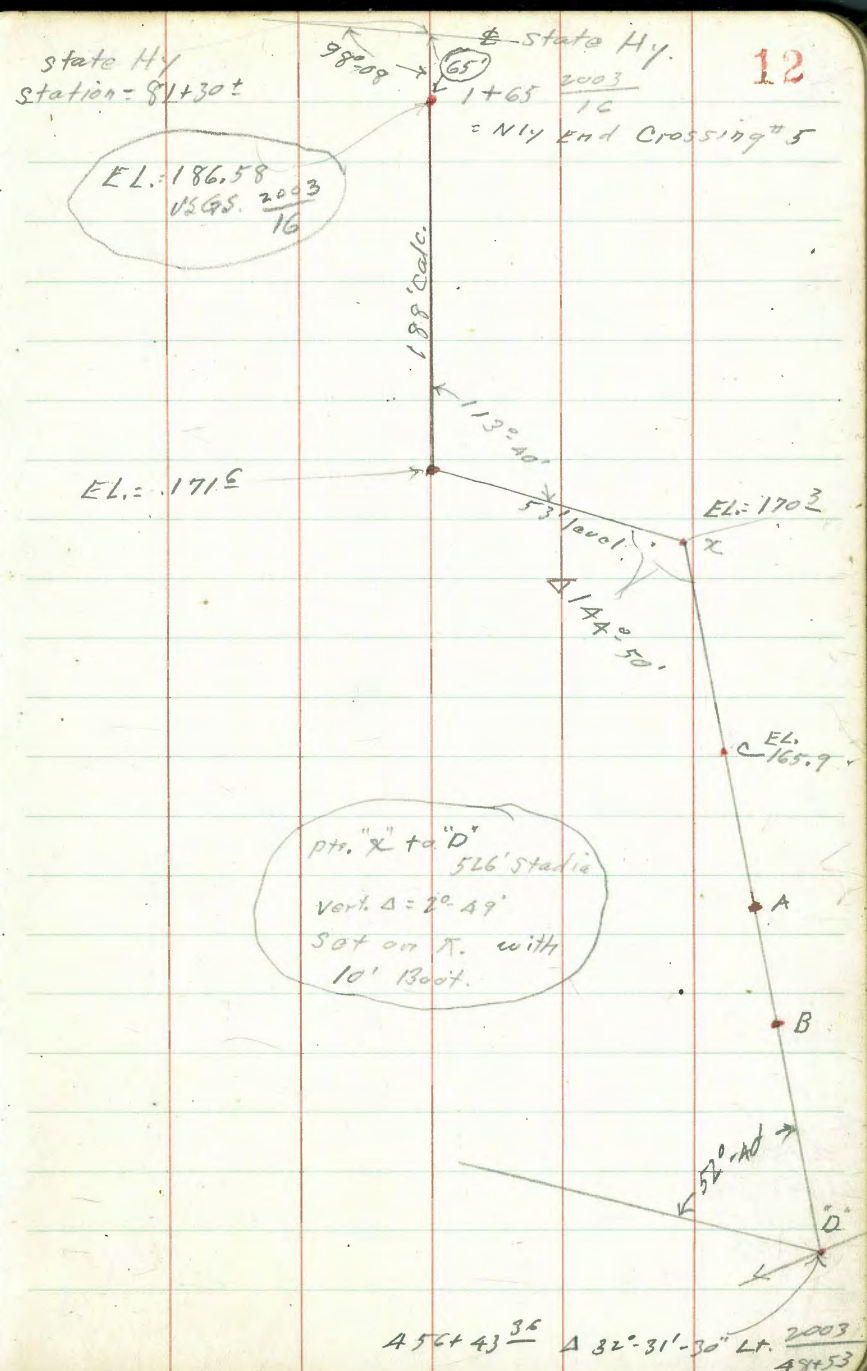
EL: 171.5

EL: 170.3

EL: 165.7

Pt. "X" to "D"  
526' stadia  
Vert.  $\Delta = 2^\circ 49'$   
Set on  $\pi$ . with  
10' Boot.

A 56+43  $\frac{36}{}$   $\Delta 32^\circ 31' 30''$  Lt.  $\frac{2003}{49+53}$





3/9/49

Tie in Crossing #8

$\frac{2003}{20}$

0+91.3 = spike = 0+00  $\frac{2003}{20}$  3.1 373.2'

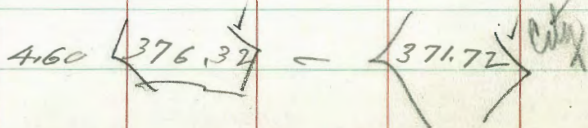
+68 3.5 372.8'

+62 6.8 369.5'

+26 5.9 370.4'

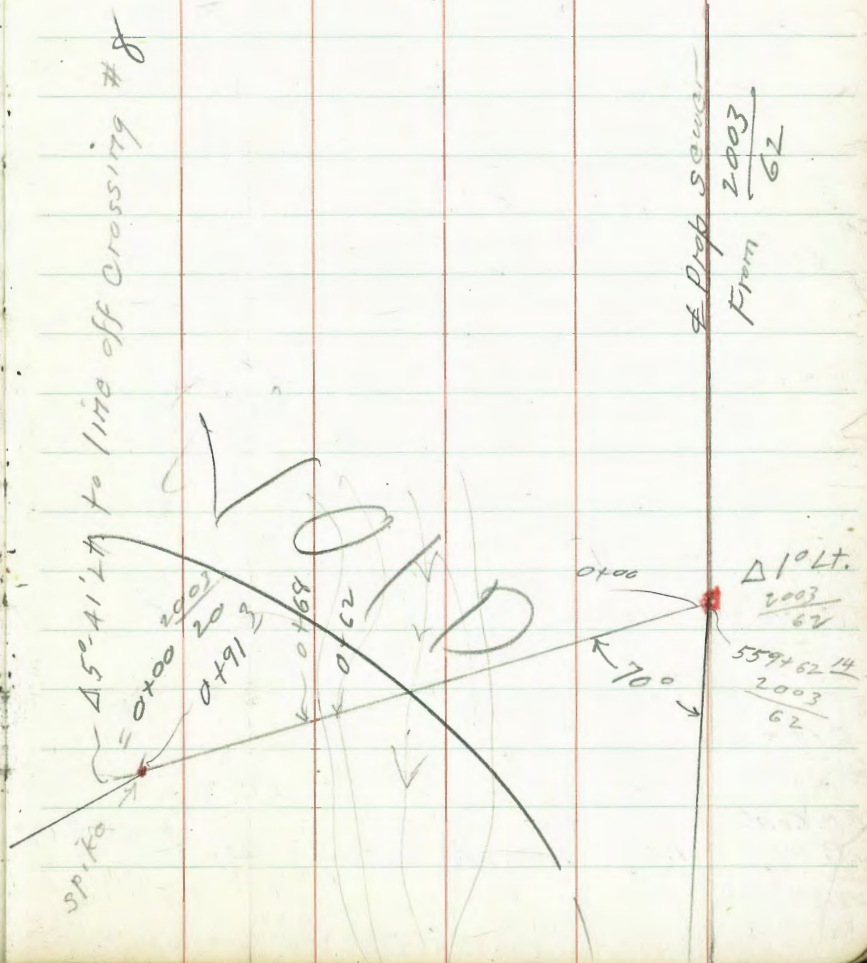
# 0+00 4.6 371.1'

559+62.14  
 $\frac{2003}{62}$   
T.P. P 48



See  $\frac{2003}{20}$

45°-41'24" to line off crossing #8





Profile Sewer branch line (#7)

sketch  $\frac{2003}{61}$

2 + 70 ± = Grad. End of exist pipe

2 + 40

2 + 00

1 + 50

1 + 00

+ 50

+ 20

0 + 10

0 + 00 = P.O.T.

B.M. P.O.T.  $525 + 10.30$   
P. 76  $7.05$   $362.35$   
 $342.35$   
2003 76

$355.30$   $\frac{2003}{76}$

357.7  
4.6

356.5  
5.8

357.6  
5.4

356.4  
5.9

355.7  
6.6

356.6  
5.7

356.1  
6.2

353.6  
8.7

355.30  
7.05

$362.35$   
 $342.35$



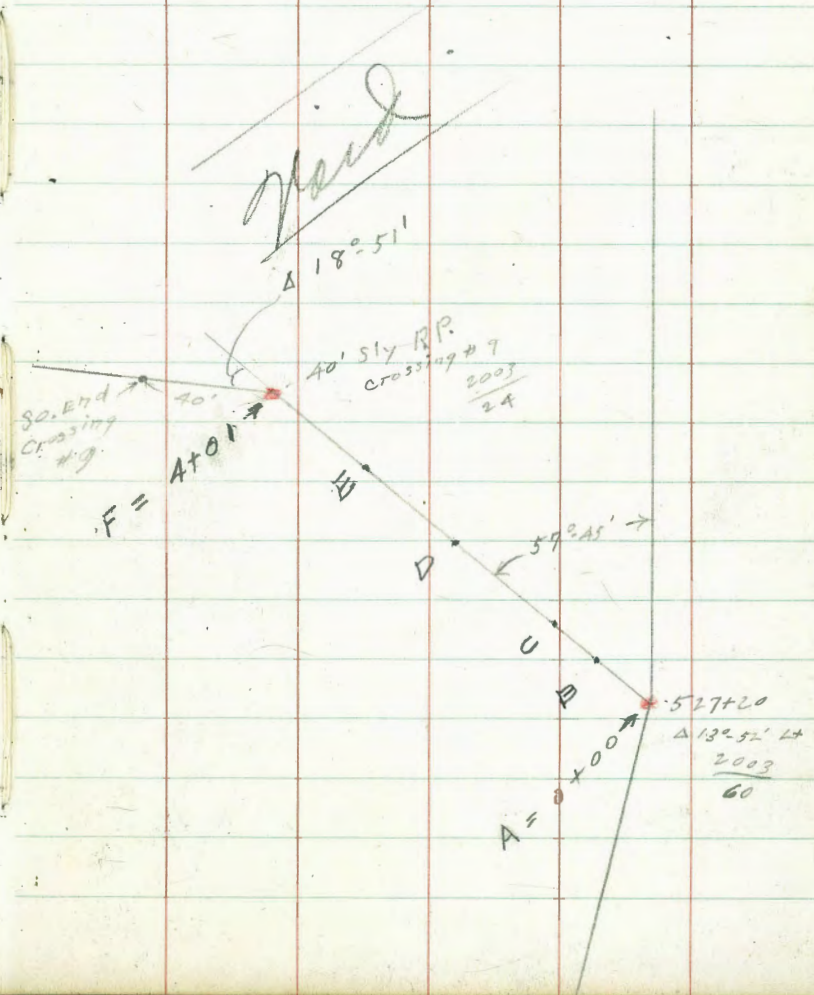
3/9/49

Ordin  
Tie in Sewer Crossing #9  $\frac{2003}{24}$

Distances and Elevations  
Taken by Stadia From "F"

road

Point	Station	Elev.
A	0+00	335.41 $\frac{2003}{72}$
B	0+26	336.6
C	0+80	335.4
D	2+03	339.9
E	2+82	350.7
F	4+01	356.54 $\frac{2003}{24}$

