

311

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FIELD BOOK

1907

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*Morena Reservoir Dam and
Spillway and Safe Duty
Enlargement.*

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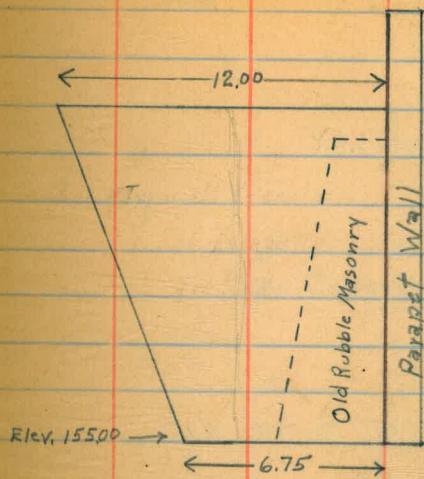
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Morena Reservoir Dam 9 Spillway 9 S.D. Engr. 1/7/30

X-sections for Excavations along Parapet Wall

Benham
Dewing
Moore

Sta	+	-	Elev.	T.P.
	958	168.20	158.62	T.P.
		1.77	166.43	B.M. Bolt on South Plaster



Excavation
Sq. FT. Cu. FT.

Note - Excavation quantities given include old rubble masonry. To obtain correct amount of excavation deduct cu yds. of old rubble masonry as obtained pages 7-10.

Notes copied from BK #310 pages 1-4
W.P.P.

Typical X-section of Excavation for New Rubble Masonry Wall.

0+00		1640	0	0
		1636	82.50	152.63
+032		1636	82.50	2157.39
+30 Angle Pt.		1638	81.56	122.34
+31 5/8 Diagonal line = 0+30 Parapet Wall		1638	81.56	122.34

Sta + K - Elev.

168.20

Excavation
Sq. Ft. Cu. Ft.

1+75			163 ¹ 51 12	163 ^E 42 0	√ 79.22	√ 1986.38
2+00			163 ² 50 12	163 ^E 42 0	√ 79.69	√ 1974.63
+25			163 ² 50 12	163 ⁵ 42 0	√ 78.28	√ 1992.13
+50			163 ⁴ 48 12	163 ² 42 0	√ 81.09	√ 2015.50
+75			163 ² 42 12	163 ⁸ 42 0	√ 80.15	√ 1997.88
3+00			163 ² 50 12	163 ⁸ 42 0	√ 79.68	√ 2009.63
+25			163 ⁵ 42 12	163 ⁸ 42 0	√ 81.09	√ 2027.25
+50			163 ³ 42 12	164 ² 42 0	√ 81.09	√ 2021.50

Sta	+	π	-	Elev.	Excavation		
					Sq FT	Cu. FT.	
3+75		168.20			$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1642}{43}$ $\frac{0}{0}$	✓ 80.63 ✓ 2021.50
4+00					$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1641}{41}$ $\frac{0}{0}$	✓ 81.09 2015.50 2015.15
+25					$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1639}{43}$ $\frac{0}{0}$	✓ 80.15 ✓ 1997.88
+50					$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1638}{43}$ $\frac{0}{0}$	✓ 79.68 ✓ 1997.88
+75					$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1639}{43}$ $\frac{0}{0}$	✓ 80.15 ✓ 1997.88
5+00					$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1640}{43}$ $\frac{0}{0}$	✓ 79.68 ✓ 1986.25
+25					$\frac{1634}{48}$ $\frac{12}{0}$	$\frac{1635}{47}$ $\frac{0}{0}$	✓ 79.22 ✓ 592.39
+32.5	BC.				$\frac{1632}{50}$ $\frac{12}{0}$	$\frac{1636}{45}$ $\frac{0}{0}$	✓ 78.75 ✓ 565.31

Sta	+	π	-	Elev.
		168.20		
5+39 ⁷ ±				
5+48 [±]				
5+57 [±]				
5+65 [±]				
5+75 [±]				
		1.66		166.54
T.P.		9.22		158.98
	0.76	159.74		
		6.89		152.85

B.M. on Bolt
on North Pileator

old Elev.
152.84

on rock East end
spillway

Excavation
Sq. Ft. Cu. Ft.

1632 52 12	1635 42 0	√ 78.28
		√ 672.90
1632 52 12	1631 51 0	√ 76.41
		√ 656.59
1629 53 12	1630 52 0	√ 74.53
		√ 638.18
1626 56 12	1628 54 0	√ 72.18
		√ 657.57
1626 55 12	1625 52 0	√ 70.77

Total 45806.23
45,806.58[✓]

cu. ft.
45,806.23 = 1696.53 cu. yds. excavation

Computed W. G. M. B. Chkd. 4/30/20 - H.D.W.
Checked B. F. M. 1696.54
279.84[✓]

Total cu. yds. to parapet wall 1696.53 1416.72[✓]
Total old rubble wall (see pages 7-10) 279.84
(see next page.) Total excavation 1416.69 cu. yds.

54750

5479

Excavation
57 ft. cu. yds.

Corrected Total of Excavation 11416.72

70.77

10.48

70.77

11427.20

Extra old Rubble Wall ^(see page 10) 3.01

Grand Total Excavation. 11424.19

Computed W.M.B.

Note:-

Chkd. - 5/29/30 - H.D.W.

Xsection for quantities of old Rubble Masonry along
Parapet of Dam

4/15/30
Benham
Drawing
Mure

B.M. Bottom
South Phase

Sta	+	π	-	Elev.
	0.75	167.18		166.43

0+00

+25

+30

+50

+75

1+00

+25

+50

Notes copied from BK #310
Pages 18-20

0 = Edge Parapet

Not Plotted
or Computed

155 ⁰	163 ⁸	163 ⁸
12	3 [±]	3 [±]
	1 [±]	0

155 ⁰	162 ⁵	162 ⁵
2 [±]	12	0

155 ⁰	162 ²	162 ²
27	5 ⁰	5 ⁰
	2	0

155 ⁰	162 ²	162 ²
22	5 ⁰	5 ⁰
	1 [±]	0

155 ⁰	161 ⁹	161 ⁹
23	5 ³	5 ³
	1 [±]	0

155 ⁰	161 ⁵	161 ⁵
22	5 ²	5 ²
	1 [±]	0

155 ⁰	161 ⁵	161 ⁵
22	5 ²	5 ²
	1 [±]	0

155 ⁰	161 ³	161 ³
26	6 ⁰	6 ⁰
	1 [±]	0

Old Rubble Masonry Wall
Sq. Ft. Co. Yds.

1540

17.95

16.92

12.27

1620

14.05

14.15

12.56

13.00

11.90

12.68

12.32

13.95

12.73

Sta + T - E/cv.
 167.18

Old Rubble Masonry Wall
 Sq FT Co Yds.

1+75

1550
23
 1612
52
20
 1612
52
0

13.55

11.50

2+00

1550
12
 1612
61
18
 1612
61
0

11.29

10.60

+25

1550
20
 1612
61
18
 1612
61
0

11.59

11.54

+50

1550
23
 1612
60
20
 1612
60
0

13.33

12.06

+75

1550
22
 1612
60
19
 1612
60
0

12.71

11.30

3+00

1550
21
 1612
62
18
 1612
62
0

11.70

11.15

+25

1550
24
 1602
63
18
 1602
63
0

12.39

11.74

+50

1550
22
 1602
62
20
 1602
62
0

12.98

10.83

Sta + π - Elev

59 ft. Cu. Yds.

3+75

1550
18

1613
52
15

1613
52
0

✓ 10.40

✓ 9.98

4+00

1550
20

1613
60
15

1613
60
0

✓ 11.16

10.35 ✓
10.37

+25

1550
20

1614
58
15

1614
58
0

✓ 11.20

✓ 10.79

+50

1550
22

1612
60
12

1612
60
0

✓ 12.09

✓ 11.57

+75

1550
23

1613
52
12

1613
52
0

✓ 12.92

✓ 11.62

5+00

1550
22

1614
61
15

1614
61
0

✓ 12.20

✓ 10.54

+25

1550
12

1616
15

1616
0

✓ 10.56

✓ 3.47

+325

1550
20

1628
12

1628
0

✓ 14.43

✓ 5.12

Sta	+	π	-	Elev.		Old Rubble Masonry Wall			
		167.18				Sq. FT	Cu Yds.		
5+392					$\begin{matrix} 1550 \\ 34 \end{matrix}$	$\begin{matrix} 1625 \\ 30 \end{matrix}$	$\begin{matrix} 1625 \\ 0 \end{matrix}$	$\sqrt{24.00}$	$\sqrt{7.54}$
5+484					$\begin{matrix} 1550 \\ 92 \end{matrix}$	$\begin{matrix} 1615 \\ 30 \end{matrix}$	$\begin{matrix} 1615 \\ 0 \end{matrix}$	$\sqrt{22.78}$	$\sqrt{6.01}$
5+574					$\begin{matrix} 1550 \\ 29 \end{matrix}$	$\begin{matrix} 1615 \\ 20 \end{matrix}$	$\begin{matrix} 1615 \\ 0 \end{matrix}$	$\sqrt{14.52}$	$\sqrt{4.41}$
5+658					$\begin{matrix} 1550 \\ 23 \end{matrix}$	$\begin{matrix} 1615 \\ 12 \end{matrix}$	$\begin{matrix} 1615 \\ 0 \end{matrix}$	$\sqrt{12.87}$	$\sqrt{3.92}$
5+750					$\begin{matrix} 1550 \\ 12 \end{matrix}$	$\begin{matrix} 1620 \\ 12 \end{matrix}$	$\begin{matrix} 1620 \\ 0 \end{matrix}$	$\sqrt{10.15}$	
								Total	$\sqrt{279.84}$
									279.84
5+830								Computed E.H.D.	10.15
								Checked W.M.B.	3.01
								Chkd - 4/30/30 - HDW	<u>282.83</u>
									W.M.B. 5-26-30.

Elevation
Top of
New Rubble Masonry Wall.
H.I. - S Elev.

	H.I.	S	Elev.
0+00	166.00	6.6	159.4
0+25		7.0	159.0
0+30		6.5	159.5
0+50		7.0	159.0
0+75		6.5	159.5
1+00		7.1	158.9
1+25		7.3	158.7
1+50		6.8	159.2
1+75		7.0	159.0
2+00		7.3	158.7
2+25		7.5	158.5
2+50		6.7	159.3
2+75		5.1	160.9
3+0.0		5.5	160.5
3+25		7.6	158.4
3+50		7.4	158.6
3+75		8.4	157.6
4+0.0		8.3	157.7
4+25		8.7	157.3
4+50		8.6	157.4

H.I. 166 is the Elev. Top of old Parapet Wall.

Note: - Elevations taken for April estimate only.

Notes copied from Book 310 Page 35-37

Final Record.
Drill ^{on} Holes in Spillway.

Apr 22, 1930
" 23, 12
W.M. Borham.

Pier No	15' Hole	10' Hole	5' Hole	4' Hole	4' Hole	
0+00	1	7.0	10.0	5.0	4.0	4.0
0+16 ²⁵	2	15.0	10.0	5.0	4.0	4.0
0+30 ⁵⁰	3	15.0	10.0	5.0	4.0	4.0
0+44 ⁷⁵	4	15.0	10.0	5.0	4.0	4.0
0+59 ⁰⁰	5	15.0	10.0	5.0	4.0	4.0
0+73 ²⁵	6	15.0	10.0	5.0	4.0	4.0
0+87 ⁵⁰	7	15.0	10.0	5.0	4.0	4.0
1+01 ⁷⁵	8	14.0	10.0	5.0	4.0	4.0
1+16 ⁰⁰	9	15.0	10.0	5.0	4.0	4.0
1+30 ²⁵	10	15.0	10.0	5.0	4.0	4.0
1+44 ⁵⁰	11	15.0	10.0	5.0	4.0	4.0
1+58 ⁷⁵	12	15.0	10.0	5.0	4.0	4.0
1+73 ⁰⁰	13	15.0	10.0	5.0	4.0	4.0
1+87 ²⁵	14	15.0	10.0	5.0	4.0	4.0
2+01 ⁵⁰	15	15.0	10.0	5.0	4.0	4.0
2+15 ⁷⁵	16	15.0	10.0	5.0	4.0	4.0
2+30 ⁰⁰	17	15.0	10.0	5.0	4.0	4.0
2+44 ²⁵	18	15.0	10.0	5.0	4.0	4.0
2+58 ⁵⁰	19	15.0	10.0	5.0	4.0	4.0
2+72 ⁷⁵	20	15.0	10.0	5.0	4.0	4.0
2+87 ⁰⁰	21	15.0	10.0	5.0	4.0	3.5
3+01 ²⁵	22	15.0	8.5	5.0	4.0	4.0
3+14 ⁵⁰	23	15.0	10.0	5.0	4.0	4.0
336 ft	228.5 ft	115 ft	92 ft	91.5 ft		

Next to Parapet Wall

Note - All holes for pier are
2" holes.

