

W  
414

# EUGENE DIETZGEN CO.

DRAWING MATERIALS, MATHEMATICAL and  
SURVEYING INSTRUMENTS

Chicago New York San Francisco New Orleans Pittsburg Toronto

Distances from Center of Roadway for Cross-Sectioning  
Roadway 16 feet wide. Side Slopes 1 on 1.  
For Single Track Embankment.

H	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	H
0	8.0	8.1	8.2	8.3	8.4	8.5	8.6	8.7	8.8	8.9	0
1	9.0	9.1	9.2	9.3	9.4	9.5	9.6	9.7	9.8	9.9	1
2	10.0	10.1	10.2	10.3	10.4	10.5	10.6	10.7	10.8	10.9	2
3	11.0	11.1	11.2	11.3	11.4	11.5	11.6	11.7	11.8	11.9	3
4	12.0	12.1	12.2	12.3	12.4	12.5	12.6	12.7	12.8	12.9	4
5	13.0	13.1	13.2	13.3	13.4	13.5	13.6	13.7	13.8	13.9	5
6	14.0	14.1	14.2	14.3	14.4	14.5	14.6	14.7	14.8	14.9	6
7	15.0	15.1	15.2	15.3	15.4	15.5	15.6	15.7	15.8	15.9	7
8	16.0	16.1	16.2	16.3	16.4	16.5	16.6	16.7	16.8	16.9	8
9	17.0	17.1	17.2	17.3	17.4	17.5	17.6	17.7	17.8	17.9	9
10	18.0	18.1	18.2	18.3	18.4	18.5	18.6	18.7	18.8	18.9	10
11	19.0	19.1	19.2	19.3	19.4	19.5	19.6	19.7	19.8	19.9	11
12	20.0	20.1	20.2	20.3	20.4	20.5	20.6	20.7	20.8	20.9	12
13	21.0	21.1	21.2	21.3	21.4	21.5	21.6	21.7	21.8	21.9	13
14	22.0	22.1	22.2	22.3	22.4	22.5	22.6	22.7	22.8	22.9	14
15	23.0	23.1	23.2	23.3	23.4	23.5	23.6	23.7	23.8	23.9	15
16	24.0	24.1	24.2	24.3	24.4	24.5	24.6	24.7	24.8	24.9	16
17	25.0	25.1	25.2	25.3	25.4	25.5	25.6	25.7	25.8	25.9	17
18	26.0	26.1	26.2	26.3	26.4	26.5	26.6	26.7	26.8	26.9	18
19	27.0	27.1	27.2	27.3	27.4	27.5	27.6	27.7	27.8	27.9	19
20	28.0	28.1	28.2	28.3	28.4	28.5	28.6	28.7	28.8	28.9	20
21	29.0	29.1	29.2	29.3	29.4	29.5	29.6	29.7	29.8	29.9	21
22	30.0	30.1	30.2	30.3	30.4	30.5	30.6	30.7	30.8	30.9	22
23	31.0	31.1	31.2	31.3	31.4	31.5	31.6	31.7	31.8	31.9	23
24	32.0	32.1	32.2	32.3	32.4	32.5	32.6	32.7	32.8	32.9	24
25	33.0	33.1	33.2	33.3	33.4	33.5	33.6	33.7	33.8	33.9	25
26	34.0	34.1	34.2	34.3	34.4	34.5	34.6	34.7	34.8	34.9	26
27	35.0	35.1	35.2	35.3	35.4	35.5	35.6	35.7	35.8	35.9	27
28	36.0	36.1	36.2	36.3	36.4	36.5	36.6	36.7	36.8	36.9	28
29	37.0	37.1	37.2	37.3	37.4	37.5	37.6	37.7	37.8	37.9	29
30	38.0	38.1	38.2	38.3	38.4	38.5	38.6	38.7	38.8	38.9	30
31	39.0	39.1	39.2	39.3	39.4	39.5	39.6	39.7	39.8	39.9	31
32	40.0	40.1	40.2	40.3	40.4	40.5	40.6	40.7	40.8	40.9	32
33	41.0	41.1	41.2	41.3	41.4	41.5	41.6	41.7	41.8	41.9	33
34	42.0	42.1	42.2	42.3	42.4	42.5	42.6	42.7	42.8	42.9	34
35	43.0	43.1	43.2	43.3	43.4	43.5	43.6	43.7	43.8	43.9	35
36	44.0	44.1	44.2	44.3	44.4	44.5	44.6	44.7	44.8	44.9	36
37	45.0	45.1	45.2	45.3	45.4	45.5	45.6	45.7	45.8	45.9	37
38	46.0	46.1	46.2	46.3	46.4	46.5	46.6	46.7	46.8	46.9	38
39	47.0	47.1	47.2	47.3	47.4	47.5	47.6	47.7	47.8	47.9	39
40	48.0	48.1	48.2	48.3	48.4	48.5	48.6	48.7	48.8	48.9	40

Example—If point is 22.6 ft. above grade, how far should it be from center line to be a slope stake point? Ans. from Table 30.6. For same slopes but other widths of roadbed, correct above figures by one-half difference in width of roadbed; thus in example above, for 20 ft. roadbed distance will be  $30.6 + (20 - 16) \div 2$  or 2 ft. added to 30.6 = 32.6. For slopes of 1 on  $1\frac{1}{2}$  see inside of back cover.