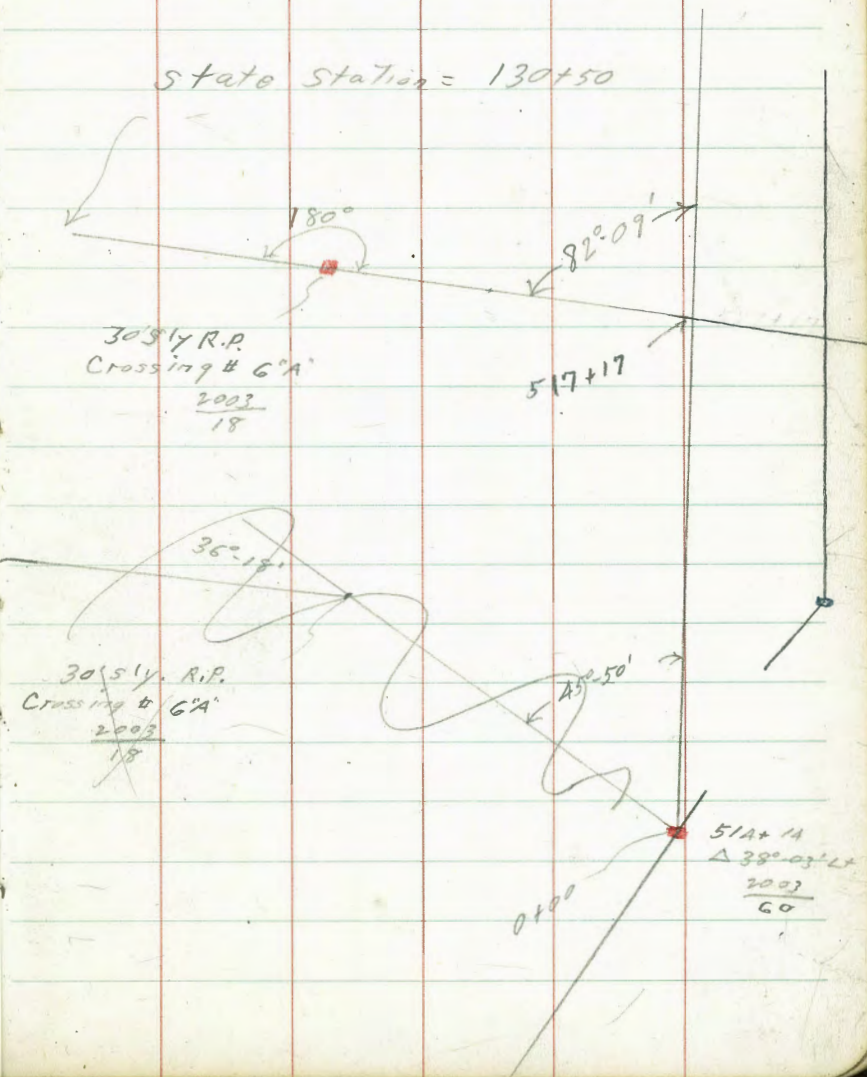




Tie in Crossing # 6 A

517-				328.0
5.4	333.4			Elev
517+17 on Main line =				
0+00	stadia	vert. L	5.0	328.4
93.0	0°-00'		4.5	328.9
190.0	0°-00'		2.6	330.8
272°	+1°-30'			
367°	+2°-0'			

State Station = 130+50









Levels Canyon East of  
Catactan Dr.

3+00  
2+20  
2+00  
1+13  
1+00  
+50  
0+35  
0+17  
0+00  
see p. 17

~~Rec 2054  
56~~

341.8'  
4.5  
340.01  
6.8  
337.94  
8.4  
335.64  
10.5  
335.64  
10.7  
337.01  
7.3  
332.94  
13.4  
331.14  
15.2  
335.41'  
10.92

~~Rec 2054  
56~~

10.92 (346.33)

(335.41)

527+20  
2003  
72

(346.33)



7+19

351.8 ✓  
6.3

7+00 Approx. Int. of water line (P.17)

351.7 ✓  
6.4

+75

350.1 ✓  
8.0

+13

349.4 ✓  
8.7

6+00

347.1 ✓  
11.0

5+70

348.2 ✓  
9.9

5+15

344.9 ✓  
13.2

4+90

345.8 ✓  
12.3

T.P. 12.35  $\langle 358.10 \rangle$  0.58  $\langle 345.75 \rangle$

$\frac{1054}{57}$

~~$\langle 358.10 \rangle$~~

4+00

342.6 ✓  
3.7

$\langle 346.33 \rangle$

~~$\langle 346.33 \rangle$~~



+55

359.1<sup>✓</sup>  
10.6

+45

360.8<sup>✓</sup>  
8.7

11+00

359.9<sup>✓</sup>  
7.8

10+10

357.9<sup>✓</sup>  
11.8

10+00

358.5<sup>✓</sup>  
11.2

+40

356.4<sup>✓</sup>  
13.3

T.P.

12.81

369.71<sup>✓</sup>

1.20

356.90<sup>✓</sup>369.71<sup>✓</sup>

+27

355.0<sup>✓</sup>  
3.1

9+00

356.0<sup>✓</sup>  
2.1

8+00

353.8<sup>✓</sup>  
1.3358.10<sup>✓</sup>358.10<sup>✓</sup>



T.P. 12.10  $\langle 381.48 \rangle$  0.33  $\langle 369.38 \rangle$

15+00

+74

+53

10+00  $\Delta 27^{\circ} 11' \text{ Rt.}$

B.M. #1

+90

+80

+67

13+00

12+00

11+65

$\langle 369.71 \rangle$

$\neq$   
368.71 ✓  
1.0

366.6 ✓  
3.1

367.6 ✓  
2.1

366.60 ✓  
3.11 on Hub.

366.2 ✓  
3.5

363.9 ✓  
5.8

365.5 ✓  
4.2

364.2 ✓  
5.5

362.4 ✓  
7.3

361.1 ✓  
8.6

$\langle 369.71 \rangle$  ✓



Set. B.M. 1/4 pipe R.E. 913

48.587 ft. of 22+19  $\Delta$

5.12 376.36 B.M. #2

22+19  $\Delta$  23° 20' Mt. (P. 17)

~~22+20.96~~  
20.42  
40

22+00

21+00

T.P. 11.60  $\left\langle \begin{array}{l} 392.08 \\ \hline \end{array} \right\rangle$  1.00  $\left\langle \begin{array}{l} 380.48 \\ \hline \end{array} \right\rangle$

20+00

19+00

18+00

17+00

16+00

$\left\langle \begin{array}{l} 391.48 \\ \hline \end{array} \right\rangle$

22

385.1  $\checkmark$   
7.0

384.8  $\checkmark$   
7.3

382.7  $\checkmark$   
9.4

$\left\langle \begin{array}{l} 392.08 \\ \hline \end{array} \right\rangle$   $\checkmark$

380.3  $\checkmark$   
1.2

378.6  $\checkmark$   
2.9

375.8  $\checkmark$   
5.7

373.4  $\checkmark$   
8.1

372.4  $\checkmark$   
9.1

$\left\langle \begin{array}{l} 381.48 \\ \hline \end{array} \right\rangle$   $\checkmark$



28+54 Top of slope  
22' RT. = N.E. Cor. Fill.

28+00

27+00

+74

26+00

25+50

T.P. 12.70  $\left\langle \begin{array}{l} 403.93 \\ \downarrow \end{array} \right\rangle$  0.85 391.23  $\downarrow$

25+00

24+00

23+00

$\left\langle \begin{array}{l} 392.08 \\ \downarrow \end{array} \right\rangle$

397.9  $\checkmark$  396.4  
6.0  $\frac{7.5}{12}$

397.5

6.4

22

N.E. Cor.

Top of Fill

396.5  $\checkmark$   
2.7

394.3  $\checkmark$   
9.6

393.9  $\checkmark$   
10.0

394.2  $\checkmark$   
9.7

390.8  $\checkmark$   
13.1

$\left\langle \begin{array}{l} 403.93 \\ \downarrow \end{array} \right\rangle$

389.9  $\checkmark$   
2.2

387.9  $\checkmark$   
4.2

386.4  $\checkmark$   
5.7

$\left\langle \begin{array}{l} 392.08 \\ \downarrow \end{array} \right\rangle$



Can check it back later.  
 in the bench lines  
 This difference is probably

S.W. B.P.  
 Rolando +  
 El. Cajon

0.85 452.04 (452.46)

T.P. 12.91 (452.89) 0.22 (437.98)

T.P. 12.71 (440.20) 0.45 (427.49)

T.P. 12.33 427.94 0.17 (415.61)

T.P. 12.28 (415.78) 0.43 (403.50)

29+64 End of line (P.17) B.M.#3

29+50 12' RT = toe of slope. S.E. Cor. fill.

29+00 5' RT = toe of slope of fill

(403.93)

399.15  
 4.80 on stub

398.7 397.7 399.4  
 5.2 6.2 4.5  
 5 5 12  
 toe slope

397.5 397.3  
 6.4 6.6  
 5  
 toe slope

(403.93)



Change Alvarado Canyon line

402+00 P.O.T. to 407+45  
407+77.40

7-5-49

Semmermeyer  
Bunch  
Clark

△ = Set stub or hub.  
□ = Fd. " " "

INDEXED  
V.L.  
JAN 18 1950

Sec. P. 49

back line 25

F. 11

410+05  
△ 55° 24 Lt

410+35  
△ 29° Lt  
FB 1629  
19

△ 46° 40' RT  
= 407+77.40 Ahead  
407+45 back

Level 26

△ 61° 06' RT  
407+29  
FB 1629  
19

Now = 402+00 = P.O.T.  
402+00 △ 10° 20' RT  
FB 1629  
18



Levels

7/5/49

Return

7/5/49

82.4  
+ 7.2 to main  
+ 90.8  
+ 1.5  
92.5  
to main pipe  
at 410+05  
top of pipe

402+00 to			
9.4 <sup>2</sup>			
407+37.40	7.8	82.7	82.7
+45	5.2	85.3	✓
A07	4.9	85.6	✓
+50	5.4	85.1	✓
406	7.0	83.5	✓
+50	9.2	81.3	✓
405	11.8	78.7	✓
404+50	12.4	76.1	✓
404	14.3	76.2	✓
+35	15.7	74.8	✓
A03	13.0	77.5	✓
+71	11.3	79.2	✓
+50	10.8	79.1	✓
402+00	9.4 <sup>2</sup>	81.1	

Tie to former job  
407+291  
Hub  
82.7

9/29/49  
Plotted on  
map file  
m.