A.L.L.
10/2/30

863.0 ft - 2" Holes.

Struck Pocket

Computed W.M.B.

Checked 8/21/30

W.D.L.

N.E End of Spillway.

A.L.L.
10/2/30

Final Record

Drill Holes ^{on} in Parapet Wall.

Pilaster No	5' Hole	5' Hole	5' Hole	2' Hole	2' Hole
1	—	2.0	—	2.0	—
2	5.0	4.0	4.0	1.5	2.0
3	5.0	4.5	4.0	2.0	2.0
4	4.5	4.0	5.0	2.0	2.0
5	4.0	4.5	4.0	1.5	2.0
6	5.0	5.0	5.0	1.5	1.5
7	4.0	5.0	5.0	2.0	1.5
8	5.0	5.0	5.0	2.0	2.0
9	4.0	5.0	5.0	1.5	2.0
10	5.0	5.0	5.0	2.0	2.0
11	5.0	5.0	5.0	2.0	2.0
12	5.0	5.0	5.0	2.0	2.0
13	5.0	5.0	5.0	2.0	2.0
14	5.0	5.0	5.0	2.0	2.0
15	5.0	5.0	4.0	2.0	2.0
16	5.0	5.0	5.0	2.0	2.0
17	5.0	4.0	4.0	2.0	2.0
18	4.5	4.5	4.5	2.0	2.0
19	4.0	5.0	3.5	2.0	1.5
20	5.0	5.0	5.0	2.0	2.0
21	5.0	5.0	5.0	2.0	1.5
22	5.0	5.0	5.0	2.0	2.0
23	5.0	5.0	5.0	2.0	2.0
24	5.0	5.0	5.0	2.0	2.0
25	5.0	5.0	2.0	2.0	2.0

16.5

13

Next to cliff at south
end of parapet wall.

Notes All holes in pilasters
are 2" holes.

$$\frac{16.5}{10/2/20}$$

2" Holes = 503.0 ft.

Final Record
 Drill Holes ^{on} Parapet Wall.

Pilaster No	5'Hole	5'Hole	5'Hole	2'Hole	2'Hole
26	4.0	5.0	3.5	2.0	2.0
27	4.5	3.0	5.0	2.0	1.5
28	4.0	5.0	5.0	2.0	2.0
29	4.5	5.0	3.5	2.0	2.0
	132 ft	134.5 ft	127.0 ft	56 ft	53.5 ft

North End at Parapet Wall

132 ft
 134.5 "
 127.0
 56.0
 53.5

 503.0

W.D.L.
 10/2/30

Computed W.M.B.

Checked 8/21/30

W.D.L.

X-sections "A" Line Rd. South of Dam
Final Location

4/15/30

Bonham
Dewing
Moore

Left

Center

Right

15

Sta + X - Grade Elev. 1.89 168.32 166.43

0+00 167.00

+50 167.00

+86 Note excavation begins at this plus 167.00
0.34 167.98

+88 1216 180.14 167.00

+100 8% to 86X
167.96

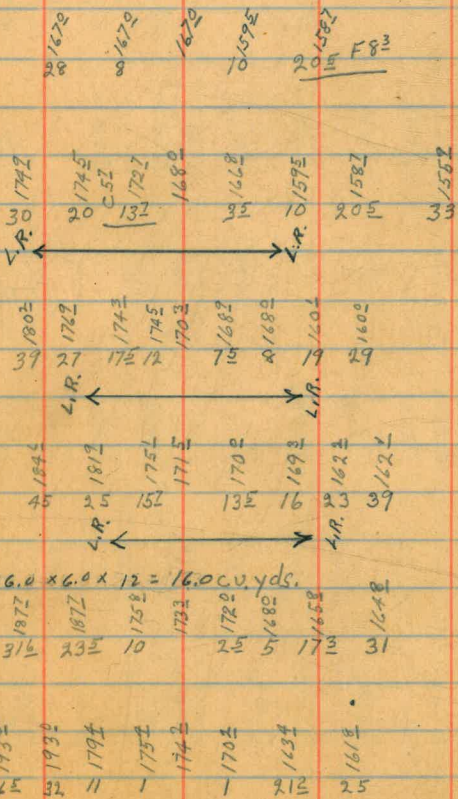
+25 169.96

+34.45 (Interpolated) 1.04 179.10

+50 5.45 184.55 171.96

+75 173.96

169.5
5.0
170.5
2.7
173.2
167.0
169.4
16.5
170.0
34.0
172.4
173.0
174.0
175.0
176.0
177.0
178.0
179.0
180.0
181.0
182.0
183.0
184.0
185.0
186.0
187.0
188.0
189.0
190.0
191.0
192.0
193.0
194.0
195.0
196.0
197.0
198.0
199.0
200.0



Sta + π - Grade Elev.

184.55

2+00

175.96

Isolated Boulders
10 cu. yds.

200	199	199	172	171	171	164
47	42	36	6	15	21	

+25

177.96

203	202	202	174	174	169	155
58	44	41	3	9	20	42

+40 P.C.

179.16

198	199	180	180	175	178	182
35	12	12	10	9	20	

+62

180.92

Isolated
Boulders. 60 cu. yds.

198	186	184	181	183	172
35	13	8	12	30	43

+84

182.68

198	174	195	182	182	172	204	174	188
35	23	20	12	5	10	19	19	50

3+06

184.44

198	192	199	200	205	212	214	215	212	212	212
35	30	25	24	24	24	13	26	33	33	70

+28

186.20

198	200	211	215	222	255	255	293
35	29		27	46	81	105	

+49 2/4

187.96

198	203	205	211	246	257	298
35	27	17		27	43	72

Left

±

Right

16

STA	+	π	-	Grade	Elev. T.P. on Pipe 211.48	P
3+75	2.35	<u>212.83</u>		189.96		2.01 2.03 2.05 2.12 2.22 2.39 2.62 2.88 2.96
4+00				191.96		2.17 2.20 2.23 2.31 2.46 2.56 2.68 2.80 2.92
4+25 ^L Angle Pt.				194.02		2.19 2.19 2.19 2.19 2.17 2.40 2.56 2.72 2.85
4+6				195.64		2.19 2.21 2.21 2.10 2.10 2.11 2.48 2.61 2.70
4+6				195.64		2.19 2.21 2.21 2.10 2.10 2.11 2.37 2.55 2.70
4+75				197.96		2.20 2.20 2.12 2.13 2.13 2.58 2.58 2.74
4+75 ^R Angle Pt.			X-sections	197.63 90° to back tangent		2.20 2.19 2.19 2.14 2.15 2.19 2.25 2.51
5+25				201.96		2.20 2.14 2.15 2.15 2.15 2.43 2.50

STA	+	π	-	Grade	Elev. T.P. on Pipe
	8.06	<u>219.54</u>			211.48

5+50				203.96	
------	--	--	--	--------	--

5+75				205.96	
------	--	--	--	--------	--

6+00				207.96	
------	--	--	--	--------	--

4/19/30

18

201	208	213	219	227	233
23	18	9	4	11	17
201	206	211	219	225	233
24	14	13	7	11	17
201	208	209	208	216	223
23	11	10	6	17	17

X-sections for May Estimate Rd South of Dam.
Bonham
Dewing
5/24/30

Sta + Π - Elev.
13.5 203.2 189.70

Sta	+	Π	-	Elev.	Σ	59-ft.	20 Cu.-ft.
4+00					203.2	508.0	17,125.0 ✓
					205 ⁵ +23 12		
					205 ^E +23		
					221 ² +18 ⁰ 33		
3+75					187 ^E 15 [±] 34	862.0	29,750.0 ✓
					193 ^E 9 [±] 28		
					193 ^E 9 [±] 16		
					197 ² 4 [±] 27		
					207 ^E +4 [±] 27		
3+50					190 ⁸ 12.4 35	1578.0	34,716.0 ✓
					191 ^E 11 ^E 13		
					195 ^E 8 ⁰		
					200 [±] 2 [±] 13		
3+28					190 ⁸ 12.4 35	1638.0	28,820.0 ✓
					190 ⁸ 12 [±] 6		
					195 [±] 7 [±]		
					190 [±] 2 [±] 13		
3+06					190 ⁷ 12 [±] 35	982.0	15,246.0 ✓
					190 [±] 12 [±] 35		
					190 [±] 12 [±] 35		
					194 [±] 8 [±] 11		
2+84					190 [±] 12 [±] 35	404.0	5,104.0 ✓
					190 [±] 12 [±] 35		
					194 [±] 8 [±] 26		
2+62					189 [±] 13 [±] 40	60.0	836.0 ✓
					189 [±] 13 [±] 40		
					182 [±] 20.5 6		
					181 [±] 22.1 18		
2+40					193 [±] 10 ⁰ 40	16.0	1,485.0 ✓
					193 [±] 10 ⁰ 7		
					185 [±] 17 [±]		
					172 [±] 30.5 "		

Sta	+	X	-	Elev.	Q	Sq. ft.	Co. ft.
		203.2					21
	0.0	190.1	1.3.1	190.1			
2+25					156.8 3.3 40	182.0	
					184.6 5.2 22	70.0	3,150.0
2+00					184.6 5.2 22	70.0	
					181.1 9.5 12	60.0	1,625.0
1+75			12.1	178.0	180.6 9.5 8		
	1.1	179.1			175.2 3.2 21	136.0	2,450.0
1+50					174.2 4.2 9		
					170.5 8.5	32.0	2,100.0
1+25							
					147.2 11.2	70.0	1,275.0
1+00							
					167.2 12.2	32.0	612.0
0+88							
					167.2 12.2	0.0	32.0
0+86							
							144,826.0

See Next Page

✓
144,326.0 cu.ft. = 5,345.4 cu.yds. ✓

Isolated Boulder Sta. 1+50 16.0 " "

Material not shown on

Xsections caused by slide

of April 28 1.5720 " "

6933.4 cu.yds.

Isolated Boulder Sta. 2+00 10.0 " "

" " Sta. 2+62 60.0 " "

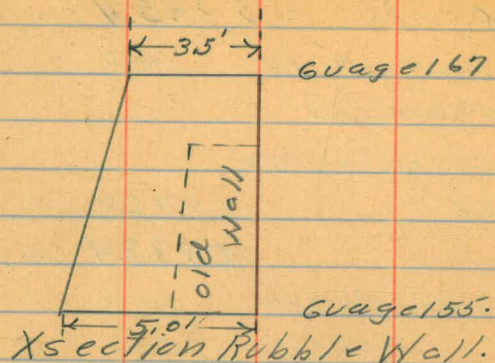
Total. ✓ 7,003.4 cu.yds.

Planimetered, Computed. W.M.B. 5/27/30.

Checked. Planimeter computation only by P.O.S. 5/29/30

✓ C.W. 5/29/30
W.W.

May Estimate
Rubble Wall.



X section Rubble Wall.

Total $\frac{1}{2}$ distance New Rubble Masonry
Wall 581 linear feet.

$$581 \times 12 \times \frac{3.5 + 5.0}{2} = 29.631 \text{ cu. ft.} = 1097.44 \text{ cu. yds.}$$

Old Rubble Wall {see page 10.}

Total

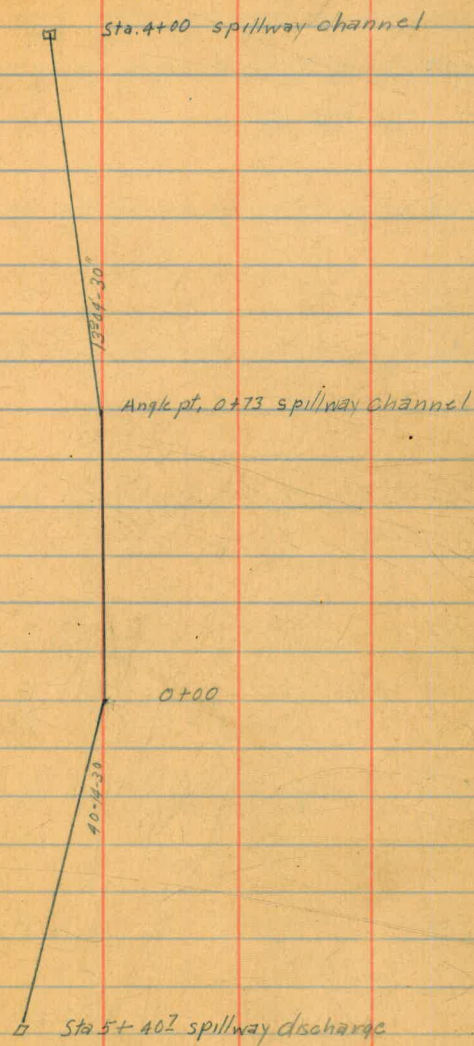
~~1097.44~~
~~282.83~~
~~814.61~~ cu. yds.