24' RT. EL = 77.8  
30' LT. EL = 75.6  
30' RT. EL = 78.2  
30' LT. EL = 74.5  
7' RT. EL = 76.8  
15' LT. EL = 73.9

+87.98	11.7	80.6	✓
415	12.8	79.5	✓
414	14.3	78.0	✓
+93	14.0	78.3	✓
+80	5.4	86.9	✓
+71 E Prop Line Sundra.	5.5	86.8	✓
+62	12.2	80.1	✓
+50	12.5	79.8	✓
413	12.1	80.2	✓
412	12.4	79.9	✓
411	11.3	81.0	✓
+55	2.0	90.3	✓
+22	0.0	92.3	✓
410+05	5.3	87.0	✓
+90	2.5	89.8	✓
+83	2.5	89.8	✓
+80	0.8	91.5	✓
+66	1.5	90.8	✓
+50	5.6	86.7	✓
+30	4.7	88.0	✓
+07			
409+06			
0.00	92.26		

Return  
187<sup>m</sup>  
67

11-16-49 Uncoated Tol Cap pipe  
= 92.3 - 5.9  
Ramp  
20' RT + 3' High  
20' LT + 3' Lower  
Top of Cap Pipe  
Top Pipe = 83.4

90.5<sup>2</sup>

92.26

92.26  
Hub  
410+05

1629  
18



Line Change 425+25<sup>00</sup> to 468+47<sup>58</sup>

FB 2003  
38

7/5/49

□ = Fd. Hub or stub  
◻ = sets

Fd stub 432+72<sup>00</sup>  
FB 2003  
38

196

R.P. stub

143.89  
Fly  
Nail in carriage  
5' above ground

$\Delta 7^{\circ}54' \text{ Rt.} = \text{New } \Delta$   
431+26.20  
 ~~$\Delta 2^{\circ}54' \text{ Rt.}$~~

Levels P.32



old line FB 2003  
38

425+45 = P.O.T. - New line  
425+45 = old  $\Delta 8^{\circ}16' \text{ Lt.}$   
FB 2003  
38

27

Final line

Changed  
11/24/49

See P.64  
11-22-49

Sta 446+31.30

Now =  $\Delta 14^{\circ}59' \text{ Lt.}$

447+00 =  
 $\Delta 17^{\circ}00' \text{ Lt.}$

3238

Levels  
P.66

Exist  
Crossing

81<sup>00</sup>47'

442+00 P.O.T.

437+10<sup>00</sup> P.O.T.

Line changed 7/6/49  
use line 5' Rt of  
this line.

use  
this line  
76-49

Wash

432+73<sup>00</sup> = P.O.T.

R.P.  
143.89

$\Delta 7^{\circ}54' \text{ Rt} = \text{New } \Delta$   
431+26.20  
 ~~$\Delta 2^{\circ}54' \text{ Rt.}$~~







Alvarado Canyon line

Change 7/5/49 - 376+47<sup>08</sup> to 381+82<sup>20</sup>

7/14/49

Sammormayer  
McCoy  
W Moore

- = Fd. stub or hub.
- = Fd. Nail
- ▣ = set stub or hub.

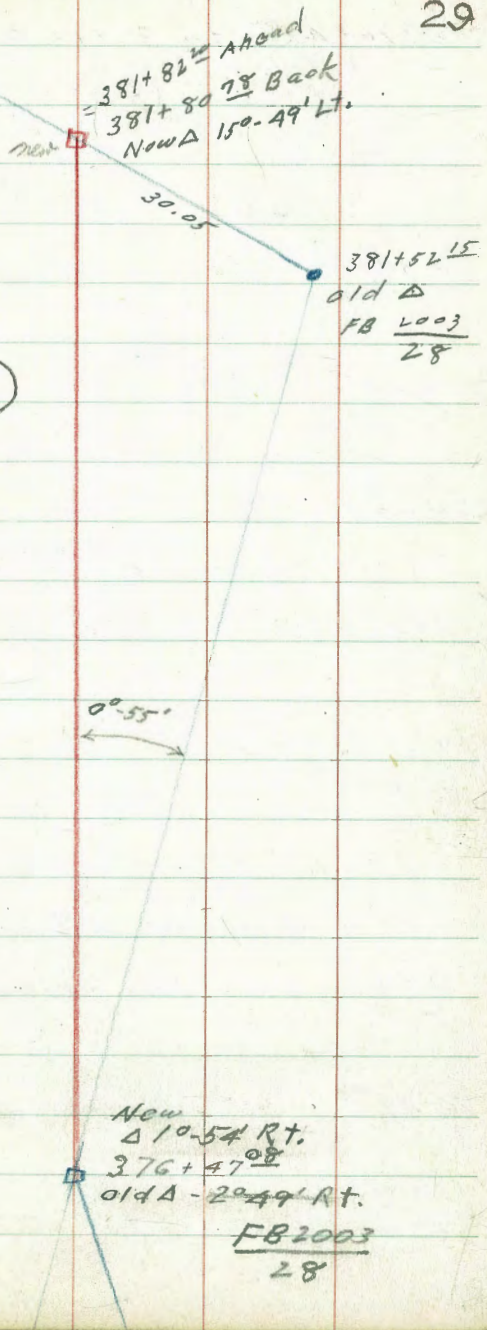
Levels P.30-

FB2003  
28

Ties P.48

Also P.55

New  
Δ 10.54 Rt.  
376+47<sup>08</sup>  
old Δ - 20.49 Rt.  
FB2003  
28





change

376+47<sup>08</sup> to 381+82<sup>20</sup>

sketch. P. 29

379 ~

+ 80

+ 50

378 ~

E.P. = Edge Pavc.

+ 42 25' LT. <sup>near</sup> Edge pavc. - 10' wide

377 ~

+ 65

+ 56 9' Lt. = Tel. pole

+ 54 8' Lt. = Pole (Power)

376+47<sup>08</sup> on stub.

stub 376+47<sup>08</sup>  
2003  
33

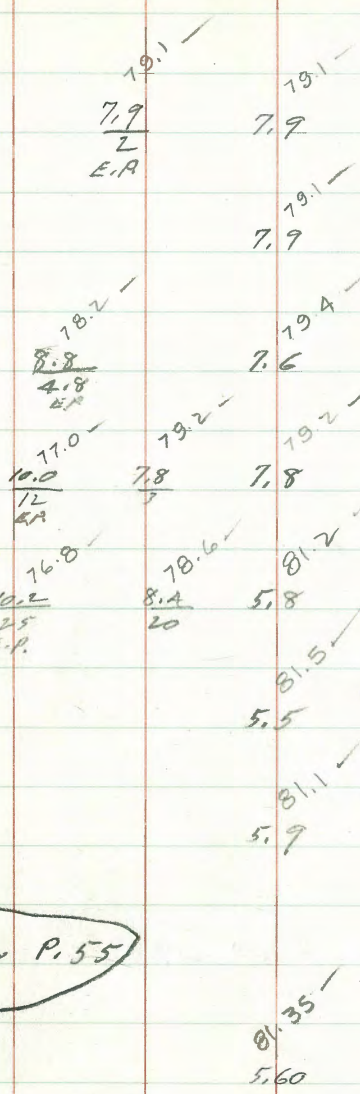
5.60

86.95

81.35

Also P. 55

86.95





388 + 82<sup>to</sup> Ahead ( $\frac{1873}{29} + \frac{2001}{29}$ )  
 388 + 80<sup>to</sup> Back

+74 = E leave paving

LT  
 43 = Edge pave  
 +50 14" Rt. " "

381 + 21 = E meats pave

+75

380 + 50

T.P. 5.88  $\langle 86.70 \rangle$  6.13 80.82

380 ~

+56 7' Rt. = P. pole # 279362

+50

379 + 36 0.5 LT. = Edge pave.

$\langle 86.95 \rangle$

5.86  
 Hub.

81.2  
 5.5

5.0  
 43  
 EP

5.0

4.6  
 142  
 EP

4.8  
 EP

4.7

4.8

$\langle 86.70 \rangle$

7.5  
 32  
 E.P.

6.0

6.8

7.2  
 02  
 E.P.

7.2

$\langle 86.95 \rangle$



Change

425+45 to 468+67<sup>49</sup>

sketch P. 27+28

T.P. 11.74  $\left\langle \frac{103.61}{4.35} \right\rangle$  91.87

+53

428+40

+60

427+20

+90

+80

+46

426

425+45 7.10  $\left\langle \frac{96.22}{4.35} \right\rangle$  89.12

$\frac{1003}{42}$

91.2  
51.0

84.4  
-11.8

82.4  
13.8

91.2  
5.0

80.7  
15.5

80.7  
15.5

90.4  
5.8

89.2  
7.0

89.12  
7.10

$\left\langle \frac{96.22}{4.35} \right\rangle$



432 ~

+ 80

+ 50

431 + 26<sup>20</sup> Δ 7°-54' RL

431 ~

+ 50

430 ~

+ 50

+ 25

429 ~

103.61

3.8

4.0

4.4

3.86  
5746

4.7

6.5

8.0

9.1

11.2

11.8

103.61



435+30

+41

434+39

433+96

+90

433+04

+96

T.P.

1.78

102.19

3.20

100.41

M.O.T.  
432+73<sup>20</sup>

+77

432+30

94.4

7.8

93.5

8.7

97.9

4.3

97.4

4.8

91.0

11.2

89.7

12.5

96.4

5.8

102.19

99.0

4.6

98.6

5.0

103.61



T.P. 6.71 111.38 2.84 104.67

440 ~

439 ~

438 ~

+93

+70 Cross wash (bears 45° to left)

+50

437 ~

436 ~

435+36

T.P. 9.12 107.51 3.80 98.39

102.19

3.0

4.1

5.3

5.4

7.9

7.3

6.6

7.7

9.2

107.51



+36 Leave wash

445 ~

T.P. 11.61 114.69 8.30 103.08

+75

444 ~

443+10 Enter wash

+50

442 ~ 4' Lt. = 18' Diam tree  
stub. P.O.T.

441 ~

440+50

10.8

9.6

114.69

8.2

7.7

8.8

4.1

4.76  
stub

6.1

8.1

111.38

103.9

105.1

103.2

103.7

102.6

107.3

106.62

105.3

103.3



451 ~

T.P. 7.06  $\langle \underline{119.08} \rangle$  2.67 112.02

450 ~

449 ~

+ 70

+ 45

448 ~

447+00<sup>00</sup>  $\Delta 17^\circ \text{ Lt.}$ 

+ 75

 $\langle \underline{+ 31^{30} \text{ sec P64} - 11/24/29} \rangle$ 

446 ~

 $\langle \underline{114.69} \rangle$ 

5.5

 $\langle \underline{119.08} \rangle$ 

2.7

5.1

6.2

7.8

6.8

 $\frac{5.20}{5446}$ 

6.9

8.6

 $\langle \underline{114.69} \rangle$



+50

T.P. 9.77  $\leftarrow$  125.54  $\rightarrow$  3.31 115.77

455~

454~

+95

+05

453~

+50

$$\begin{aligned} &= 74 + 284 \\ 452 \sim & \Delta 50.07 \text{ RT.} \end{aligned}$$

451+50

119.08

10.5

115.0 -

125.54

4.1

115.0 -

5.4

113.7 -

5.9

113.2 -

5.6

113.5 -

6.9

112.2 -

6.0

113.1 -

5.48

113.60 -

5.46

6.2

112.9 -

119.08



459 ~

458 ~

457+90  $\Delta 16^{\circ}-57'-30''$  Rt

+80

+65

457+30

+95

+30

456 ~

125.54

4.9

5.1

4.99

stab.