A.L.B.
10/2/30

X Sections Channel
Below Spillway.

24

Ties between spillway discharge + spillway channel



Sta Grade

0+00 On Diagonal Line

0+00 120.0

0+13 112.5

0+25 112.5

0+38

0+50

0+68

0+73 87.9 95.9 100.9 100.9 84.1 86.1 104.8 116.1 171.8 203.8
58 28 12 6 19 24 30 122 176

0+75 87.9 95.9 95.0 87.2 101.2 106.6 127.2 177.2 212.2
58 28 12 21 30 58 90 111 179

Base Line

117¹ 122¹ 124¹ 124¹ 126¹ 131¹ 156¹ 164¹ 241¹ 248¹
29 36 33 46 53 55 68 136 160
117¹ 118¹ 117¹ 117¹ 116¹ 116¹ 119¹ 122¹ 122¹ 123¹ 125¹ 127¹ 128¹ 129¹ 129¹
18 23 25 26 27 29 31 30 40 46 47 42 42 47 50
166¹ 229¹ 236¹
72 73 72

114¹ 115¹ 116¹ 118¹ 123¹ 127¹ 128¹ 130¹ 141¹ 158¹ 169¹ 175¹ 200¹ 223¹ 230¹
14 25 35 35 42 46 50 60 72 82 91 114 135 149

110¹ 113¹ 121¹ 128¹ 140¹ 161¹ 169¹ 171¹ 217¹ 225¹
29 32 50 63 82 94 114 135 176

103¹ 103¹ 108¹ 109¹ 127¹ 137¹ 137¹ 170¹ 173¹ 209¹ 219¹
10 13 31 30 40 44 112 135 175

93¹ 93¹ 101¹ 104¹ 108¹ 127¹ 129¹ 140¹ 175¹ 214¹
13 15 19 33 43 50 65 117 172

87¹ 87¹ 101¹ 116¹ 117¹ 135¹ 172¹ 179¹ 202¹ 206¹
17 22 38 50 65 120 143 160 174

Base Line

578

Base Line

20

0+82 ✓

87¹/₆₀ 95²/₂₆ 91¹/₁₂ 84¹/₁₂ 95⁵/₂₅ 94¹/₃₆ 102⁰/₈₀ 122²/₆₇ 133²/₈₁ 155⁸/₁₀₀ 176⁰/₁₁₀ 186²/₁₃₅ 200²/₁₆₂ 207¹/₁₇₄

1+00

92²/₆₆ 92²/₂₂ 97²/₁₂ 88⁰/₁₂ 86³/₂₉ 98¹/₅₀ 111⁴/₆₆ 122²/₇₅ 125²/₈₁ 152²/₁₀₅ 175⁷/₁₃₅ 184¹/₁₄₂

1+13 ✓

77¹/₆₇ 80²/₂₁ 80²/₁₂ 79²/₁₂ 82⁸/₂₃ 96¹/₃₀ 117²/₇₀ 127¹/₈₁ 128³/₈₆ 139³/₁₀₀ 151²/₁₁₅ 154²/₁₂₆

1+25

62²/₇₀ 68⁵/₁₇ 74¹/₁₄ 76¹/₁₄ 77²/₁₂ 89⁴/₄₇ 96³/₄₅ 99¹/₅₀ 127²/₈₀ 134²/₈₆ 142⁰/₉₂

1+50

55³/₆₅ 64¹/₃₇ 62²/₁₄ 71²/₁₄ 72³/₂₀ 92⁶/₅₀ 123¹/₁₁₂

1+63 ✓

49¹/₆₈ 47²/₃₈ 60¹/₁₆ 62¹/₁₆ 62²/₂₀ 76²/₄₂ 79⁵/₅₀ 119²/₁₀₄

1+75

42²/₇₀ 57²/₃₇ 54⁴/₁₈ 58²/₁₈ 66⁸/₂₇ 72⁰/₅₀ 115²/₉₄

1+85^{EE}

44⁶/₅₀ 50⁶/₃₇ 17¹/₁₀ 63⁵/₂₆ 67²/₅₀ 88³/₉₃

Base Line

579.

Base Line

2+00

$$\begin{array}{ccc} 31\frac{1}{2} & 37\frac{1}{2} & 47\frac{1}{2} & 45\frac{1}{2} \\ 62 & 32 & 16 & \end{array} \quad \begin{array}{ccc} 53\frac{1}{2} & 55\frac{1}{2} & 65\frac{1}{2} \\ 26 & 43 & 50 \end{array}$$

2+25

$$\begin{array}{ccc} 31\frac{1}{2} & 40\frac{1}{2} & 38\frac{1}{2} \\ 57 & 10 & 44 & 50 \end{array} \quad \begin{array}{ccc} 38\frac{1}{2} & 41\frac{1}{2} \\ 44 & 50 \end{array}$$

2+41

$$\begin{array}{ccc} 29\frac{1}{2} & 30\frac{1}{2} & 37\frac{1}{2} \\ 59\frac{1}{2} & 2 & \end{array} \quad \begin{array}{ccc} 34\frac{1}{2} \\ 50 \end{array}$$

2+50

$$\begin{array}{ccc} 13\frac{1}{2} & 22\frac{1}{2} & 15\frac{1}{2} & 18\frac{1}{2} & 26\frac{1}{2} & 16\frac{1}{2} \\ 35 & 10 & 26 & 40 & 50 \end{array}$$

2+62+0

$$\begin{array}{ccc} 12\frac{1}{2} & 3\frac{1}{2} & 12\frac{1}{2} & 9\frac{1}{2} \\ 27 & 22 & 50 \end{array}$$

2+75

$$\begin{array}{ccc} 10\frac{1}{2} & 7\frac{1}{2} & 1\frac{1}{2} & 7\frac{1}{2} & -0\frac{1}{2} \\ 18 & 27 & 45 & 50 \end{array}$$

3+00

$$\begin{array}{ccc} -0\frac{1}{2} & -7\frac{1}{2} & -6\frac{1}{2} & -14\frac{1}{2} \\ 76 & 40 & 50 \end{array}$$

3+25

Base Line

$$\begin{array}{ccc} -22\frac{1}{2} & -21\frac{1}{2} & -21\frac{1}{2} \\ 26 & 26 & 50 \end{array}$$

Isolated boulder 22x15x27 = 480 cu yds.

Base Line

Sta.

3+50

-25¹-31¹
50

3+58

-29¹-37⁶
50Isolated boulder 5'x10'x16' = 29⁶ cu. yds.

3+75

-38³-46⁴
50

4+00

-23²-45⁰
3-47³
50

Base Line

4/21/38
 X-sections Road South of Dam for June Estimate
 Bonham
 Dewing

⊥

29

Sta	+	κ	-	Elev.								
	16.00	176.00		166.00								
0+86						167 ⁰ ₁₆	167 ⁰	167 ⁰				
0+88						172 ⁰ ₂₀	167 ⁰ ₁₁	167 ⁰ ₁₁	167 ⁰ ₁₁			
1+00						175 ⁰ ₂₂	169 ⁰ ₁₁	168 ⁰ ₁₁	168 ⁰ ₁₂			
+25						176 ⁰ ₂₂	171 ⁰ ₈	170 ⁰ ₈	170 ⁰ ₁₆			
+50						187 ⁰ ₃₁	178 ⁰ ₂₂	177 ⁰ ₁₅	172 ⁰ ₁₀	172 ⁰ ₁₀		
+75	710	182.70	640	175.60		193 ⁰ ₃₇	182 ⁰ ₂₄	181 ⁰ ₁₉	174 ⁰ ₈	174 ⁰ ₈	173 ⁰ ₈	
2+00						199 ⁰ ₄₂	188 ⁰ ₃₁	184 ⁰ ₂₅	176 ⁰ ₇	176 ⁰ ₇	175 ⁰ ₁₂	
+25						202 ⁰ ₄₃	191 ⁰ ₃₃	187 ⁰ ₂₄	178 ⁰ ₇	177 ⁰ ₁₃	177 ⁰ ₁₃	
2+40 P.C.	471	196.11		191.40		190 ⁰ ₃₅	189 ⁰ ₂₄	188 ⁰ ₈	178 ⁰ ₁₃	178 ⁰ ₁₃		
2+62						190 ⁰ ₃₅	190 ⁰ ₂₄	180 ⁰ ₆	177 ⁰ ₁₀	179 ⁰ ₁₆	181 ⁰ ₁₆	185 ⁰ ₂₁
2+84						190 ⁰ ₃₅	191 ⁰ ₂₁	182 ⁰ ₈	181 ⁰ ₈	182 ⁰ ₈	208 ⁰ ₄₃	
3+05						190 ⁰ ₃₅	192 ⁰ ₂₀	185 ⁰ ₉	184 ⁰ ₉	186 ⁰ ₁₀	218 ⁰ ₂₅	225 ⁰ ₂₆

⊥

Sta + π - Elev.
196.11

3+28					190 ² ₃₅	191 ⁵ ₁₇	186 [±] ₉	186 [±]	188 ² ₈	197 ² ₁₄	219 ⁶ ₁₄	263 ² ₄₆	
3+49 ²⁷ PT					190 ² ₃₅	192 ² ₁₆	188 [±] ₈	188 ²	190 [±] ₈	178 ² ₁₁	200 [±] ₁₄	222 ² ₁₂	258 ² ₄₃
3+75	943	200.83		191.40	201 ² ₂₄	196 [±] ₂₁	190 ² ₁₀	190 [±]	196 [±] ₃	204 [±] ₂₄	221 [±] ₃₂	227 ² ₃₄	
4+00					200 [±] ₂₂	192 [±] ₁₃	191 ⁸	192 ² ₂	198 [±] ₁₆	212 ² ₁₃	218 ² ₂₃	221 ² ₃₃	
4+25 ²⁷					197 ⁸ ₂₀	193 ⁵ ₁₆	193 ²	194 [±] ₃	213 ⁸ ₂₇	235 ² ₃₇	241 [±] ₄₁		
4+41					194 [±] ₃₃	195 ²	210 ⁸ ₃₁	241 [±] ₄₁					
4+46					194 [±] ₃₃	195 ²	210 ⁸ ₃₁	237 [±] ₃₁	256 ² ₃₅				
4+75					196 ⁸ ₂₄	197 ²	202 [±] ₇	223 [±] ₂₃	237 ² ₂₃				
4+95 ⁸⁴	1012	210.01	0.24	199.89	198 [±] ₁₉	198 [±]	198 [±] ₆	200 ² ₁₁	219 ² ₁₄				
5+25					200 [±] ₁₉	201 [±]	201 [±] ₄	204 [±] ₇	Vertical Bank				
5+50					204 ² ₁₆	204 [±]	204 [±] ₂	211 ² ₆	Vertical Bank				
5+75					205 ²	205 ² ₊	Vertical Bank						

Sta	+	π	-	Elev.
		210.01		
6+00				
6+05				

206 ⁶ ₁₇	207 ⁴	207 ⁶ ₆
--------------------------------	------------------	-------------------------------

208 ⁴ ₈	208 ⁶	208 ⁴ ₈
-------------------------------	------------------	-------------------------------

Note:-

See page 32-33 for quantities.

Notes -
From pages
29-31.

32

Sta.	Sq. ft.	Cu. ft.
0+86	0.0	
		60.0
0+88	60.0	
		792.0
1+00	72.0	
		1,450.0
1+25	44.0	
		1,925.0
1+50	110.0	
		2,850.0
1+75	118.0	
		3,375.0
2+00	152.0	
		3,200.0
2+25	104.0	
		1,035.0
P.C. 2+40.	34.0	
		2,156.0
2+62	162.0	
		8,844.0
2+84	642.0	
		19,954.0
3+06	1,172.0	
		28,116.0
3+28	1,384.0	
		29,216.0

Sta. P.T.	59.ft. Cu.ft.	
3+50	1,272.0	30,200.0
3+75	1,144.0	25,075.0
4+00	862.0	19,300.7
4+25.7	640.0	14,920.5
4+46.	830.0	0.0
4+46.	952.0	20,735.0
4+75	478.0	10,420.8
4+95.8	524.0	13,110.8
5+25	374.0	7,025.0
5+50	188.0	3,650.0
5+75	104.0	1,925.0
6+00	50.0	125.0
6+05.	0.0	

Computed V.M.B. 6-28-30
Checked

X sections	9,239.0 cu. yds.
Isolated Boulders.	86.0 " "
Slide	1,572.0 " "
	<hr/>
	10,897.0 cu. yds.

249,460.8 cu. ft. = 9,239 cu. yds.

Spillway Discharge
Original X-sections

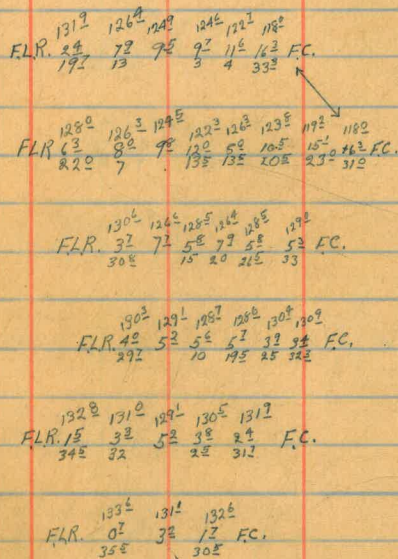
4/8/30
Bonham
Dewing
Moore

Sta	+	π	-	Elev.	
	3.35	156.19		152.84	B.M. East End Spillway
T.P.			12.54	143.65	
	1.58	145.23			
T.P.			11.86	133.37	
	0.96	134.33			
			5.58	128.75	T.P. set at W. end Spillway
0+0.0					on split of angle
0+10.3					at right angles
0+25					
0+50					
0+75					
1+00					
	8.64	142.01	0.96	133.37	

FLR. = Face Ledge Rock

Base

F.C. = Face Concrete



✓ ALL.

Sta + π - Elev

14201 ✓

1+25

133² 134⁵ 133² 132¹ 131² 133¹
 F.L.R. 8² 7⁵ 5¹ 7² 9⁶ 8³ F.C.
 36⁵ 27 4 20 30

1+38

135¹ 135¹ 133⁶ 134⁸ 134² 134¹ 133³ 134⁸
 F.L.R. 6² 6² 8⁴ 7² 7² 7² 8² 7² F.C.
 36⁵ 25 15 7 10 21 27

1+50

130² 134² 133² 134⁵
 F.L.R. 5² 7² 8⁶ 7⁵ F.C.
 37 12 27

1+65

137⁵ 135⁷ 133⁵ 133² 135²
 A.S. 6² 8⁵ 8⁴ 6² F.C.
 38⁵ 21 17 27

1+75

136² 133² 134² 133² 133² 135²
 F.L.R. 5² 8² 7² 8² 8² 6¹ F.C.
 38⁵ 19 22 15 25

1+85

136⁵ 134⁴ 135² 134² 134² 136²
 F.L.R. 5² 7² 6⁵ 7² 8² 5² F.C.
 39⁵ 15 7 5 28⁵

2+00

137⁵ 136² 136⁰ 136² 136² 136¹ 136¹ 135² 137¹ 137²
 F.L.R. 4⁵ 5⁸ 6² 5¹ 5² 3² 3² 4² 4² F.C.
 41 33 20⁵ 6 13 21 24⁵ 27⁵

2+10

138² 132² 136² 135² 136² 137² 137²
 F.L.R. 3² 5⁸ 6² 6² 5² 4⁵ 4² F.C.
 43⁵ 35 22 13 15 27

2+19

139¹ 136² 134² 135² 137⁵ 137² 137² 136² 136² 137⁵
 F.L.R. 2² 6² 7² 6² 4² 4² 3² 4² 4² F.C.
 47⁵ 43⁵ 31 17 7 13 14 27

2+31

Slush bottom on right side

141⁵ 135² 134² 135² 136² 137²
 F.L.R. 0⁵ 6² 7² 7² 5² 4² Tac Concrete Spillway
 52 41 33 17 22

2+50

138² 135² 135² 135² 136² 137²
 F.L.R. 3⁵ 6² 6² 6² 5² 4² T.C.S.
 46⁵ 31 20⁵ 8 20

2+75

137¹ 137¹ 137¹ 135² 136² 138² 138²
 F.L.R. 2³ 4² 5² 6² 6² 5² 4² 3² T.C.S.
 50 46 40 36 26 12 18

Sta. + H.I. - Elev.

3+00 142.01

3+25

3+50

12.75 154.50
0.26 141.75

3+66

3+75

T.L.R. 173.5 157.0
55 49

3+75

T.L.R. F.L.R.
173.5 157.0 165.0 162.5
55 49 47 43

3+97

226.0 216.5 189.9
97 84 73

3+97

226.0 216.5 189.9 163.6 160.8 155.2 152.8
97.0 84 73 40 37 32 26

4+13

227.0 217.5 196.0 180.0
97 87 68.0 61

4+13

226.6 222.3 199.0
96 90 70

4+25

224.0 211.5 198.5
95 80 70

4+25

225.0 213.0 198.5
95 80 70

Base.

138⁰ 137² 137⁰ 137² 137⁵
F.L.R. 40 48 38 27 15 T.C.S.
47.5 31 16 16

F.L.R.
40.2 138.6 139.0 140.1 140.1 140.6
1.8 3.4 3.0 1.9 1.8 1.4 T.S.
51 39 31.0 27.5 14.3

F.L.R.
145.2 143.0 141.1 140.8 141.0
+3.2 + 1.0 0.9 1.2 1.0 T.S.
49 40.0 25 12.2

F.L.R. 153.3 144.0 141.5 141.7
1.2 10.5 13.0 12.8 T.S.
49.2 21.5 11.2

154.5 153.8 149.1 143.4 143.4 141.9 141.8
5.4 11.1 11.1 12.6 12.7 T.S.
42 37 22.0 19.5 16 10.7

149.1 143.4 143.4 141.9 141.8
5.4 11.1 11.1 12.6 12.7 T.S.
42 37 22 19.5 16 10.7

143.9 144.5 143.4 142.7
10.6 10.0 11.1 11.8 T.S.
40 36 32 13.5 7.5 8.6

143.9 144.5 143.4 142.7
10.6 10.0 11.1 11.8 T.S.
25 19 16 13.5 7.5 8.6

147.9 154.6 144.3 143.9 143.3
6.6 10.2 10.6 11.2 T.S.
58.5 36 26 24.5 17 14.5 7.5

147.9 154.6 144.3 143.9 143.3
6.6 8.7 10.2 10.6 11.2 T.S.
60 58.5 36 24.5 18.5 14.5 7.5

149.9 144.3 143.8 143.9
4.6 10.2 10.7 10.6 T.S.
50 41 30 26 13 6.5

150.0 149.9 144.3 143.8 143.9
4.5 10.2 10.7 10.6 T.S.
50 41 30 26 13 6.5

A.L.L.
7/24/30

4+50

154.50

4+75

5+00

T.L.R. $\frac{157.4}{21}$

5+15

T.L.R. $\frac{157.9}{11.5}$

5+25

T.L.R. $\frac{158.4}{15.5}$

5+35

5+40.7

5+44.05

Base

210.0
 198.0 179.5 158.3 153.9 144.7 144.9
 45 70 50 40. 0.6 9.8 9.6 T.S.
 83.5 5.5
 F.L.R.
 T.L.R. 182.1 181.6 173.3 155.1 149.7 146.5 145.7
 59 51 43 29 28 4.6 9.0 8.8 T.S.
 3.5
 165.6 152.9 147.1 147.3 147.5
 192.0 181.0 172.6 1.6 7.4 7.2 7.0 T.S.
 62 52 40 37 11.5 8 2.5

147.1
 192.0
 181.0 181.1 164.7 157.9 149.9 147.9 148.1
 62 52 40 25 11.5 11.5 6.4 T.C.S.
 1.5
 Isolate Bould. cr. 31 cu. yds. F.L.R.
 182.0 181.0 181.1 168.2 158.4 152.4 148.4 148.5
 62 52 40 25 15.5 15.5 6.0 T.S.
 1.0
 F.L.R.
 T.L.R. 193.5 180.6 169.1 159.0 152.7 150.6 146.8
 50 38 23 18 15 11.0 1.8 4.5 5.7 T.S.

182.8
 37
 F.L.R.
 149.6
 4.9

A.L.L.
 9/29/20

166.0 153.5
 16.0 1.0

Ledge Rock not in X sections.

20 x 12.5 x $\frac{12.5}{2}$ = 55.5 cu. yds.
 10 x 10 x 25 = 92.6 " " "
 26 x $\frac{10}{2}$ x $\frac{12}{2}$ = 28.9 " " "
 8 x 7 x 15 = 31.1 " " "
 Total 208.1 cu. yds.

Spillway
June estimate,

Base

39

Sta.	+ H.I	-	El. v.
B.M.	4.58	157.42	152.84
	0.74	149.14	148.40

3+25

3+50

3+66

3+75 }
3+75 }

3+97 }
3+97 }

4+13 }
4+13 }

4+25 }
4+25 }

4+50

start of ledger rock.

4+75

5+00

5+15

B.M. 6.73 159.57

152.84

Finalisthe.
Original Xsection

59 ft. cu. ft.

0.0

600.0

48.0

1,896.0

189.0

2,029.5

262.0

308.0

13,002.0

874.0

936.0

18,152.0

708.0

556.0

6,510.0

529.0

760.0

16,325.0

546.0

8,150.0

106.0

3,000.0

134.0

2,535.0

204.0

1,670.0

F.L.R.
145.2 141.7 140.0 139.8 140.6 140.7 140.9
49 47.1 43.8 39.0 32.3 12.5

F.L.R. F.L.R.
153.3 147.1 144.1 142.5
49.2 49.2 45.7 41.3 34.7 24.5 11.5

F.L.R. F.L.R.
157.0 145.4 142.5 142.9 142.5
49.0 44.7 39.5 31 23 11.0

F.L.R. F.L.R.
223.7 193.7 176.0 159.2 146.2 142.7 142.5
94 80 60 52 49 44 9

F.L.R. F.L.R.
226.0 211.6 185.4 177.9 158.4 148.4 144.2 143.3 143.2
89 88 68 60 55.4 48 42 7.4

2215 216.0 211.6 89 88 68 60 55.4 48 42 7.4
89 89 85.4
219.4
211.3
88.5 82.3
208.2 195.4
178.1 169.4 152.2 145.7 144.2 143.8 144.0 6.9

Duplicate section
7.4
180.75.0 66.5 55.3 45.3 36 29 6.9

F.L.R.
207.0 197.9 182.2 176.4 161.1 147.6 144.6 144.6 144.7
80.5 73 70 62.5 43 32.7 27.4 5

T.L.R. F.L.R.
177.0 171.8 167.0 160.0 156.0 147.5 145.5 145.6 145.6
47.0 45.5 38 33 32 29.3 24 3.6

T.L.R. F.L.R.
161.0 159.3 153.3 148.3 146.5
28.4 27.4 23.5 23.5 20 147.4 147.9 2.5

F.L.R. F.L.R.
180.9 174.8 165.4 150.4 148.7 147.9
43.4 39 25 19 16.5 11 9 148.3

Sta.	+	M.I.	-	Elev.
5+25				
5+35				
5+40.7				
5+44.05				

Base

181.0	181.0	178.5	163.1	F.L.R.	152.2	148.6	144.1	148.6
52	47	43	21	18.4	13.5	8.7		
182.5	165.3	158.5	154.0	148.6	148.8			
46	22	19	19	11.4				
182.8	T.L.R.	F.L.R.	153.4	157.2				
37	160.0	157.3	12	7.5				
					166.0	153.5		
					16			

59.ft. Cu.ft.

130.0

1,190.0

108.0

359.1

18.0

30.1

0.0

70,448.7 =

2,609.2

Cu. yds.

Computed W.M.B. 6-29-30

Checked.

Xsections 2,609.2 cu yds.

Isolated Buildings 31.0 " "

Measured Ledge 2080

(Backfill retaining walls) 72.7

Page 74

2,920.9

Total Yardage 2,920.9 cu yds.

Ledge Rock 340 cu yds.

Total Class. 2,580.9 cu yds.

Computed W.M.B. 9-24-30

Checked W.D.L 9-24-30

Checked

Spillway
Ledge Rock
June estimate.

sta.	sq. ft.	cu. ft.
4+50.	00	275
4+75	22	825
5+00	44	1.245
5+15	122	815
5+25	41	280
5+35	15	94.05
5+40.7	18	30.15
5+44.05	00	

3.564.2 = 132 cu. yds.

Measured Ledge Rock 208 " "

Total Ledge Rock 340 cu. yds.

Computed W.M.B. 6-29-30
Checked W.D.L 9-24-30

Final Estimate Outlet Tower.

Concrete Work.

ϕ Diam. Barrel = 19'-10" Cir. = 62.31 ft. cu ft.
 Barrel
 $4'-6" + 5'-4" \times 8" \times 62.31' = 4.9167 \times 0.67 \times 62.31' = 20.524$
 $\frac{2}{\text{Key}} 62.31' \times 2' \times 4' = 62.31 \times .83 \times .17 = 3.46$
 Steps
 $9 \times 3.0' \times 8" \times 7" \times \frac{1}{2} = 9 \times 3.0 \times 0.67 \times 0.5 \times 5.83 = 5.25$
 Key in Well. 7.0
 $4" \times 2' \times 5.5' = 0.39$
 Well.
 $1'-11" + 1'-11" + 8'-8" = 7'-6" \times 8" \times 3'-6" = 17.58$
 Roof, Lower
 ϕ Diameter. 20'-11 $\frac{7}{8}$ " = 20.9896 Cir. = 65.9673
 Roof upper
 ϕ Diameter. 9'-3 $\frac{3}{4}$ " = 9.2656' Cir. = 29.1086'
 Average
 $\frac{2}{\text{Slope of Roof}} = 8.7083$
 $47.538' \times 8" \times 8.7083' = 276.98$
 Top of Roof
 $9'-7" + 9'-0" = 9'-3\frac{1}{2}" \phi$ Diam. = 67.84
Total 574.57

Deductions.