4.2

5.7

2.6

10.2

10.1

9.2

125.54

120.6 -

120.4 -

120.55 -

121.3 -

119.8 -

117.9 -

115.3 -

115.4 -

116.3 -



+12

464 ~

T.P. 11.86  $\left\langle \frac{135.09}{4.05} \right\rangle 123.23$ 

↓

463 + 80<sup>86</sup>  $\Delta 28^\circ 43'$  R.T.

463 ~

462 + 82<sup>55</sup> P.O.T.

462

461 ~

T.P. 8.24  $\left\langle \frac{127.28}{6.50} \right\rangle 119.04$ 

460 ~

459 + 60<sup>3</sup>

125.54

≠

8.9

126.2 ✓

124.6 ✓

10.5

 $\left\langle \frac{135.09}{4.05} \right\rangle$ 

123.23 ✓

 $\frac{4.05}{5.46}$ 

123.0 ✓

4.3

122.79 ✓

 $\frac{4.49}{5.46}$ 

5.5 121.8 ✓

120.3 ✓

7.0

 $\left\langle \frac{127.28}{6.5} \right\rangle$ 

6.5 119.0 ✓

117.5 ✓

8.0

 $\left\langle \frac{125.54}{8.0} \right\rangle$



468 + 67.49 Ahead

469 + 47.58 Back

(P.28)

4.52

139.57

139.62  
2003  
63

468 ~

+50

467 ~

+50

466 ~

+72<sup>B</sup>

P.O.T. 1/2

T.P.

10.02

144.09

1.02

134.07

465 ~

464 + 50

135.09

41

4.52

4.8

4.3

5.0

6.0

8.0

8.84

H.B

144.09

4.4

6.4

135.09



Line change from Little Murray Ely.

508+15.70 to 538+10.30

537+65.30

7/19/49

Sommermeier  
McCoy  
Allen  
W. Moore

□ = Fd stub

■ = Set. stub

Same point as  
514+14 2003  
60

514+03.95  
Δ 39° 45' Lt

Levels P. 43

Ties P. 51

Revised

508+15.70  
(From P. 11)  
= New Δ

Δ 37° 46' Rt.

= 538+10.30 Ahead FB2003  
.61

537+65.30 Back  
Δ 11° 01' Rt.

Revised

524+00.00  
Δ 6° 56' Lt.

514+03.95  
Δ 39° 45' Lt

Same point as 514+14.00  
2003  
60



508+15<sup>70</sup> to 537+65<sup>30</sup>

Sketch - P. 42

5/19/49

515~

514+03.25 Δ 39°-45' Lt

514~

513~

T.P. 7.69 332.10 1.07 324.41

512~

*Revised  
Location*

511~

510~

509~

508+15<sup>70</sup> Δ 37°-46' Rt.

5.37 325.48

320.11

B.M. 44

P. 10

327.01  
5.1

326.15  
5.95  
5.46

326.2  
5.9

326.2  
5.9

332.10

324.8  
0.7

322.4  
3.1

321.4  
4.1

320.3  
5.2

321.28  
4.20

325.48



T.P. 6.93  $\langle 337.15 \rangle$  2.22 330.82

521~

520+50

520~

519~

518+70

T.P. 2.51  $\langle 333.04 \rangle$  1.57 330.53

518~

517~

516~

332.10

*Revised  
location*

$\langle 337.15 \rangle$

3.1

3.6

5.8

6.0

2.2

$\langle 333.04 \rangle$

0.8

4.3

4.6

332.10

*Revised  
location*

329.9

329.4

327.2

327.0

330.8

331.3

327.8

327.5



150

529~

528~

T.P. 9.29  $\left\langle \begin{array}{l} 346.35 \\ 0.09 \end{array} \right\rangle$  337.06

527~

526~

525~

524+100<sup>m</sup> =  $\Delta$  6°-56' Lt. (P. 42)

523~

522~

$\left\langle \underline{337.15} \right\rangle$

45

5.6

8.7

8.5

$\left\langle \underline{346.35} \right\rangle$

0.5

2.1

3.7

4.98

Stub

5.3

5.8

$\left\langle \underline{337.15} \right\rangle$

*Revised  
location*

*Revised  
location*

340.8

337.7

337.9

336.7

335.1

333.5

332.22

331.9

331.4



+40

5.1

345.4

+25

6.8

343.7

535+06

7.9

342.6

+85

5.2

345.3

534~

5.9

344.6

T.P.

6.60

350.51

2.44

343.91

350.51

533~

2.8

343.6

532~

4.0

342.4

531~

6.1

340.3

530~

5.4

341.0

346.35

346.35

Remnant  
SectionRemnant  
Section



Remind  
Location

538 + 10.30 Ahead }  
537 + 65.30 Back } =  $\Delta$  11°-01' RT. (P. 42)

537 ~

536 ~

350.51

Remind  
Location

348.27  
2.24

346.8  
3.7

346.2  
4.3

350.51



Ties Grantville + Out lots

Map 776

8/29/49

○ = Ad pipe or pin

□ = Ad Hub

▣ = Set Hub

Semmermeyer

McCoy

Allen

Rorer

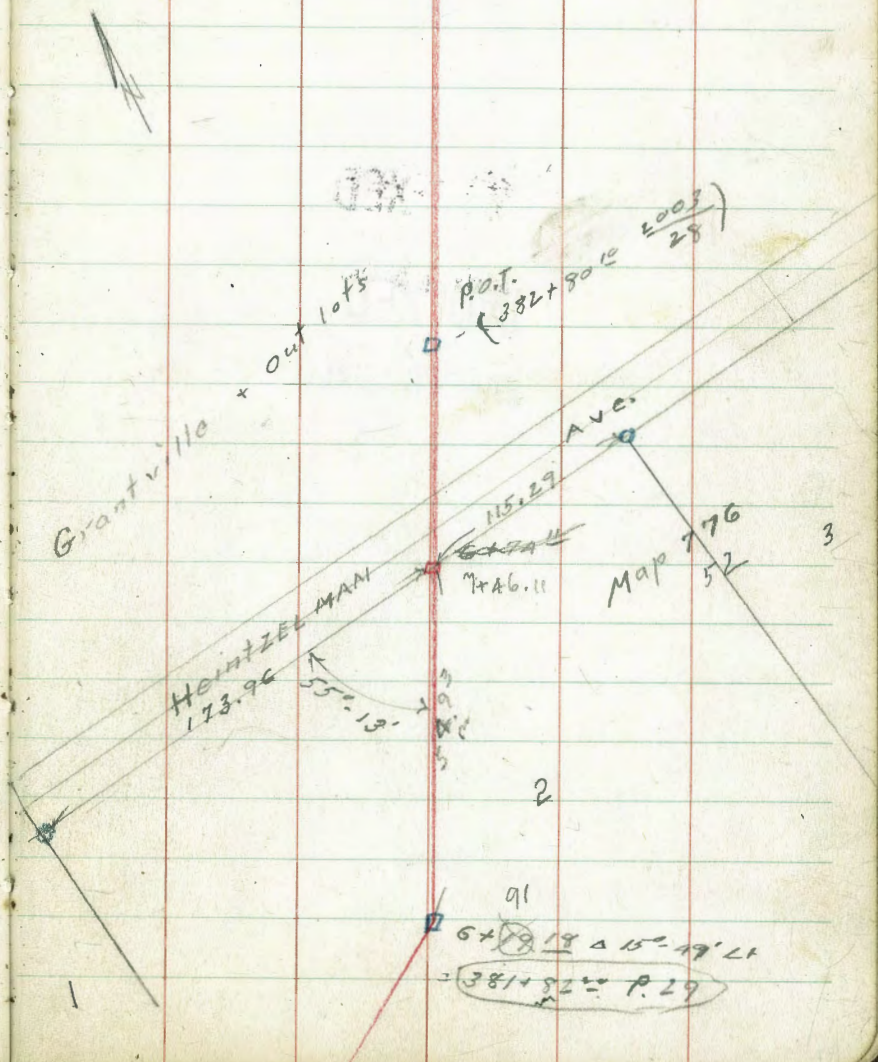
INDEXED

M.K.