Dom.
 Diam. 5'-6" = $3.1416 \times \frac{5.5^2}{2} \times 8" = 15.84$ cu ft.
 Manhol.
 Diam. 22" = $3.1416 \times (11")^2 \times 8" = 1.76$ cu ft.
 17.60
 556.97

120 bags of cement.

Holes: 1"

10 - 1'-6" = 15 linear feet.

Steel Rings.

47.5 feet - $\frac{3}{4}$ " ϕ 1.502[#] @ = 71 lbs.

Reinforcing Steel

42
July 3, 1930

Barrell	Linear feet.
59 - 7'-0"	$\frac{5}{8}$ " ϕ = 413.0 ✓
10 - 3'-0"	$\frac{5}{8}$ " ϕ = 30.0 ✓
40 - 19'-3"	$\frac{5}{8}$ " ϕ = 770.0 ✓
Top of Tower. 1 - 9'-6"	$\frac{5}{8}$ " ϕ = 9.5 ✓
1 - 10'-0"	$\frac{5}{8}$ " ϕ = 10.0 ✓
2 - 3'-6"	$\frac{5}{8}$ " ϕ = 7.0 ✓
2 - 4'-0"	$\frac{5}{8}$ " ϕ = 8.0 ✓
2 - 9'-0"	$\frac{5}{8}$ " ϕ = 18.0 ✓
2 - 7'-6"	$\frac{5}{8}$ " ϕ = 15.0 ✓
2 - 3'-6"	$\frac{5}{8}$ " ϕ = 7.0 ✓
4 - 2'-6"	$\frac{5}{8}$ " ϕ = 10.0 ✓
1 - 5'-6"	$\frac{5}{8}$ " ϕ = 5.5 ✓
33 - 19'-3"	$\frac{5}{8}$ " ϕ = 635.0 ✓ - 635.05
1 - 6'-0"	$\frac{5}{8}$ " ϕ = 6.0 ✓
1 - 10'-0"	$\frac{5}{8}$ " ϕ = 10.0 ✓
59 - 10'-0"	$\frac{5}{8}$ " ϕ = 590.0 ✓
	2544.0 x 1.05 [#] = 2671.2 lbs.
6 - 10'-0"	$\frac{5}{8}$ " ϕ = 60.0
8 - 5'-0"	$\frac{5}{8}$ " ϕ = 40.0
	} x 1.05 [#] = 105.0 lbs.
	2,776.2 lbs

Computed 7/15/30
W.M.B.

Checked 8/21/30

W.D.L.

A.C.L.
10/1/30

Final Estimate

Parapet Wall.

Panel No.	Length.	Panel No.	Length.
28	6.2	3	8.2
27	20.1	2	8.7
26	17.8	1	8.7
25	14.8	Total length. 528.3	
24	22.0	of Wall without pilasters.	
23	22.4	2" Holes	503.0 ft.
22	22.6		
21	22.1	1" Holes.	278.0 ft.
20	22.5		
19	23.4		
18	23.3		
17	22.1		
16	22.2		
15	22.2		
14	22.5	Concrete Deduction -	
13	22.7	Bridges cat	
12	22.4	17.4 x 10" x 10" = 12.08 cu.ft.	
11	22.6	2 x 2' x 2' x 1'-4" = 10.67 cu.ft.	
10	22.6	2 x 1' x 1' x 1/3' = .67 cu.ft.	
9	22.1	23.42 cu.ft. =	
8	22.2	A.L.L. 10/11/30 0.87 Cuyds.	
7	22.8		
6	21.6	Checked 8/21/30	
5	8.8	W.D.L.	
4	8.9		

Concrete in Wall

43

Key 5	28'-3" x 1/2' x 1/3' =	29.35	✓
Pilaster Wall	{	528.3 x 5 x 5/8 =	2201.25 cu.ft. ✓
		528.3 x 1 1/2' x 5" =	27.52 cu.ft. ✓
		29 x 5' x 2' x 2' =	580.00 cu.ft. ✓
		29 x 1' x 1' x 1/3' =	12.70 9.67 cu.ft.
		2847.79 cu.ft. = 105.47	✓
Deduct.		2850.88 " "	0.87 ✓

621 bags Cement. (104.72) 104.60 Cuyds. ✓

Steel reinforcement. A.L.L. 10/11/30

1177.5' { 1" □ = 3300.6' @ 3.43# = 14,038.83 ✓
1" □ = 7000' @ 3.43# =

971.5' of 5/8" φ = 1177.5' @ 1.05# = 1,020.08 ✓

390 - 4 1/2" 1/2" □ = 1787.5' @ 0.86# = 1537.25 ✓

Misc. Lengths. 3/4" φ = 4374' @ 1.52# = 6648.48 ✓

140 - 8'-0" 5/8" φ = 1120' @ 1.05# = 1,176.00 ✓

14418.04

Note 1 For 1" □ take depth of 2" Holes

drilled into concrete and add 4 1/2" for

each Hole. 503.0 ft. of drilled Holes.

142 holes x 4 1/2" = 674.5 ft.

1177.5 ft. of 1" □ steel

Note 2 For 5/8" φ anchors in wall take depth of 2"

holes drilled into concrete and add 4'-9" for

each hole. 278.0 ft. of drilled Holes

146 holes x 4 1/2" = 693.5 ft.

971.5 ft. of 5/8" φ steel.

Computed. W.M.B. 8/15/30

Final Estimate
Parapet Wall

List of Steel Reinforcement.

2-16'7" = 33'-2"	3/4" ϕ	3-34'6"	103'-6"	3/4" ϕ
4-22'7" = 90'-4"	"	3-35'5"	106'-3"	"
2-22'0" = 44'-0"	"	3-35'3"	105'-9"	"
2-22'1" = 44'-2"	"	3-34'2"	102'-6"	"
2-35'7" = 71'-2"	"	3-34'3"	102'-9"	"
2-36'9" = 73'-6"	"	3-34'2"	102'-6"	"
6-36'3" = 217'-6"	"	3-34'6"	103'-6"	"
6-36'2" = 217'-0"	"	3-34'9"	103'-9"	"
8-36'7" = 292'-8"	"	3-34'6"	103'-6"	"
8-36'6" = 292'-0"	"	3-34'7"	103'-9"	"
2-37'3" = 74'-6"	"	3-34'7"	103'-9"	"
2-37'5" = 74'-10"	"	3-34'2"	102'-6"	"
2-36'0" = 72'-0"	"	3-34'3"	102'-9"	"
2-28'10" = 57'-8"	"	3-34'9"	104'-3"	"
2-31'10" = 63'-8"	"	3-33'7"	100'-9"	"
2-34'2" = 68'-4"	"	3-20'1"	60'-3"	"
2-14'0" = 28'-0"	"	3-20'7"	61'-9"	"
3-13'0" = 39'-0"	"	3-20'0"	60'-0"	"
3-32'2" = 96'-6"	"	3-20'7"	61'-9"	"
3-29'10" = 89'-6"	"	3-15'7"	46'-9"	"
3-26'10" = 80'-6"	"		4374 feet.	"
3-34'0" = 102'-0"	"		Transferred Page 43.	"
3-34'5" = 103'-3"	"			"
3-34'7" = 103'-9"	"			"
3-34'3" = 102'-9"	"			"

A.C.L.
10/1/30

W.M.B.
Computed 8-17-30

Checked 8/21/30
W.D.L.

Final Estimate
Piers & Bridge.

Reinforcing Steel.

Piers.

22-21'0"	}	1 1/8" \square = 462.0 ft @ 4.34" =	
1-13'0"		1 1/8" \square = 13.0 ft @ 4.34" =	
23-16'0"		1 1/8" \square = 368.0 ft @ 4.34" =	
23-11'0"	}	1 1/8" \square = 253.0 ft @ 4.34" =	
46-9'0"		1 1/8" \square = 414.0 ft @ 4.34" = 6,553.4	165
92-13'0"	}	5/8" Φ = 1196.0 ft @ 1.05" =	
46-14'0"		5/8" Φ = 644.0 ft @ 1.05" =	
46-14'6"		5/8" Φ = 667.0 ft @ 1.05" =	
-16'0"		5/8" Φ = 736.0 ft @ 1.05" =	
46-17'6"	}	5/8" Φ = 805.0 ft @ 1.05" = 4,250.4	
46-9'6"		1" \square = 437.0 ft @ 3.43" =	
46-10'6"	}	1" \square = 483.0 ft @ 3.43" = 3,155.6	
230-9'6"		5/8" Φ = 2,185.0 ft @ 1.05" =	
12-8'0"	}	5/8" Φ = 96.0 ft @ 1.05" =	
12-6'0"		5/8" Φ = 72.0 ft @ 1.05" =	
12-4'0"		5/8" Φ = 48.0 ft @ 1.05" = 2,521.05	

ALL

Computed 8/10/30
W.M.B.

Checked 8/21/30
W.D.L.

Reinforcing Steel Bridge Slab.

231	30'0" = 6,930.0 ft	5/8" Φ @ 1.05" =	
99	32'0" = 3,168.0 ft	5/8" Φ @ 1.05" =	
56	11'0" = 616.0 ft	5/8" Φ @ 1.05" =	
352	6'6" = 2,288.0 ft	5/8" Φ @ 1.05" =	13,652.0
4	12'0" = 48.0 ft	3/4" Φ @ 1.52" =	165
4	16'0" = 64.0 ft	3/4" Φ @ 1.52" =	200.64
1	20'0" = 20.0 ft	3/4" Φ @ 1.52" =	
2	8'6" = 7.0 ft	5/8" Φ @ 1.05" =	
1	5'0" = 5.0 ft	5/8" Φ @ 1.05" =	
1	6'0" = 6.0 ft	5/8" Φ @ 1.05" =	
2	6'6" = 13.0 ft	5/8" Φ @ 1.05" =	
4	7'0" = 28.0 ft	5/8" Φ @ 1.05" =	
4	8'0" = 32.0 ft	5/8" Φ @ 1.05" =	
1	9'0" = 9.0 ft	5/8" Φ @ 1.05" =	
10	10'0" = 100.0 ft	5/8" Φ @ 1.05" =	
8	11'0" = 88.0 ft	5/8" Φ @ 1.05" =	
7	12'0" = 84.0 ft	5/8" Φ @ 1.05" =	
8	13'0" = 104.0 ft	5/8" Φ @ 1.05" =	
10	14'0" = 140.0 ft	5/8" Φ @ 1.05" =	
4	15'0" = 60.0 ft	5/8" Φ @ 1.05" =	
1	16'0" = 16.0 ft	5/8" Φ @ 1.05" =	
1	17'0" = 17.0 ft	5/8" Φ @ 1.05" =	
2	19'0" = 38.0 ft	5/8" Φ @ 1.05" =	
1	20'0" = 20.0 ft	5/8" Φ @ 1.05" =	
1	23'0" = 23.0 ft	5/8" Φ @ 1.05" =	
2	40'0" = 80.0 ft	5/8" Φ @ 1.05" =	
	870.0 ft		1913.50

10/2/30
A.L.L.

Structural Steel.
Spillway.

					lbs.
38	18'-2 1/4"	6"	15"	Channels	7516.875 ✓
1	13'-2 3/4"	6"	"	"	198.408 ✓
1	14'-7"	6"	"	"	218.750 ✓
1	14'-8"	6"	"	"	220.000 ✓
1	16'-0 1/4"	6"	"	"	240.312 ✓
1	16'-1 1/4"	6"	"	"	241.563 ✓
1	17'-5 1/2"	6"	15"	"	261.875 ✓
					8897.783 ✓
19	13'-9 1/4"	8"	34"	H'Beams	8895.737 ✓
1	14'-6"	8"	"	"	493.000 ✓
1	15'-11 1/4"	8"	"	"	541.875 ✓
1	17'-4 1/2"	8"	34"	"	590.75 ✓
					19521.562 ✓

Total Weight. 19,419.345 lbs. ✓

6" C Anchors. 251.02

Theoretical Check. 19,670. lbs. ^{Act.} 10/2/30

Structural Steel as billed by McClintock-Marshall Co. and as allowed in the estimate 20,289 lbs

W.M.B.

W.M.B.
Computed. 8/19/30

Checked 8/21/30

W.D.L.

July Estimate
Channel below Spillway Lower end.

Bankway
Drawing
7/25/30

Sta + K - Elev. Lt. Base Line Rt.

Note - Base line elevations determined by levels slopes by stadia + hand level Sec BK 309 - Page 29-30

Sta	+	K	-	Elev.	Lt.	Base Line	Rt.	Sq. FT	Cu FT
0+00	8.13	127.50		119.37		123 ⁰ ₂₂ 123 ² ₂₁ 149 ⁶ ₆₁ 173 ⁸ ₈₂ 223 ² ₁₃₆		888.0	56,315.
0+00						123 ⁰ ₂₄ 124 ¹ ₅₂ 132 ⁶ ₅₃ 137 ⁵ ₈₁ 152 ⁰ ₁₆₀		2,330.0	54,425.
0+25						114 ⁸ ₃₂ 115 ³ ₅₆ 120 ⁸ ₉₄ 149 ⁶ ₁₀₉ 175 ⁰ ₁₁₉ 184 ⁷ ₁₅₃ 204 ⁸ ₁₆₃ 212 ¹ ₁₆₃		2,024.0	
T.P.	0.68	115.37	12.81	114.69					
0+38						115 ³			55,150..
0+50						110 ⁹ ₅₆ 111 ¹ ₈₆ 131 ² ₀₀ 159 ³ ₁₁₉ 173 ⁶ ₁₄₀ 193 ² ₁₆₂ 203 ⁸		2,388.0	
0+68						108 ²			41,350..
0+75						104 ⁵ ₂₀ 109 ³ ₄₆ 109 ¹ ₇₆ 126 ⁶ ₉₈ 142 ² ₁₂₈ 172 ² ₁₄₃ 194 ⁴		920.0	11,500..
1+00						95 ⁰ ₁₂ 100 ⁰ ₂₉ 110 ² ₇₀ 121 ⁴ ₉₈ 143 ²		0.0	218,740.

B.M. on Guide wall

on Diagonal
Note - Use 35' between sections.

At right angles

Final Est.
 July Estimate
 Channel opposite Spillway
 upper end.

7/25/30
 Benham
 Dewing

Baseline

Sta + π - Elev.
 3.6 140.5 136.9

Excavation 49
 59-ft. Cu.ft.

0+00 seepage ¹⁰⁰ 48,0+00 on diagonal

(Seepage 54)

0+10.3

Final Est.

126.0 124.8 124.0 123.0 146.2 128.8 129.5 123.0
 22.0 20 20 32 25.5 12 31

32.0

779.1

0+25

Final est.

128.0 126.0 126.5 142.0 141.2 126.3 125.8 126.5
 30.8 20 20 31 28 7 20

74.0

1000.0

0+50

130.2 129.2 130.2 130.2 130.0
 30 15 10 23

6.0

75.0

0+75

130.8 130.6 130.3
 33 18

0.0

512.5

1+00

132.2 131.8 130.3 130.2 131.8
 35 31 23 19

41.0

1489.5

1+25

134.2 132.0 131.2 132.5
 37 29 20

78.0

1287.0

1+38

134.3 132.2 132.2 133.1
 36 32 21

120.0

1392.0

1+50

134.4 132.2 132.4 133.4 133.3
 37 33 24 19

112.0

1440.0

1+65

135.4 133.4 132.2 134.4 133.5
 38 34 20 5

80.0

670.0

1+75

135.2 133.4 132.2 134.0
 38 30 20

54.0

565.0

12,870

Sta	+	T	-	Elev.	Base Line	Excavation 50	
						Sq. ft.	Cu. ft.
1+85		140.5			$\frac{135^{\circ}}{40}$ $\frac{133^{\circ}}{32}$ $\frac{133^{\circ}}{21}$ $\frac{134^{\circ}}{21}$	59.0	900.0
2+00					$\frac{137^{\circ}}{41}$ $\frac{134^{\circ}}{35}$ $\frac{134^{\circ}}{21}$ $\frac{136^{\circ}}{4}$ $\frac{136^{\circ}}{4}$	61.0	510.0
2+10					$\frac{138^{\circ}}{35}$ $\frac{134^{\circ}}{32}$ $\frac{134^{\circ}}{18}$ $\frac{136^{\circ}}{2}$	44.0	382.5
2+19					$\frac{139^{\circ}}{46}$ $\frac{135^{\circ}}{41}$ $\frac{134^{\circ}}{34}$ $\frac{135^{\circ}}{13}$ $\frac{136^{\circ}}{8}$ $\frac{136^{\circ}}{8}$ $\frac{137^{\circ}}{21}$	44.0	264.0
2+31					$\frac{138^{\circ}}{48}$ $\frac{137^{\circ}}{41}$ $\frac{135^{\circ}}{24}$ $\frac{135^{\circ}}{7}$ $\frac{136^{\circ}}{2}$	0.0	0.0
2+50					$\frac{138^{\circ}}{47}$ $\frac{135^{\circ}}{28}$ $\frac{136^{\circ}}{18}$ $\frac{136^{\circ}}{9}$ $\frac{137^{\circ}}{9}$	0.0	0.0

Total Excavation

$\frac{A.L.V.}{10/2/30}$
 11,264.6 =
 417.2
 cu. yds.

Computed W.M.B. 9-22-30

Checked W.D.L. 9-23-30

July Estimate
Spillway Channel.

Showing slide left in Channel.

Sta.	Sq. Ft.	Cu. Ft.
1+75	0.0	
		49.125
1+00	1,310.0	
		29,000.0
0+75	1,010.0	
		17,625.0
0+50	400.0	
		6,250.0
0+25	100.0	
		3,625.0
0+00	190.0	
		2,830.0
Diagonal 0+00	10' Dist. 376.0	
		3,430.0
0+10.3	290.0	
		3,138.0
0+25	137.0	
		2,262.0
0+50	44.0	
		800.0
0+75	20.0	
		250.0
1+00	0.0	
Total		118,335.0 = 4.383 cu. yds.

Note:-

Excavation = 8.679 cu. yds.

Embankment = 4.383 " "

left in Channel

Balance for 4.296 cu. yd.

July Estimate.

Isolated Boulders 302 cu. yds.

Total 4.598 cu. yds.

Computed. W.M.B. 7/26/30

Checked C.D. 7/26/30.

Exc. = 8,739 cu. yds.

Emb. 4,447 " "

4,292 cu. yds.

Boulders - 302 " "

Total = 4,594 cu. yds.

A.C.L.

7/30/30

Final Record
 Drill Holes in Parapet Wall

Panel No

1	2.0	2.0				
2	2.0	2.0	2.0	2.0	2.0	2.0
3	2.0	2.0	2.0	2.0	2.0	
4	1.5	1.5	2.0	2.0		
5	2.0	1.5	2.0	2.0	1.5	1.5
6	1.0	2.0	2.0	1.5	1.5	2.0
7	2.0	2.0	2.0	2.0	2.0	2.0
8	2.0	2.0	2.0	1.5	2.0	2.0
9	2.0	2.0	2.0	2.0	2.0	2.0
10	2.0	2.0	2.0	2.0	1.5	0.5
11	2.0	2.0	2.0	2.0	2.0	2.0
12	2.0	2.0	2.0	2.0	2.0	2.0
13	2.0	2.0	2.0	2.0	2.0	2.0
14	2.0	1.5	2.0	2.0	2.0	2.0
15	2.0	2.0	2.0	2.0	2.0	2.0
16	1.5	2.0	2.0	2.0	1.5	1.0
17	2.0	1.5	1.5	2.0	1.5	1.5
18	2.0	2.0	2.0	1.5	2.0	2.0
19	2.0	2.0	2.0	2.0	2.0	2.0
20	2.0	2.0	2.0	2.0	2.0	2.0
21	2.0	2.0	2.0	2.0	2.0	2.0
22	2.0	2.0	2.0	2.0	2.0	2.0
23	2.0	2.0	2.0	2.0	2.0	2.0
24	2.0	2.0	2.0	-	-	-
25	1.5	2.0	2.0	-	-	-

South end of parapet wall next to cliff

Note: All holes in panel are 1" holes.

53

Final Record
on
Drill Holes in Parapet Wall.

Panel No

26	1.5	1.5	1.5	-	-	-
27	2.0	2.0	2.0	-	-	-
28	2.0	2.0	2.0	-	-	-
	A.L.L.	A.L.L.	A.L.L.	A.L.L.	A.L.L.	A.L.L.
	53.0 ft	53.5 ft	53.0 ft	42.5 ft	(39.5 ft)	36.5 ft

North end parapet wall

1" Holes = 278 feet.

53.0 ft
 53.5
 53.0
 42.5
 39.5
 36.5

 278.0 ft

A.L.L.
10/2/30

Checked 8/21/30

W.D.L.

Final Cross sections

of Spillway Channel

B.M. 8.68 128.09 ^{Adjusted} distance. 119.41

0+0.0 = 0+10.3 Baseline back.

Rt. L to Base
Back. F. 40°49' to section ahead: 6.00'

0+0.0
Diagonal
40°49' to
Section ahead

8.40'

0+0.0
Diagonal
90°38' to
Section ahead.

17.38'

0+0.0
Rt. L to Base
Line S.W.

T.P. 12.8.09 8.69 13.0 119.41

4.63 124.04

0+13.

12.0'

T.P. 124.04 11.38 112.66

Sept.
1930.
Excavation 54

59 ft. Cu. ft.

26.0

4,760.0

123.2 123.8 125.0 125.6 Vert. 1562.0
or 4.0 21 55.0 Face

Duplicate Cross section ahead
from this point.

123.7 123.7 124.2 123.4 126.4 132.3
0.0 4.0 17.0 27.0 42.0 55.0

136.2 141.8 153.8 167.3 178.4 195.2 199.2
55.0 58.9 69.5 83.8 95.0 113.6 113.6

229.9 237.0 247.5 Vertical face
142.9 143.5 158.7

1,524.0

33,673.8

123.7 123.7 122.5 124.9 133.1 157.2
0.0 4.0 35.0 47.0 65.0 82.6

175.9 212.5 222.6 226.6 1/4:1 slope up
103.1 144.6 160.3 160.3

2,351.0

30,498.0

120.2 119.7 118.6 116.1 122.9 131.2
0.0 4.0 50.0 40.0 60.0 70.9

150.5 167.6 170.3 180.7 183.7 191.7 205.7
82.0 103.5 105.5 118.1 118.5 125.4 141.6

219.5 Vertical face.
165.6

2,341.0

26,856.0

26,856

Base Line

Excavation 55

Sq. ft. Cu. ft.

Sta + HI - Adjusted Elev
 T.P. 3.37 116.03 Distance 112.66

0+25
 13.0

112.7 111.7 111.8 116.5 128.6 166.3
 0.0 4.0 33.0 56.5 69.3 108.0
 179.0 195.3 208.2 218.7 Vertical face.
 114.3 131.0 152.8 166.8

2,135.0

T.P. 116.03 11.48 104.55
 3.96 108.51
 13.0

29,061.5

0+38
 12.0

104.3 104.3 102.0 113.5 133.6 166.8
 0.0 4.0 38.3 57.7 81.0 103.4
 181.1 211.6 213.9 Note: Old ground vertical face
 121.3 162.2 162.2

2,336.0

B.M. 10.54 101.50 90.96

30,888.0

0+50
 18.0

99.2 97.8 97.1 102.5 104.5 133.3
 0.0 4.0 35.7 52.5 52.5 88.0
 145.2 168.3 200.3 210.7 213.2
 89.5 112.4 149.0 165.9 165.9
 Note - Vertical face.

2,812.0

T.P. 101.50 10.54 90.96
 1.75 92.71
 18.0

46,980.0

0+68
 5.0

90.1 88.1 88.5 95.1 108.2 111.1
 0.0 4.0 40.9 54.7 55.2 64.2
 162.2 190.2 200.5 original ground.
 108.6 135.3 149.0

2,408.0

East Line

Sta + Hi - Adjusted Elev
Distance

0+73.
Rt. to Back
Base Line
LL 12.5'

T.P. 92.71 1.75 90.96
1.17 92.13 16.92^v

0+73
Rt. to base
ahead.

2.0^v

0+75

7.0^v

0+82

T.P. 92.13 3.43 88.70
18.0^v
0.65 89.35

1+00

T.P. 89.35 12.84 76.51
13.0^v

Excavation 56

59 ft. Cu. ft.

10,947.5

Base Line

88.2 87.0 87.0 92.7 95.1 108.2 111.1
0.0 6.0 45.7 55.0 54.7 55.2 64.2
162.2 190.2 197.0 original ground.
108.6 135.3 144.0

1.971.0^v

31,555.8

88.1 86.2 134.4 162.3 172.0 194.0
0.0 47.0 90.0 117.6 128.0 143.0
Vertical Face.

1.759.0

3,518.0

88.1 86.2 134.4 162.3 172.0 194.0
0.0 47.0 90.0 117.6 128.0 143.0
Vertical Face.