JAN 18 1950

48

211+53.75 ✓  
386+442 ✓  
2007  
28



point.  
382+8015  
2007  
28

Map 776  
52

91  
6+19.19 Δ 15°-19' 21"  
381+822 P.29

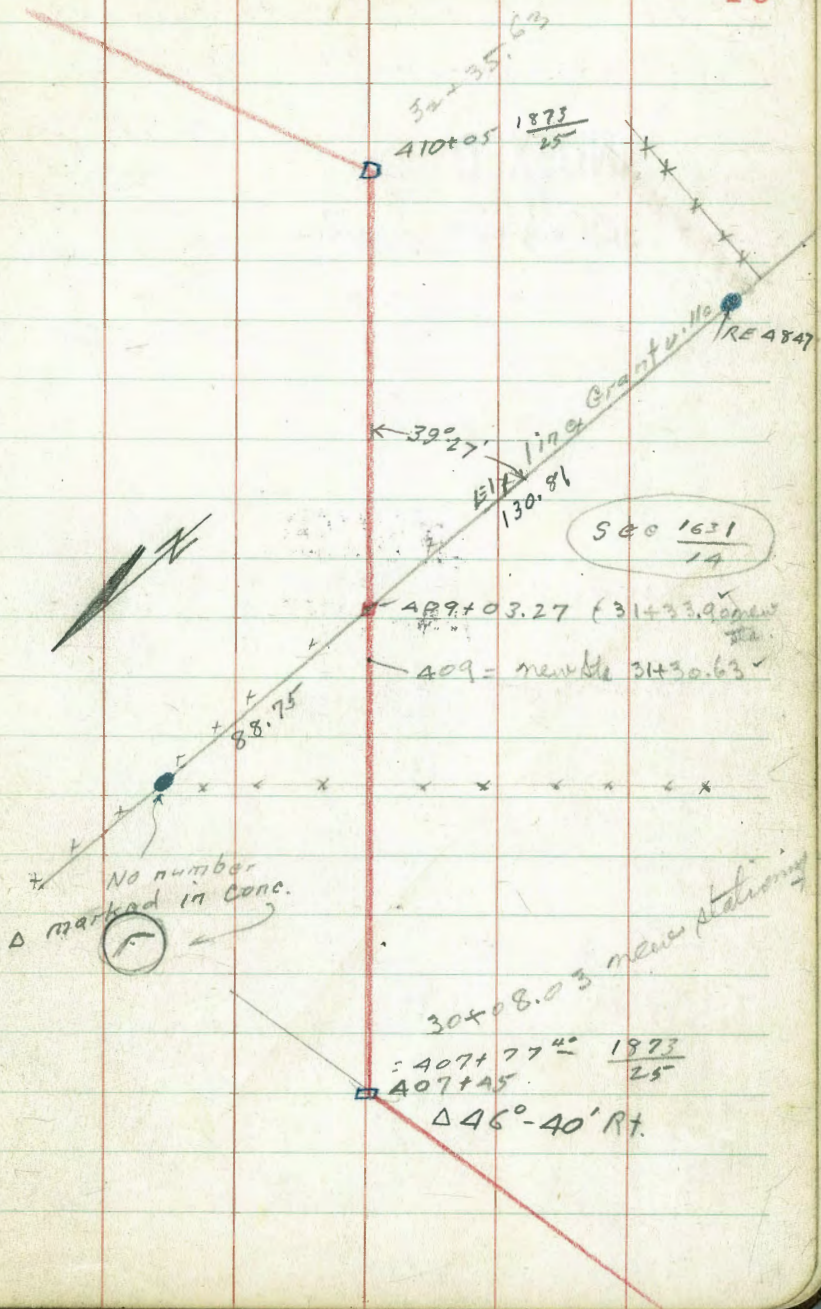


Ties East line Lot 53  
Grantville & out lots

8/20/47

- = Fd. Hub
- = Fd. Conc. Filled 2" pipe
- = Sat. Hub

INDEXED  
M K  
JAN 18 1950









Ties at Sta. 512+60.09

Ely. line of college grounds

8/30/49

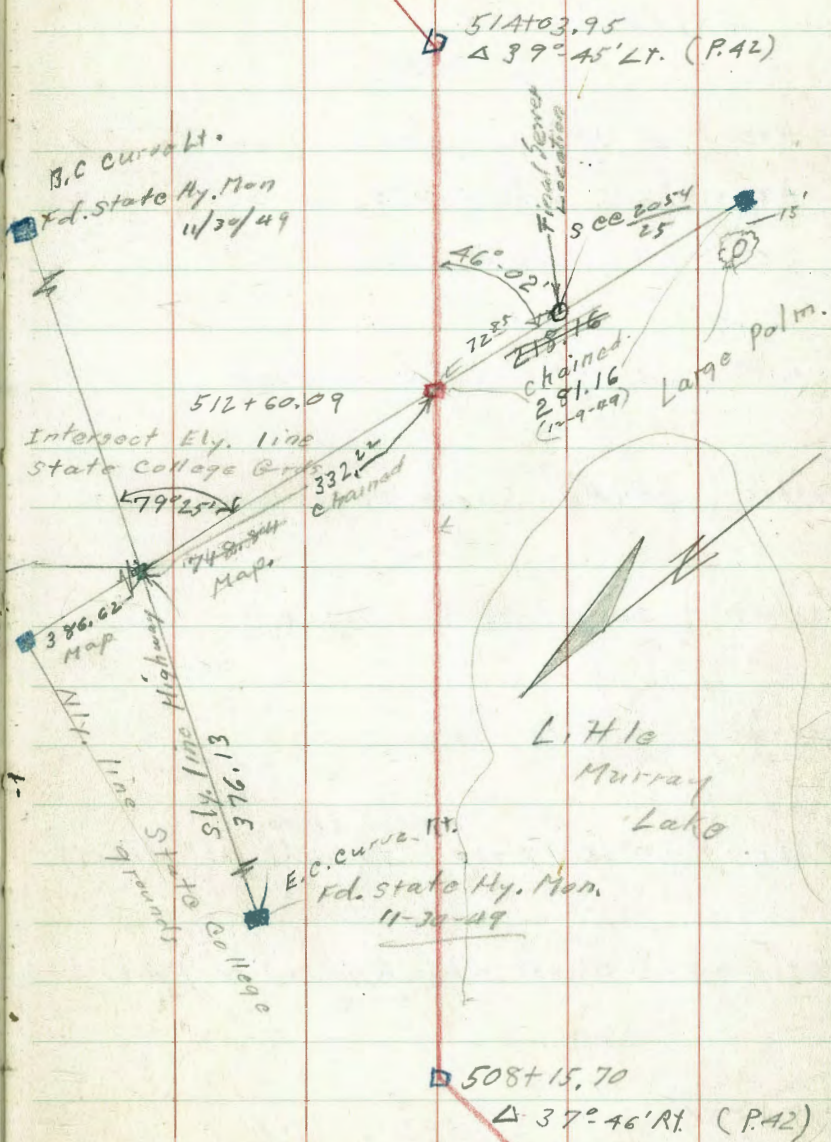
Sommermeier  
MSCOJ  
Allan  
Rorer

■ = Ed. Map #1880 (N. Glover)

□ = " 1/2 or stub.

▣ = set stub.

Tie to state Hy. - Made 11/30/49

set stub  
11/30/49



Additional notes on Location  
of fences, Bldgs. etc. 9/29/49

381+52<sup>15</sup> to.

see 2003  
28

+92 4' Rt. = Cor. shed

+91 = barb. wire fence.

+83 9' Rt. = Cor. shed

+63<sup>2</sup> 2' Rt. = 6" diam. olive

383+55 chicken wire fence

383+48 22' Rt. = Cor. house

383+42 = chicken wire fence

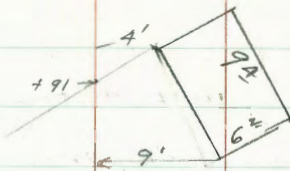
Pepper tree  
382+99 - 9' Lt. = Trippe trunk (12" diam)

382+49 cross chicken wire fence

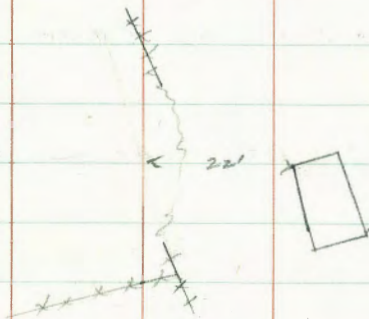
381+52<sup>15</sup>  
 $\Delta 16^{\circ} 45' - 30^{\circ} Lt.$   $\frac{2003}{28} + \frac{1873}{29}$

⊕

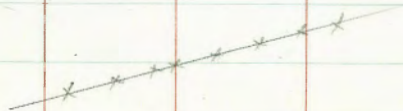
52



⊙



⊙ 9'



⊙



Station same as in  $\frac{1631}{12}$

9/29/49

391+74

+61 10' Rt. = End board fence

391+56<sup>72</sup> =  $\Delta$  11°-41' Lt.

+14 4' Rt. = start board fence

+11 11' Rt. = Cor shed

391+02 8' Rt. = Cor. shed

+98 7' Rt. = Power pole

390+75 19' Rt. = cor. shed

+54 13' Rt. = start conc. wall

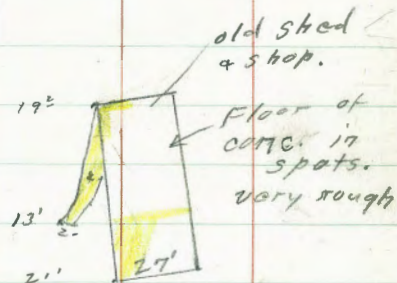
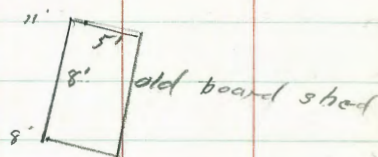
390+25 21' Rt. = cor shed

388+90<sup>75</sup>  $\Delta$  Rt. 55°-56' ( $\frac{1631}{12}$ )

⊕

53

xxxxxxx





9/29/49

±

54

394+45 31<sup>±</sup> <sup>ctr.</sup> Rt. Circular water trough. 4<sup>±</sup> outside diam.

T.P. 1.53  $\langle 79.72 \rangle$  11.30  $\langle 78.19 \rangle$   
 393+71<sup>61</sup> 0.68  $\langle 89.49 \rangle$  — 88.81  
 P.O.T.  
 1631  
 13720

69.3	71.3	74.1	72.8	71.6	76.7
10.4	8.4	5.6	6.9	5.1	1.0
50		33 <sup>±</sup>	35	70	100

$\langle 79.72 \rangle$  Top of corr.