1.759.0

11,123.0

Lt Base Line Rt

104.8 104.8 109.8 109.8 88.4 86.5 85.9 134.4
37.0 34.0 29.0 19.0 0.0 4.0 4.8 9.0
1/2' slope down.
162.3 175.0 172.0
117.6 130.0 145.0
original ground.

1.419.0^v

15,813.0

1/2' slope down. 104.1 104.1 85.2 82.8 84.2 101.3
49.5 9.5 0.0 8.0 41.0 60.8
120.8 use 119.8
72.4 72.4

338.0

3,802.5

Sta	+	H ₁	-	Elev
T.P.	8.35	84.86		76.51

Base
LINE

LT RT

Excavation 57

Sq.ft. Cu.ft.

1+13

56.7	96.7	81.6	80.2	81.3	130.3
64.3	24.3	5.0	0.0	36.5	87.2

1/2% slope down. Original ground.

247.0

T.P.		84.86	8.35	76.51
------	--	-------	------	-------

2,304.0

12.0

7.40 83.91

1+25

34.8	94.8	82.3	79.4	78.6	91.0	108.5
65.8	25.8	10.4	0.0	33.3	49.0	70.0

1/2% slope down. Original ground.

237.0

T.P.		84.86	12.93	70.98
------	--	-------	-------	-------

6,887.5

25.0

2.99 73.97

1+50

87.7	87.7	72.3	68.3	69.6	95.5	126.6
62.2	25.2	9.3	0.0	33.8	58.1	117.8

1/2% slope down. Vertical face up.

314.0

Make 96.1
56.1
0.6

T.P.		73.97	12.36	61.61
------	--	-------	-------	-------

2,736.5

13.0

8.30 69.91

Make 87.0
59.7

1+63

81.3	81.3	67.8	65.1	13.7	73.6	87.6	0.0
23.9	34.9	12.2	0.0	32.0	43.7	59.7	

1/2% slope down.

107.0

12.0

1,506.0

Sta	+	Hi	-	Elev
T.P.		69.91	8.30	61.61

	4.98	66.59		
--	------	-------	--	--

1+75

82.7	82.7	71.9	62.2	61.4	61.3	71.7	144.0
78.7	48.7	26.9	8.0	0.0	36.2	50.0	

1/2% slope down. original ground.

T.P.		66.59	4.98	61.61
------	--	-------	------	-------

10.85

1,898.8

	2.19	63.80		
--	------	-------	--	--

1+85⁸⁵

4.15

80.7	68.1	60.5	59.5	61.9	74.0	206.0
53.1	15.4	7.2	0.0	44.0	62.0	

1/2% slope down. original ground. 427.5

1+90²⁰ End of Excavation

T.P.		63.80	12.15	51.65
------	--	-------	-------	-------

End of Excavation

308,798.6
Cu ft

	10.99	62.64		
--	-------	-------	--	--

2+00

68.0	66.3	58.0	57.2	1/4% slope up
48.3	22.7	0.0	44.0	Boulder.

T.P.		62.64	12.99	49.65
------	--	-------	-------	-------

Computed 9/23/30 W.M.B.
Checked 9/23/30 W.D.L.

(See Page 69).

11,436.98 Cu Yds.

	0.68	50.33		
--	------	-------	--	--

2+25

50.3	42.8	40.0	Vertical face
50.0	0.0	51.0	Boulder

Base
Line.

Sta	+	H _i	-	Elev
		50.33		
2+41 ^o				
T.P.		50.33	0.68	49.65
	12.91	62.56	0.64	61.92
	12.20	74.12	0.50	73.62
	12.82	86.44	1.89	84.55
	8.38	92.93	1.96	90.97

LT	RT	
39.3	34.3	31.4
50.0	0.0	35.0
		Big Boulder
		25.9
		57.0
		Vertical face.

B.M. = 90.96
90.97 = .01 Check.

Final Cross Section of Roadway
South End of Dam.

Sta	+	H ₁	-	Adjusted. Dist.	Elev.	Lt	Base Line	Rt.	Total Excavation ⁶⁰	
									Sq.ft.	Co.ft.
B.M.	105	192.45			191.40					
T.P.	0.22	182.08	10.59		181.86					
T.P.	1.22	173.27	10.03		172.05					
0+86						167.0	167.0	167.0	0.0	
				2		13.0	6.0	8.0		28.0
0+88						182.3	174.9	167.9	168.4	168.4
						47.1	23.9	14.4	0.0	8.0
									198.7	1/2:1 Slope Down
T.P.		173.27	1.22		172.05				74.3	
				12						28.0
	2.42	174.47								
1+00						182.3	175.4	169.1	169.3	169.7
						42.2	22.4	12.0	0.0	11.0
									127.5	1/2:1 Slope Down
									64.5	
T.P.		174.47	2.42	25	172.05					50.0
										875.0
	4.14	176.19								
1+25						197.0	178.3	171.6	171.0	171.2
						51.9	22.9	8.0	0.0	6.6
									15.0	1 1/2:1 Slope down
				9.12						20.0
T.P.		176.19	4.14		172.05					405.8

Sta	+	H _i	-	Adjusted	Elev
T.P.	4.54	176.59		Dist.	172.05

1+34 ⁴⁵
Diagonal

$\frac{194.6}{46.2}$	$\frac{182.9}{35.0}$	$\frac{171.9}{10.0}$	$\frac{171.7}{0.0}$	$\frac{170.7}{18.0}$	1/2:1 Slope Down.
----------------------	----------------------	----------------------	---------------------	----------------------	----------------------

69.0

T.P.		176.59	4.54	14.94	172.05
------	--	--------	------	-------	--------

1,172.8^v

5.82 177.87

1+50

$\frac{185.2}{45.8}$	$\frac{179.3}{22.9}$	$\frac{173.0}{9.0}$	$\frac{172.9}{0.0}$	$\frac{173.2}{8.5}$	1/2:1 Slope Down.
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88.0

T.P.		177.87	5.82	25.0	172.05
------	--	--------	------	------	--------

2,275.0^v

7.76 179.81

1+75

$\frac{182.6}{47.4}$	$\frac{183.0}{24.3}$	$\frac{175.0}{8.5}$	$\frac{174.8}{0.0}$	$\frac{175.1}{10.0}$	1/2:1 Slope Down.
----------------------	----------------------	---------------------	---------------------	----------------------	----------------------

94.0

T.P.		179.81	7.76	25.0	172.05
------	--	--------	------	------	--------

2,475.0^v

9.21 181.26

2+00

$\frac{188.0}{30.0}$	$\frac{186.7}{23.9}$	$\frac{176.5}{8.5}$	$\frac{176.2}{0.0}$	$\frac{176.1}{9.0}$	1/2:1 Slope Down.
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104.0

25.0

$\frac{2000}{46.4}$

2,087.5^v

Base
Line

Total
Excavation^{6'}
Sq. ft. Co. ft.

Sta	+	H _i	-	Adjusted	Elev.
T.P.		181.26	9.21	Dist.	172.05

10.56 182.61

2+25

<u>193.1</u>	<u>189.0</u>	<u>178.4</u>	<u>177.6</u>	<u>177.1</u>	1 1/2% slope Down.
36.9	24.1	8.7	0.0	12.7	
	<u>197.1</u>	<u>193.6</u>			
	46.4	41.3			

63.0

T.P.		182.61	10.56	15.0	172.05
------	--	--------	-------	------	--------

472.5

11.55 183.60

2+40 P.C.

<u>191.4</u>	<u>190.2</u>	<u>179.1</u>	<u>178.8</u>	<u>178.4</u>	1 1/2% slope Down.
30.3	24.7	8.5	0.0	14.0	
	<u>193.1</u>	<u>193.0</u>			
	54.3	43.7			

0.0

T.P.		183.60	0.40	20.1	183.20
------	--	--------	------	------	--------

1,065.3

B.M.	9.83	193.03	1.62		191.41
------	------	--------	------	--	--------

B.M. = 191.40
191.41 - 201 Check.

B.M.	1.04	192.45			191.40
------	------	--------	--	--	--------

T.P.	3.77	185.63	10.59		181.86
------	------	--------	-------	--	--------

2+62

<u>190.8</u>	<u>191.0</u>	<u>180.7</u>	<u>180.6</u>	<u>180.1</u>	<u>186.2</u>	106.0
35.5	24.4	9.0	0.0	11.4	23.6	
				<u>185.1</u>		
				34.9		

22.0

7,590.0

EASE
LINE

Total
Excavation 62

Sq. ft. Co. ft.

Sta	+	H _i	-	Adjusted Elev Dist.	Lt	Base Line	Rt.	Total Excavation ⁶⁴	
								sq.ft.	cu.ft.
3+75		195.66							
T.P.		195.66	0.35	195.31					
				25.0					
	2.19	197.50							
4+00									
T.P.		197.50	2.19	195.31					
				24.42					
	3.04	198.35							
4+25 ⁷¹ Diagonal.									
T.P.		198.35	3.04	195.31					
				19.65					
	4.25	199.56							
4+46									
				0.0					

1984	196.2	196.7	190.9	191.1	191.5	199.9	206.4
25.4	25.0	19.5	10.0	0.0	6.0	15.2	25.6
					22.59	241.2	252.0
					25.6	37.2	48.4
					261.2		
					50.7		
							1,084.0

200.5	192.5	192.5	192.9	198.3	212.1
22.0	13.5	0.0	3.2	9.5	11.9
			221.3	231.6	243.6
			26.7	33.1	41.6
			252.9	268.1	
			49.8	53.7	
					844.0

1921	1937	193.9	194.9	215.1	232.4
21.0	16.5	0.0	4.5	27.3	31.3
			244.5	253.9	272.0
			44.1	54.5	61.9
			291.7		
			80.8		
					558.0

194.9	195.2	135.4	207.7	218.2	227.2
31.0	17.0	2.0	11.4	23.5	26.8
			237.3	248.7	255.7
			36.2	41.5	51.3
			271.8	294.9	
			57.4	81.2	
			0.0		
					509.0

Sta	+	H ₁	-	Adjusted Elev Dist.
		212.60		
5+50				25.0
5+75				25.0
6+00				5.0
6+05				
T.P.		212.60	11.31	201.29
T.P.	1.25	202.54	6.05	196.49

Total Excavation road. 11,329.8 cu. yds.
 Ledger rock Class 2 2,712.3
 Balance Class 1 8,617.5 cu. yds.

LT	Base Line	RT	Total Excavation CC sq. ft. Cu. ft.
204.8 17.0	204.5 0.0	204.9 2.0 Vertical face.	209.3 6.5 189.0
205.1 23.0	205.7 0.0	206.4 4.0 Vertical face.	140.0
206.7 16.5	207.3 0.0	207.8 6.0 Vertical face.	47.0
			0.0
Total.			244,976.2 = 9,073.2 cu. yds.

B.M. = 196.50
 196.49 .01 check.

Computed W.M.B. 9-19-30

Checked W.D.L. 9-22-30

Total Cuyds in X sections of Road. = 9,073.2

Isolated Boulders. = 86.0

Allowance for Boulder removal = 13.0

Removing slide etc. = 1,894.6

Granite surfacing 92 loads 3 yds. e = 276.0

Total allowed on road. = 11,329.8

CH - W.D.L.
 9/24/30

cu. yds
 W.M.B.

Final Cross sections
Road. South of Dam
Ledge Rock

Sta.	Adjusted Dist.	Excavation 67	
		sq. ft.	cu. ft.
0+86		0.0	
	2		28.0
0+88		28.0	
	12		168.0
1+00		0.0	
2+96.0		0.0	
	10.85		596.8
3+06.0		110.0	
	24.55		4529.5
3+28.0		259.0	
	22.93		5755.4
P.T. 3+49.96		243.0	
	25.04		7674.8
3+75		370.0	
	25.0		8825.0
4+0.0		336.0	
	24.79		9197.1
4+25.71 20°53' R.		406.0	
	19.64		6058.9
4+46.0		211.0	
	0.0		
4+46.0		300.0	
	29.0		7554.5

Sta.	Adjusted Dist.
4+75.0	21.09
4+96.84 60.28' RT.	29.36
5+25.0	25.0
5+50.0	

Excavation 68

59.ft.	Cu.ft.
221.0	7,054.6
448.0	11,553.2
339.0	4237.5
0.0	
	13,233.3
	2,712.3
	Cu. yds.

Computed. W. M. B. 9-18-30

Checked. W. D. L. 9-22-30

Summary of Excavation
Spillway Channel below
Spillway crest. Channel.

67

Cross sections ^{pages} 54-58 11,437.0 cu. yds.
{ Allowance for removal
of boulders at end of
Spillway channel. } 293.0 cu. yds.
2000 cu. yds.

Isolated Boulders. _____ cu. yds.

Cross sections ^{pages} 49-50. 417.2 cu. yds.

Total All excavation. 12,147.2 cu. yds.

Computed 9-24-30 W.M.B.

Checked

Total 12,147.2 cu. yds

Allowance/ ass. 2 4,000.0

Balance - Class. 1 8,147.2 cu. yds.

W.M.B.

Checked W.D.L. 9/24/30

Concrete Piers.

Pier	Average Top Elev.	Field Measurements	cu.ft.
# 1	170.17	1 - 208.07	= 207.79
# 2	168.89	1 - 192.08	= 191.89
# 3	167.56	1 - 175.75	= 175.38
# 4	166.23	1 - 159.14	= 158.86
# 5-23	166.17	19 - 158.08 @	= 3003.52
			3737.44

Notes:- Concrete in Standard Piers.
 Piers 5-23 Top Elev. 166.17
 Bottom Elev. 155.00
 11.17
 $11.17 \times 1.33 \times 8.67 = 128.80$ cu.ft. **128.80 cu.ft. A.C.L.**
 Bevel above 155.0
 $11.17 \times 1.33 \times 0.67 \times 2 = 9.90$ cu.ft. **9.90 cu.ft. A.C.L.**
 Pier # 18
 Concrete from gauge 155.0 down to old concrete.

A.C.L.
 10/2/30 138.42
 cu. yds.

4.5 3.5 2.5 1.7 1.1 0.8 0.4 0.2 0.2 0.3
 or 1 2 3 4 5 6 7 8 8.33 =
 12.73
 $12.63 \text{ sq. ft.} \times 1.33' = 16.80$ cu.ft. **16.93 cu.ft. A.C.L.**
 Bevels below 155. A.C.L. ck.
 $\frac{5.1 + 4.5}{2} \times \frac{1.33 \times 0.67}{2} = 2.15$ ck.
 $\frac{1.0 + 0.3}{2} \times \frac{1.33 \times 0.67}{2} = 0.30$ ck.
 2.45 cu.ft.
 A.C.L. above 155 ✓
 A.C.L. below 155

Concrete in 1 ft. in height of pier above 155.0
 $1' \times 1.33' \times 8.67 = 11.53$ cu.ft.
 Bevels
 $1' \times 1.33' \times 0.67 \times 2 = 0.89$ cu.ft.
 Total 12.42 cu.ft.
 A.C.L.
 10/2/30

Total in Standard Pier.
 above 155 138.70 ✓ A.C.L.
 below 155 19.38
 158.08 cu.ft. ✓
 A.C.L.

Computed 8/19/30
 W.M.B.
 Checked 8/20/30
 W.D.L.

Pipe Rails

Bonham
Leckey
8/19/30

93

From	To	Dist.	Field Measurement
Parapet Wall	Pier #3	25.6'	South Side
Parapet Wall	Pier #3	38.5'	North Side
Pier #3	Pier #23	570.0'	Both Sides
Pier #23	End of Retaining Wall #2	36.9	
Total		671.0	✓

Tower Rail

$$2 - 19.5 = 39.0'$$

ALL
10/2/30

Total Pipe Rails = 710 Linear feet.

Computed 8/18/30
M.M.B.

Checked 8/20/30
W.D.L.

Back Fill Retaining Walls
N.E. End of Spillway

Leeley
Barhorn
8/18/30

Checked 8/19/30
W.D.L. 74

Class No. 1 Excavation

0+00

0.0
0.0

0.0
10.0

59.ft. 0.0

Cu.ft. 104.13

+02^e

12.0
0.0

12.5
8.5

104.13

226.20

+04^e

12.4
0.0

13.3
9.5

122.07

0.00

+04^e

12.4
0.0

11.7
4.5

9.2
5.0

7.5
9.5

97.03

456.26

+08^e

11.6
0.0

10.5
1.5

9.6
3.0

8.6
6.5

4.4
13.0

105.75

209.75

+10^e

10.4
0.0

9.5
3.0

7.0
9.5

4.7
12.0

1.2
17.0

104.00

296.94

+14^e

9.9
0.0

8.2
2.5

0.0
13.0

65.68

239.63

+17^e

9.6
2.0

7.5
4.0

5.2
8.5

4.1
9.0

0.0
12.0

71.25

61.00

+18^e

7.6
0.0

7.2
4.0

5.4
5.0

0.0
10.5

50.75

246.86

+25^e

6.4
0.0

5.7
2.0

3.1
3.5

0.0
7.5

24.70

144.94

+32^e

4.8
0.0

4.0
2.5

0.6
3.5

0.0
5.0

13.75

13.75

+34^e

0.0
0.0

0.0

Deduct Pilasters Retaining Wall #2 = 34.92
(see Page #76) Cu.ft.

Total Cu.ft.
1,963.54 Cu.ft. = 72.7 cu yds.
Computed 8/22/30
W.M.B.

A.C.L.
10/2/30

1,998.46
34.92
1,963.54

Checked - W.D.L. 8/19/30

Concrete Deck Slab ^{Bonham}
Leekey.
8/19/30

75

Irregular section Floor = 10" thick

$$\frac{32.6 + 30.0}{2} \times 100' = \frac{59.4 \text{ ft.}}{313.00}$$

$$\frac{11.2 \times 8.7}{2} = 48.72$$

$$\frac{10.7 \times 11.0}{2} = 5.35$$

$$9.5 \times 0.2 \times \frac{2}{3} = 1.27$$

$$\frac{32.6 \times 8.0}{2} = 130.40 \quad \swarrow \text{A.C.L.}$$

498.74

32.76

Deduct $\frac{2}{3} \times 23.4 \times 2.1 = 32.76$

^{59 ft.}
465.98

$$465.98 \text{ cu. ft.} \times 10'' =$$

388.30

Deduct Cover Plates $13.25 \times 0.55 \times 0.83 \times 22 = 133.07$
cu. ft.

Note 2 Irregular section
from north side-pier
#3 to parapet wall.

Parapet Wall to N. side Pier #3 = 388.30

{ Pier #3 - N. side Pier #23
 $285.0 \times 10' \times \frac{5}{16}$ } = 2375.00

Total = 2763.36

Deduct Cover Plate Slots. 133.07

2630.28

9741 cu. yds.

A.C.L.
10/1/30

Computed
W.M.B. 8/19/30

Checked 8/29/30
W.D.L.

Retaining Walls
N. E. End Spillway

Leekey
Bentham
8/19/30

Retaining Wall #2

12.9 12.8 12.6 12.2 11.3 10.1 9.2 8.9 8.4 6.9
0.0 4.2 10.7 12.5 14.0 17.0 20.8 23.1 25.1 26.1

Counterfort #1

11.4' x 3.0' x 1.0'
2

6.0 5.4 5.4 2.8 1.0 0.0
28.6 30.6 33.8 35.3 36.7 38.6

(12" thick)

346.19
17.10

Counterfort #2

9.4' x 2.6' x 1.0'
2

12.22

Counterfort #3

5.6' x 2.0' x 1.0'
2

5.60

Extra Bevel

12.1' x 0.67' x 0.67'
2

2.72
388.83

Total Wall #2.

Retaining Wall #1

17.9 17.5 12.0 9.7 8.7 7.9 6.7 5.8 4.2 1.4 0.0 105.06
0.0 1.3 2.5 4.7 5.3 6.7 7.5 8.1 8.4 11.7 16.0

(12" thick)

Deduct

4.7' x 1.2' x 0.6'
2 = 1.67 cu ft.

11.69
103.37

Total Wall #1.

Total Walls #1 & #2 = 487.20 cu ft. =
18.04 cu yds

ACL
10/2/30

Computed 8/19/30
W.M.B.

checked 8/21/30
W.D.L.

76

Cu. ft.

ACL

Retaining Walls
Additional Work.

41'-2" Holes drilled.
12.5'-1" Holes drilled.
Steel reinforcement.

41	9'0" @	3/4" φ	36.0ft.	1.52 ^{cu} e	165
3	5'0" @	3/4" φ	15.0ft.	1.52 ^{cu} e	77.52
41	10'0" @	1" □	40.0ft.	3.43 ^{cu} e	ALL
2	9'0" @	1" □	18.0ft.	3.43 ^{cu} e	ALL
2	10'0" @	1" □	20.0ft.	3.43 ^{cu} e	267.54
2	5'0" @	3/4" φ	10.0ft.	1.52 ^{cu} e	ALL
2	10'0" @	3/4" φ	20.0ft.	1.52 ^{cu} e	45.60
1	40'0"	5/8" φ	40.0ft.	1.05 ^{cu} e	
1	34'0"	"	34.0ft.	"	
2	33'0"	"	66.0ft.	"	
1	28'0"	"	28.0ft.	"	
2	25'0"	"	50.0ft.	"	
1	24'0"	"	24.0ft.	"	
4	17'0"	"	68.0ft.	"	
2	15'0"	"	30.0ft.	"	
1	13'0"	"	13.0ft.	"	
1	9'6"	"	9.5ft.	"	
3	20'0"	"	60.0ft.	"	
3	9'0"	"	27.0ft.	"	
3	5'0" @	"	15.0ft.	"	
9	8'6"	"	76.5ft.	"	
9	12'0" @	"	108.0ft.	1.05 ^{cu} e	

5	6'0" @	5/8" φ	30.0ft.	1.05 ^{cu} e
2	10'0"	"	20.0ft.	"
14	7'0" @	"	98.0ft.	"
3	4'6"	"	13.5ft.	"
2	2'6"	"	5.0ft.	"
4	5'6"	"	22.0ft.	"
6	11'0" @	"	66.0ft.	1.05 ^{cu} e
				903.5ft 1.05 ^{cu} e = 948.68

(see page 76)
Concrete = 18.04 cu. yds.

Computed. 8/19/30
W.M.B.
Checked 8/21/30
W.D.L.

Spans 9 Gates
Lengths.

Span	Clearance	Gates	Washer	Length outside Diam.
1	12'-11 $\frac{1}{8}$ "	12'-8 $\frac{5}{8}$ "	2 @ 1"	
2	12'-11 $\frac{1}{8}$ "	12'-8 $\frac{5}{8}$ "	1"	2'-6"
3	12'-11 $\frac{1}{8}$ "	12'-8 $\frac{5}{8}$ "	1"	2'-6"
4	12'-10 $\frac{3}{4}$ "	12'-8 $\frac{1}{2}$ "	7/8"	2'-5 $\frac{5}{8}$ "
5	12'-11"	12'-8 $\frac{5}{8}$ "	1"	2'-6"
6	12'-10 $\frac{3}{4}$ "	12'-8 $\frac{1}{2}$ "	7/8"	2'-5 $\frac{5}{8}$ "
7	12'-11 $\frac{1}{4}$ "	12'-8 $\frac{1}{2}$ "	1 $\frac{1}{8}$ "	2'-6 $\frac{1}{2}$ "
8	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
9	12'-11 $\frac{3}{8}$ "	12'-8 $\frac{1}{2}$ "	1 $\frac{1}{8}$ "	2'-6 $\frac{1}{2}$ "
10	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
11	12'-10 $\frac{7}{8}$ "	12'-8 $\frac{5}{8}$ "	1 $\frac{1}{8}$ "	2'-6 $\frac{1}{2}$ "
12	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
13	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
14	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
15	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
16	12'-11"	12'-8 $\frac{1}{2}$ "	1"	2'-6"
17	12'-10 $\frac{7}{8}$ "	12'-8 $\frac{1}{2}$ "	1" or 7/8"	2'-6"
18	12'-11"	12'-8 $\frac{3}{4}$ "	1" or 1 $\frac{1}{8}$ "	2'-6"
19	12'-11 $\frac{1}{8}$ "	12'-8 $\frac{1}{2}$ "	1" or 1 $\frac{1}{8}$ "	2'-6"
20	12'-11"	12'-8 $\frac{9}{16}$ "	1"	2'-6"
21	12'-11"	12'-8 $\frac{9}{16}$ "	1"	2'-6"
22	12'-10 $\frac{3}{4}$ "	12'-8 $\frac{9}{16}$ "	7/8"	2'-5 $\frac{5}{8}$ "

Note: Washers figured to give 1/4"
play on each end. W.F.B.

DIRECTIONS FOR USE OF TABLES

TABLE No. 1.

Distance of slope stake from side or shoulder
stake for any width roadway, slope 1 1/2 to 1
If ground is nearly level, the cut or fill at side
stake is located by the double entry method in
left column and top row. The number in left

of table in same row and column gives distance

IMPROVED TABLES

AND

INFORMATION

TABLE No. 2.

To find Tangent and External for curve of
any other degree, divide by degree of curve and
add correction found in column of correction.

Degree of curve with a given T may be found
by dividing tangent (or external), opposite T by
given tangent (or external).

The distance from a point on the tangent to
the curve is very nearly the square of the tangent
length divided by twice the radius.

1927
Remainder left. 66 bags. 1575 lbs.
16.5
1558.5 used.