89.49



7-29-49

55

See P. 30

+93 12' Lt. = Pole # 519746H

+73 14' Lt. = deadman

+40

375+15 Top of bank

375+01 Top of bank

374+89<sup>6</sup>  $\frac{2003}{P33}$

376+47<sup>8</sup>  
P. 30

7.57

88.92

81.35

on a stub

85.1 ✓  
3.8

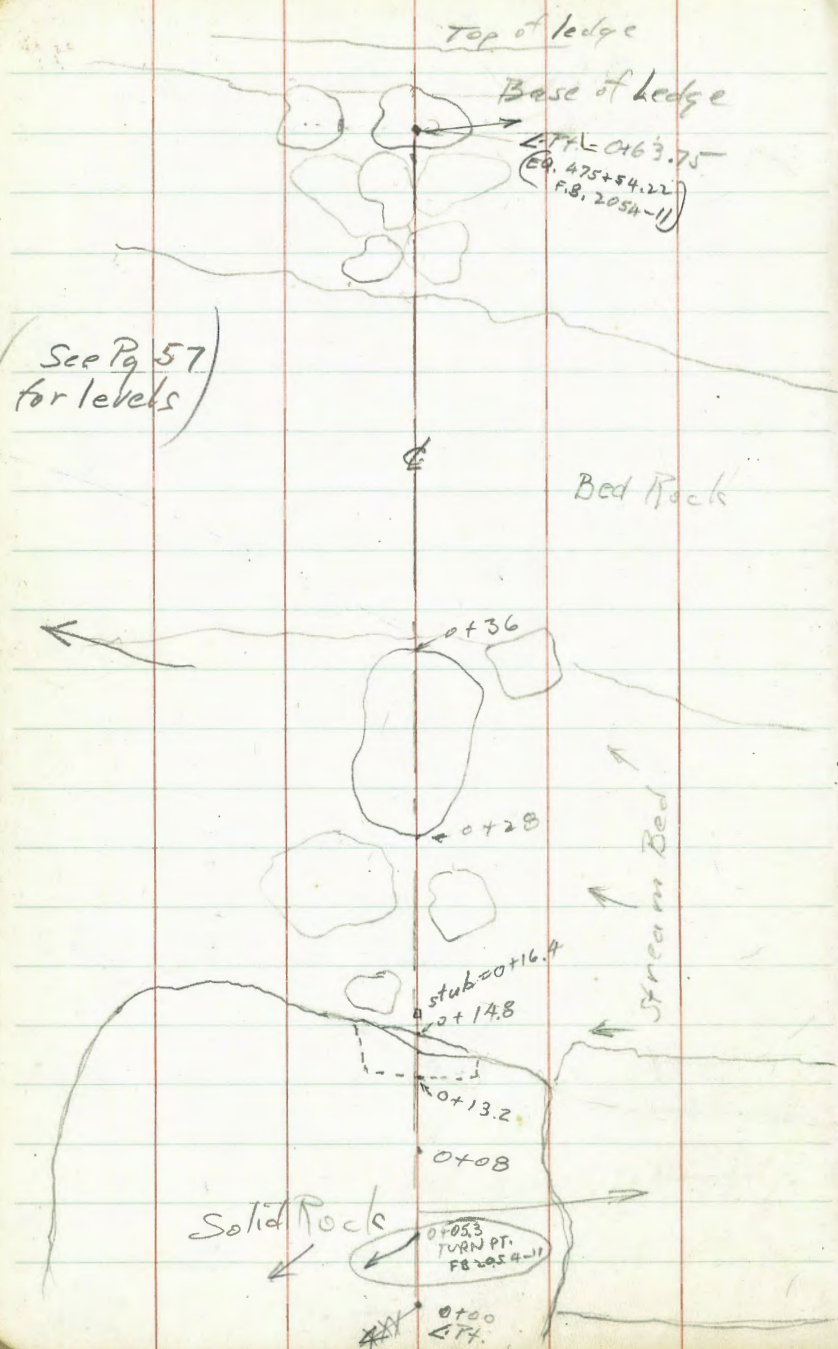
85.0 ✓  
3.9

80.7 ✓  
8.2

80.4 ✓  
8.5

88.92







10/10/49  
 NO. 20129  
 N. Newcomb  
 R. COY  
 W. Moore

#1 Bridge Site -  
 Profile of Sewer X-ing. of Wash  
 Above Sta. 473+96.66 (2003)  
 as Recommended by H. Cole  
 (See Pg 56 for Sketch)

- 0+15 Bottom of Wash. (Gravel)
- +14.8 Lower level of Rock forming Niche
- +14 Overhang on  $\perp$
- +13.5 Break at edge of Rock
- +13.2 Underhang forming Niche in Rock
- 0+08 High pt. ( $\perp$  3' Higher)
- 0+00 Proposed  $\perp$  outlet X-ing  
 Solid Track

Stake at  
 473+96.66  
 2003  
 63

105  $\langle 219.61 \rangle$

218.56

$\langle 219.61 \rangle$

LT.	$\perp$	RT.
198.1	213.3	213.3
21.5	6.3	6.3
10	5	3
201.0	209.9	204.9
15.6	15.7	14.7
2		5.5
201.1	201.1	201.1
12.5	12.5	12.5
2.0		1.8
201.0	210.2	210.2
5.6	6.2	7.9
1		2
201.6	209.6	204.6
12.0	12.0	12.0
2.0		1.8
214.81	215.11	211.4
7.8	7.5	5.2
5		4
214.6	214.5	213.3
5.0	5.1	6.3
5		5

Face of  
 ledge  
 Broken ledge  
 Edge  
 cut out of  
 214.6  
 7.0

Edge Rock  
 Solid Rock



0+63.75 3' x 3' x 4' Rock over L.R.  
3' ht. Large Boulder

58

261	25.9	22.2	211.5	24.8	25.8
3.5	3.7	7.4	8.1	4.8	3.8
10	5.0	7.8	(Natural Ledge Approx)	2	10
Ledge			Ledge		

+57 center of Group of Large Rocks

216.4	25.6	208.2	220.4	25.1	221.4
3.2	4.0	11.4	11.2	4.5	5.2
10	3	Top on Bed Rock	8	9	10
Crackled Ledge		Ledge		Ledge	

0+49 Section across Bed Rocks

11.7	208.3	11.4
10	11.3	10
Face of Ledge		

+36 Begin Exposed Bedrock - End L.R.  
2' ht. 4' x 6' Louse Rock

205.9	201.6	208.2	206.1	207.7
13.7	12.0	11.4	14.4	11.9
11	5	5	5	10
Stream Bed				

+33 High point of Large Rock

211.6  
8.0

0+28 Begin Large Rocks

200.0	208.0	208.5	206.1	206.4	204.4	205.0
19.6	11.6	11.3	13.5	13.5	9.2	15.2
12	11	3	2	2	2	3
Large Piece of Ledge			L.R.		Stream Bed	

0+16.4 Stub 2' ht. 4' x 4' Boulder

204.6  
15.4

(219.61)



0+68 Face ledge at top.

220.6 ✓  
+1.0

0+66 Base of Ledge (Projection of £)

218.9  
0.7  
10

326.5  
4.1

212.0 ✓  
7.6

249.2  
5.4  
1

245.4  
4.2  
10

<219.61> ✓



10/12/49 Profile of Sewer X-ing of Wash  
 Above 480 + 15.68  $\frac{2003}{58}$   
 as recommended by H. Cole WD 20129  
 (see pg 63 for Sketch)

Alvarado Sewer 60

- 0+08 On Lower Ledge
- 0+07 Extreme of Overhang
- 0+05 Junction of Bottom of Overhang and Lower Ledge
- 0+04.7 Edge Ledge
- 0-06 On Solid Ledge

Station	Left	Center	Right
0+08	$\frac{12}{12}$ 246.0	$\frac{10}{10}$ 248.0	$\frac{17.0}{3.0}$ 241.0 DIT
0+07			$\frac{16.1}{2.0}$ 241.9 Edge Ledge
0+05		$\frac{7.8}{0}$ 240.2	$\frac{15.5}{15.9}$ 242.5 244.6
0+04.7	$\frac{0.9}{1.0}$ 257.1	$\frac{3.9}{2}$ 254.1 253.6	$\frac{5.4}{2.1}$ 252.6 245.6
0-06		$\frac{0.9}{1.0}$ 257.1	$\frac{4.5}{1.0}$ 253.5
0+00	$\frac{1.7}{1.0}$ 256.3	$\frac{2.8}{6.0}$ 255.2 256.3	$\frac{5.2}{9}$ 252.8 250.8

5' ledge to south  
Edge of ledge to south

0+00 =  $480 + 15.68$   
 $\frac{2003}{58}$   
 X in Rock (E 3' up)  
 (580 480+16.20)  
 FB 2054-16)

Stub at 480 + 780.5  
 $\frac{2003}{66}$   
 1.25  $\leftarrow 258.05 \rightarrow$  256.8

$\leftarrow 258.05 \rightarrow$



0+45 Top of Broken Cube + Rock

238.0	19.0	247.7	4	242.8	2	239.9	7	240.2	61
70	7	10.3	1.2	15.2	1.8	17.8	17.8	14.1	245.9
6	30	2.5	2	1.5	0.5	8	10		
	Top corn ledge		Edge Broken cube		Bed Rock		Loose Rock		

0+44 Corner Ledge (Corner loose cube of) Rock

240.6	240.0	240.2
17.9	18.0	17.8
10	10	10
	Sand at Toe	Toe

0+30

240.0	240.2
17.7	17.8
10	10
	Toe

0+24

240.0	240.2
18.7	18.0
10	10

0+20 & Low Water channel

238.7	238.0	238.3	249.0
19.3	20.0	19.7	18.0
10	10	10	10
		in low channel	

0+13.7 Edge Lower Ledge

239.7	245.1	242.0	239.3	237.0
19.3	13.9	16.0	18.7	20.1
10.2	10	0.7	10	10
stream bed			Low channel	

0+12

245.5	242.2	241.3	238.4	239.0
12.5	15.8	16.7	18.6	19.0
10	3.7	4.0	4.0	10
	Corner Lower ledge	stream bed	Open stream bed	

(258.05)



0+67 Upslope on Projection of  
Same line

Stubs  
0+56.8 Proposed  $\leftarrow$  Lt.

0+54 On Natural Slope

+49 to +50 Loose Boulder

0+47

0+46.8 Back edge of Broken Cube  
of Ledge

Lt.

$\phi$

Rt.

62

262.3  
262.05  
 $\frac{+4.3}{10}$   $\frac{+4.0}{10}$   $\frac{+5.2}{5}$   $\frac{+3.0}{10}$

255.0 255.5 254.1 253.8 254.8 254.8  
 $\frac{3.0}{10}$   $\frac{2.5}{3}$  3.92  $\frac{4.2}{1.2}$   $\frac{3.2}{8}$   $\frac{3.2}{10}$

Natural Slope  
(Loose Rocks Between)

254.1 253.6 251.4 251.4 248.4  
 $\frac{3.7}{10}$   $\frac{4.4}{4}$  6.6  $\frac{6.6}{Broken}$   $\frac{9.6}{10}$   
Ledge Rock

253.4  
4.6

239.5 250.9 251.6 252.0 247.4 246.8 245.1  
 $\frac{18.5}{11}$   $\frac{7.1}{10}$   $\frac{6.4}{7}$   $\frac{6.0}{1.5}$  10.6 11.2 12.9  
Edge Loose  
Ledge Rock

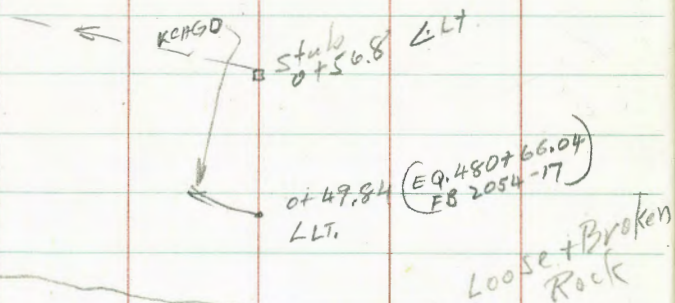
243.3  
19.7

{ 258.05 }



(See Pg. 60 for Levels)

63



Ledge

Loose Section of Ledge Resting on Bedrock

Flow

E

Low Water channel 0+20

0+13.7  
0+12  
0+08

0+05

$480+568 = 0+00$   
 $\frac{2003}{58}$   
0-06



11-22-49

Levels 446+31<sup>20</sup> to 451+97<sup>48</sup>

64

+38

108.14  
6.5

+18

106.44  
8.2

448 ~

106.94  
7.7

+85

108.64  
6.0

+67

105.64  
9.0

+28

108.44  
6.2

447 ~

108.24  
6.4

+62

107.54  
7.1

446+31<sup>20</sup> Δ = 140.59' Lt.

106.674  
7.91  
Stub

5.09 < 114.58 > - < 109.47 > 447+00 P.37

< 114.58 >



11/24/49

(= old sta. 452 to  $\Delta$  5° 07' Rt. P. 28)

451+97<sup>43</sup> =  $\Delta$  3° 06' Rt.

+50

451 ~

450+50

T.P.

7.19  $\langle 120.01 \rangle^{\vee}$  1.70  $\langle 112.82 \rangle^{\vee}$

450 ~

+50

449 ~

448+50

$\langle 114.58 \rangle^{\vee}$

65

113.61<sup>1</sup>  
6.40  
Pub.

113.9<sup>1</sup>  
5.1

114.3<sup>1</sup>  
5.7

113.5<sup>1</sup>  
6.5  
 $\langle 120.01 \rangle^{\vee}$

112.7<sup>1</sup>  
1.9

111.6<sup>1</sup>  
3.0

109.9<sup>1</sup>  
4.7

108.8<sup>1</sup>  
5.8

$\langle 114.58 \rangle^{\vee}$



Levels 446+31<sup>30</sup>

11-22-89

Sly. to Crossing #4 (2003-P15)

3+23<sup>4</sup> = Start. @ existing sewer crossing

3 ~

+50

T.P. 12.84  $\left\langle \begin{array}{l} 131.21 \\ \end{array} \right\rangle$  0.73  $\left\langle \begin{array}{l} 118.37 \\ \end{array} \right\rangle$  T.P.

2 ~

+50

1+23<sup>4</sup> (Aprox. to old line)

1 ~

+50

+19

0+00 = 446+31<sup>30</sup> (P. 6A) sketch-P 27, 446+31<sup>30</sup>

12.43  $\left\langle \begin{array}{l} 119.10 \\ \end{array} \right\rangle$  ~  $\left\langle \begin{array}{l} 106.67 \\ \end{array} \right\rangle$  P 6A.

#

00

130.6<sup>v</sup>

0.6 - End.

127.9<sup>v</sup>

3.3

123.0<sup>v</sup>

$\left\langle \begin{array}{l} 8.2 \\ 131.21 \\ \end{array} \right\rangle$

116.9<sup>v</sup>

2.2

112.5<sup>v</sup>

6.6

111.5<sup>v</sup>

7.6

110.5<sup>v</sup>

8.8

110.5<sup>v</sup>

8.6

109.9<sup>v</sup>

9.2

106.6<sup>v</sup>

12.43

$\left\langle \begin{array}{l} 119.10 \\ \end{array} \right\rangle$



1/2 849.  
C.S.

Levels

F.B. 2003  
37

4

67

N. 413+ to 433+

T.P. 6.00 <91.60> 12.21 <85.60>

415 ~

+89 22' 18" = out let 48" Culvert

+75

+60

414 ~

+90

+87 Δ-2°-32' Lt.

413+57 35+49.63

5.55 <97.81> — <92.26>

2003  
1

Also in  
Hub.  
A10+25  
2003  
39

84.71  
13.1

80.41  
12.4  
80.26  
17.55  
22  
2.5

80.11  
17.7

85.01  
12.8

86.71  
11.1

86.81  
11.0

88.81  
9.0  
and.

88.91  
8.9

<97.81> ✓<sup>N</sup>



T.P. 12.14  $\left\{ \begin{array}{l} 93.88 \\ \hline \end{array} \right\}$  9.86  $\left\{ \begin{array}{l} 81.74 \\ \hline \end{array} \right\}$   
420~

81.1 ✓  
10.5

419~

80.3 ✓  
11.3

state B.M. (30' off. state)  $\left( \begin{array}{l} + 0.33 \\ 90.51 \\ \hline 90.84 \end{array} \right)$  state  
Approx 64' Lt. of. 418~ 6.88  $\left( \begin{array}{l} 6.12 \\ \hline 84.72 \end{array} \right)$

418~

80.1 ✓  
11.5

+52 12<sup>3</sup> Rt. = outlet 42" Culvert

79.6 ✓  
12.0  $\left( \begin{array}{l} 80.10 \\ \hline 11.50 \\ \hline 122 \\ \hline I.E. \end{array} \right)$

417~

B.P. S.E. Cor Blow off Valve Box  
Lt. 416+07 10.45 81.15

80.1 ✓  
11.5

416~

80.8 ✓  
10.8

+80

81.1 ✓  
10.5

415+54

86.3 ✓  
5.3

$\left\{ \begin{array}{l} 91.60 \\ \hline \end{array} \right\}$

$\left\{ \begin{array}{l} 91.60 \\ \hline \end{array} \right\}$



428~

91.9 ✓  
8.1

427+44<sup>20</sup> stub. P.O.T.

92.2 ✓  
7.80

T.P. 6.59 <99.99> ✓ 0.48 <93.40> ✓

<99.99> ✓

427~

92.5 ✓  
1.4

426~

90.0 ✓  
3.7

425~

88.1 ✓  
5.8

424~

84.1 ✓  
9.8

423~

83.7 ✓  
10.2

422~

82.7 ✓  
11.2

421~

<93.88> ✓

81.8 ✓  
12.1

<93.88> ✓



+92

+70

+38

431+26<sup>50</sup>

△

19+

+05

431~

430+90

Set B.M. on Hub

62<sup>54</sup> + 430+00

430~

429~

 $\langle 99.99 \rangle$ 
6.07  $\langle 93.92 \rangle$ 98.4<sup>✓</sup>  
1.897.9<sup>✓</sup>  
2.196.0<sup>✓</sup>  
4.092.4<sup>1✓</sup>  
7.5892.00<sup>✓</sup>  
8.096.4<sup>✓</sup>  
3.695.1<sup>✓</sup>  
1.995.4<sup>✓</sup>  
4.693.4<sup>✓</sup>  
6.6
 $\langle 99.99 \rangle$



BM 068  
 1629 spike in tree 3.97  
 11

-0.09

95.93

96.02 ✓

+07

433—

+85

+73

+55 128' Rt. = outlet 42" Culvert.

432+13

99.99 ✓

71

89.5 ✓  
 19.5

95.5 ✓  
 2.5

98.3 ✓  
 1.7

91.1 ✓  
 8.9

91.3 ✓  
 8.7

95.5 ✓  
 4.5  
 128  
 I.E.

90.1 ✓  
 7.9

99.99 ✓



Sketch Page 50

11-29-49  
S.W.  
McCoy  
Jack  
K.

Levels Connection to 5 Freeway King

5+00		5.7	134.4 ✓
+40		8.6	131.5 ✓
+68		12.3	127.8 ✓
5+75		10.8	129.3 ✓
T.P.	12.82 <140.14> ✓	1.10	<127.32> ✓
6+20		2.0	126.4 ✓
6+50		5.0	123.4 ✓
6+75		6.9	121.5 ✓
7+00		8.5	119.9 ✓
7+33		12.2	116.2 ✓
7+44.85 to 2nd		12.05	116.37 ✓
ASB+10.52		5.96	123.06 ✓
Stob AS1+40 Page 39	7.87 <128.42>		<120.55>

Top Pipe  
8.5% Cor.  
State Col. Group



2+30

9.3 153.7 ✓

2+55

12.0 151.1 ✓

I.P.

12.21

 $\langle 163.05 \rangle$  ✓

0.07

 $\langle 150.84 \rangle$  ✓

3+00

2.4 148.5 ✓

3+50

2.7 148.2 ✓

3+83 total

3.32 147.59 ✓

ground

4+05

11.0 139.9 ✓

4+29

12.6 138.3 ✓

4+52

11.3 139.6 ✓

I.P.

11.96

 $\langle 160.91 \rangle$  ✓

1.19

 $\langle 138.95 \rangle$  ✓

A+50

1.2

138.9 ✓

A+80

4.5

136.6 ✓

 $\langle 100.14 \rangle$  ✓



Sketch Page 50.

Top Freeway  
Fill 30 high.

0-50	#5 King Freeway.		
0-40	Extend Pipe	+2.7	185.9
0-30	40' down to King with extension of King. 22' 06" ht	5.29	(178.40) ✓
+25		4.3	179.4 ✓ 8" Tree on
+60		5.0	178.7 ✓
0+91.3- Bot. 9.35		1.24	(174.34) ✓
1+00		2.2	173.4 ✓
1+15		5.8	169.8 ✓
1+40		8.7	166.9 ✓
1.P.	12.84	(175.58) ✓	0.31 (162.74) ✓
1+75		0.6	162.4 ✓
2+06		5.4	157.6 ✓
		(163.05) ✓	

186.58  
2.09  
184.51  
6.12  
178.39

162.4 155.4  
132 20.2  
R.9 R.14  
Dist



Ties at 25+33.45 (P. 17)

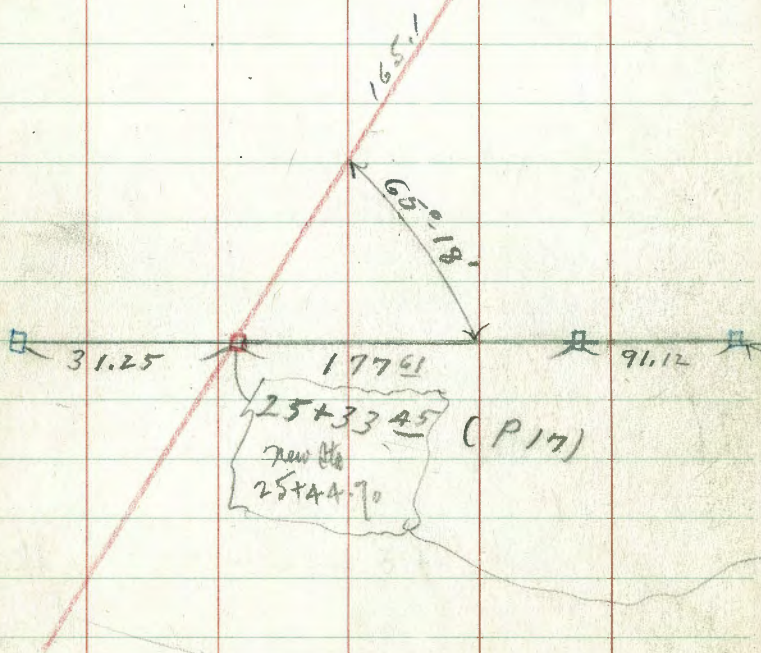
12-13-49

- = Fd. L+T.
- = Fd. 1/2 Hub
- = 30' Hub + dist

To Alvarado  
Canyon  
line.

Fanco line

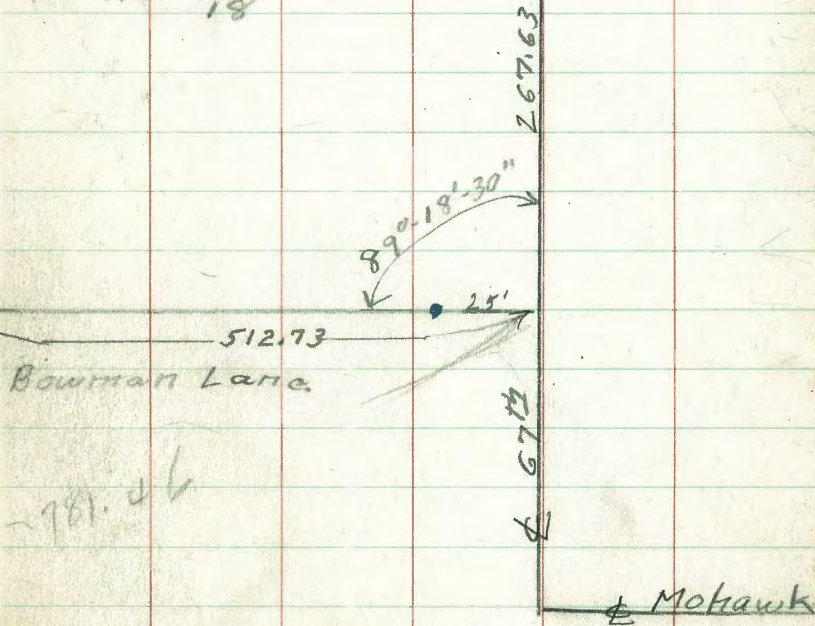
XXXXX  
 23+68 =  
 new Sta. 23+79.45



75

See La Mesa Colony  
tie points

Pub T.P.  $\frac{29}{18}$



781.46













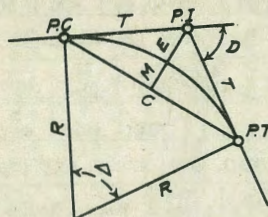






# DIETZGEN'S RAILROAD CURVE AND REDUCTION TABLES

Copyright, 1914, by Eugene Dietzgen Co., New York City



### CURVE FORMULAS

Radius= $R = \frac{50}{\sin \frac{D}{2}}$  (1) Degree of Curve= $D$  and  $\sin \frac{D}{2} = \frac{50}{R}$  (2)

Tangent= $T = R \tan \frac{\Delta}{2}$  (3) Length of Curve= $L = 100 \frac{\Delta}{D}$  (4)

Middle ordinate= $M = R(1 - \cos \frac{\Delta}{2})$  (5)  $= R \text{vers} \frac{\Delta}{2}$  (6)

External= $E = T \tan \frac{\Delta}{4}$  (7)  $= R + \cos \frac{\Delta}{2} - R$  (8)  $= R \text{exsec} \frac{\Delta}{2}$  (9)

Long Chord= $C = 2 R \sin \frac{\Delta}{2}$  (10)  $\Delta =$  Central Angle

### EXPLANATION AND USE OF TABLES

**Stations.**—Given P. I.=Sta. 161+60.35 to find Sta. of P. C. and P. T.  $\Delta=62^\circ 10'$   $D=8^\circ 20'$ . From Table IV for  $1^\circ$  curve  $T=3454.1$  and  $\div 8\frac{1}{2}=414.49$  ft. From Table V correction=.36 or  $T=414.85$  ft. P. C.=Sta. P.I.- $T=157+45.50$ . Also from (4)  $L=746.00$  and P. T.=Sta. P. C.+ $L=164+91.50$ .

**Offsets.**—Tangent offsets vary (approximately) directly with  $D$  and with square of the distance. Thus tangent offset for Sta. 158 on above curve is 2.16 ft. found as follows. From Table III tangent offset for 100 ft.=7.27 ft. Distance= $158 - \text{Sta. P. C.} = 54.50$ , hence offset= $7.27 (54.50 + 100)^2 = 2.16$  ft. Also square of any distance divided by twice the radius equals (approximately) the distance from tangent to curve. Thus  $(54.50)^2 \div (2 \times 688.26) = 2.16$  ft.

**Deflections.**—Deflection angle= $\frac{1}{2} D$  for 100 ft.,  $\frac{1}{4} D$  for 50 ft., etc. For  $c$  ft.=(in minutes)  $.3 \times C \times D^\circ$  or=defl. for 1 ft. from Table III  $\times C$ . For Sta. 158 of above curve= $.3 \times 54.5 \times 8\frac{1}{2} = 136.2'$  or  $2^\circ 16.2'$ , or= $2.50 \times 54.5 = 136.2'$  from Table III. For Sta. 159 deflection angle= $2^\circ 16.2' + 8^\circ 20' + 2 = 6^\circ 26.2'$ , etc.

**Externals.**—May be found in similar manner to tangents. Thus  $E$  for curve above is 115.37. For from Table IV for  $1^\circ$  curve  $E=960.6$  for  $8^\circ 20' = 960.6 \div 8\frac{1}{2} = 115.27$  and from Table V correction=.10 or  $E=115.37$  ft. Or suppose  $\Delta=32^\circ$  and  $E$  is measured and found to be 42 ft. What is  $D$ ? From Table IV  $E=230.9$  and  $\div 42 = 5.5$  or  $D=5^\circ 30'$ .



5 - 4321 70109  
 431 26.00  
 74 19  
 14389

508 + 59.54

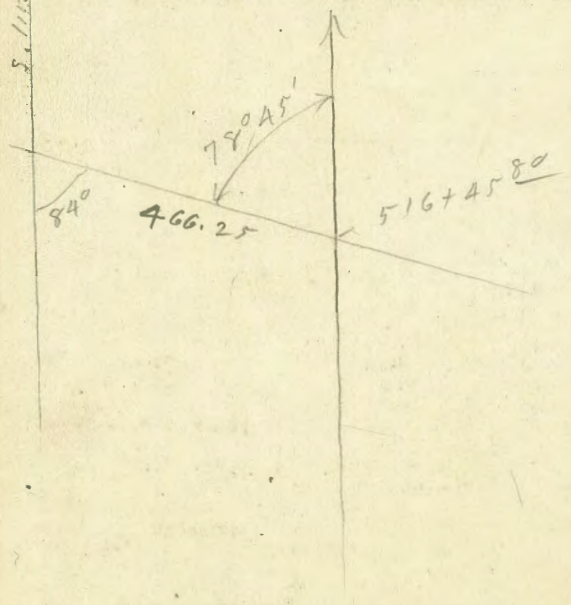
1570

4384

43.00

97.04

S. line Highway



2834  
53  
20

**DISTANCES FROM CENTER OF ROADWAY FOR  
CROSS-SECTIONING.**

Roadway 16 feet wide. Side Slopes 1 on 1½  
For Single Track Embankment.

H	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	H
0	8.0	8.2	8.3	8.5	8.6	8.8	8.9	9.1	9.2	9.4	0
1	9.5	9.7	9.8	10.0	10.1	10.3	10.4	10.6	10.7	10.9	1
2	11.0	11.2	11.3	11.5	11.6	11.8	11.9	12.1	12.2	12.4	2
3	12.5	12.7	12.8	13.0	13.1	13.3	13.4	13.6	13.7	13.9	3
4	14.0	14.2	14.3	14.5	14.6	14.8	14.9	15.1	15.2	15.4	4
5	15.5	15.7	15.8	16.0	16.1	16.3	16.4	16.6	16.7	16.9	5
6	17.0	17.2	17.3	17.5	17.6	17.8	17.9	18.1	18.2	18.4	6
7	18.5	18.7	18.8	19.0	19.1	19.3	19.4	19.6	19.7	19.9	7
8	20.0	20.2	20.3	20.5	20.6	20.8	20.9	21.1	21.2	21.4	8
9	21.5	21.7	21.8	22.0	22.1	22.3	22.4	22.6	22.7	22.9	9
10	23.0	23.2	23.3	23.5	23.6	23.8	23.9	24.1	24.2	24.4	10
11	24.5	24.7	24.8	25.0	25.1	25.3	25.4	25.6	25.7	25.9	11
12	26.0	26.2	26.3	26.5	26.6	26.8	26.9	27.1	27.2	27.4	12
13	27.5	27.7	27.8	28.0	28.1	28.3	28.4	28.6	28.7	28.9	13
14	29.0	29.2	29.3	29.5	29.6	29.8	29.9	30.1	30.2	30.4	14
15	30.5	30.7	30.8	31.0	31.1	31.3	31.4	31.6	31.7	31.9	15
16	32.0	32.2	32.3	32.5	32.6	32.8	32.9	33.1	33.2	33.4	16
17	33.5	33.7	33.8	34.0	34.1	34.3	34.4	34.6	34.7	34.9	17
18	35.0	35.2	35.3	35.5	35.6	35.8	35.9	36.1	36.2	36.4	18
19	36.5	36.7	36.8	37.0	37.1	37.3	37.4	37.6	37.7	37.9	19
20	38.0	38.2	38.3	38.5	38.6	38.8	38.9	39.1	39.2	39.4	20
21	39.5	39.7	39.8	40.0	40.1	40.3	40.4	40.6	40.7	40.9	21
22	41.0	41.2	41.3	41.5	41.6	41.8	41.9	42.1	42.2	42.4	22
23	42.5	42.7	42.8	43.0	43.1	43.3	43.4	43.6	43.7	43.9	23
24	44.0	44.2	44.3	44.5	44.6	44.8	44.9	45.1	45.2	45.4	24
25	45.5	45.7	45.8	46.0	46.1	46.3	46.4	46.6	46.7	46.9	25
26	47.0	47.2	47.3	47.5	47.6	47.8	47.9	48.1	48.2	48.4	26
27	48.5	48.7	48.8	49.0	49.1	49.3	49.4	49.6	49.7	49.9	27
28	50.0	50.2	50.3	50.5	50.6	50.8	50.9	51.1	51.2	51.4	28
29	51.5	51.7	51.8	52.0	52.1	52.3	52.4	52.6	52.7	52.9	29
30	53.0	53.2	53.3	53.5	53.6	53.8	53.9	54.1	54.2	54.4	30
31	54.5	54.7	54.8	55.0	55.1	55.3	55.4	55.6	55.7	55.9	31
32	56.0	56.2	56.3	56.5	56.6	56.8	56.9	57.1	57.2	57.4	32
33	57.5	57.7	57.8	58.0	58.1	58.3	58.4	58.6	58.7	58.9	33
34	59.0	59.2	59.3	59.5	59.6	59.8	59.9	60.1	60.2	60.4	34
35	60.5	60.7	60.8	61.0	61.1	61.3	61.4	61.6	61.7	61.9	35
36	62.0	62.2	62.3	62.5	62.6	62.8	62.9	63.1	63.2	63.4	36
37	63.5	63.7	63.8	64.0	64.1	64.3	64.4	64.6	64.7	64.9	37
38	65.0	65.2	65.3	65.5	65.6	65.8	65.9	66.1	66.2	66.4	38
39	66.5	66.7	66.8	67.0	67.1	67.3	67.4	67.6	67.7	67.9	39
40	68.0	68.2	68.3	68.5	68.6	68.8	68.9	69.1	69.2	69.4	40

Example—If point is 22.6 ft. above grade, how far should it be from center line to be a slope stake point? Ans. from Table 41.9. For same slopes but other widths of roadbed correct above figures by one-half difference in width of roadbed; thus in example above for 20 ft. roadbed distance will be 41.9 + (20—16) \* 2 or 2 ft. added to 41.9 = 43.9. For slopes of 1 on 1 see inside of front cover.