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## INDEX

Pg. 1-13 Tunnel Portal Entrance Portal 1-13

6. " Pilot

18-22 Outlet Tower Shaft Ex 18-22

22-23 " " " to Tun Lining.

23-24 " " " Backfill.

33-36 Spillway Ogee Concr Ex.

37-41 " " Below floor.

42-43 " Side Lining Warped See.

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2001-0001

## EL CAPITAN DAM

Excavation in tunnel entrance  
approach and portal structure.

## ~ PORTAL STRUCTURE ~

Below elev. 563

STA	End Area of Rock Section #	Cut & Rock
-----	-------------------------------------	---------------

0+14	31	
	31	4.59

0+10	31	
------	----	--

0+10	36	
		8.00

0+04	36	
------	----	--

0+04	377	
		55.85

0+00	377	
------	-----	--

0+00	202.3	
		243.54

0-41.33	115.9	
---------	-------	--

311<sup>.98</sup>

Computed from planimeter measurements

of cross-sections plotted from field notes

in Field Books # 330 pg. 19-23 &amp; pg. 45-49 and

# 331 pg. 56-58

Comp. by R.E.L

checked by J.W.W.

STA	END AREA OF ROCK SECTION #	CUT & ROCK
-----	-------------------------------------	---------------

0-41.33	128.7	25.38
---------	-------	-------

0-47	113.0	
------	-------	--

0-47	336.2	37.56
------	-------	-------

0-50	340	
------	-----	--

62.<sup>.94</sup>

STA	End Area of Rock Section □	Cd Rd Rock	End Area of Earth Section □	Cut Rd Earth
0+13.5	0	0		
	0.83		12.64	
0+20	6.4	6.4	195	
		13.18		47.83
0+15.8	163		420	
		16.31		26.79
0+14	326.4		406	
		60.12		67.81
0+10	485.2		509.4	
		141.32		142.58
0+04	786.7		773.8	
0+04	940.3		762.5	
		133.02		115.89
0+00	855.5		802.0	
0+00	1460.5		856.0	
	256.51		175.76	
0-05	1309.8		1042.2	
	671.79			

STA	End Area of Rock Section □	Cd Rd Rock	End Area of Earth Section □	Cd Rd Earth
0-05	1309.8	1042.2	1042.2	197.47
		235.01		
0-10	1228.3		1090.5	
		414.93		423.74
0-20	1012.3		1197.7	
		344.78		457.63
0-30	849.5		1273.5	
		280.91		480.91
0-40	667.4		1323.4	
		175.52		391.05
0-48	517.4		1316.2	
0-48	515.8		1447.6	
		38.63		105.56
0-50	527.1		1402.4	
		Total for portal structure	2486.0	2645.66 ✓
			1489.78	

Sta	End A of Rock Section	Cu Ht Rock	Approach —	End A of Earth Section	Cu Ht Earth
0-50	314.3	1281.5		58.20	
0-60	0			1225.65	
0-75		1365.9		1217.87	
0-100		1264.7		281.23	
0-106		1266.4			267.48
0-108	1252.1	93.28		792.04	
0-125	1263.8			137.60	
0-128	1212.9	-		0-275	54
0-145	1128.0			736.95	0
0-158	1050.9			524.55	

Sta	End Area of Earth Section	Cu Vd Earth.
0-161	1027.2	115.45
		514.16
0-175	956	659.04
0-195	823.4	263.23
0-204	756	226.55
0-212	773.2	
0-222	671.2	
		582.00
0-258	201.8	
		80.53
0-275	54	
		20.00
0-295	0	
		7737.51 Cu yd Earth
		58.20 " Rock
		Total rock = $2544 \text{ y}^3$
		Earth = $\frac{7737.51}{2645.66}$
		G.W.C. $10383 \text{ y}^3$

EL CAPITAN DAM  
Excavation in tunnel exit  
approach and portal structure

- PORTAL STRUCTURE -

Rock below ELEV 542.

sta End □' Cu. Yd  
Area

11+67.77 222.7

42.53

11+72.77 236.6

11+72.77 134.8

154.16

12+00.02 170.7

12+00.02 212.6

70.31

12+08.77 221.3

12+08.77 164.4 r

6.07

12+09.77 163.7

12+09.77 685

12+12.77 610 76.67

12+12.77 695 349.74

See line on Rose -

4

Computed from planimeter measurements of  
cross sections plotted from tied notes in  
Field Books # 331 pg. 52-55 & pg. 66 & pg. 69-75  
comp. by R.E.L. Checked by J.W.W.

<u>PORTAL STRUCTURE</u>					
Sta	End □' Area	Cu. Yd Rock	End Area Rock	Cu.Yd earth	(above Elev 542)
11+47.77				0	
				72.36	
11+59.77				325.6	
				34.45	
11+61.77		0		604.5	
				0.84	23.79
11+62.77		45.1		680.3	
11+62.77		145.1		780.2	
11+63.77			48.49		131.23
11+66.77		509.4		991.4	
				24.79	36.66
11+67.77		829.4		988.2	
				186.44	179.26
11+72.77		1184.2		947.7	
				539.70	449.41
11+82.77		1730.2		1479.0	
				800.26	927.16

Sta.	End Area	Cu.Yd	End Area	Cu.Yd	APPROACH				
	Rock	Rock	Earth	Earth.	Sta.	End Area	Cu.Yd	End Area	Cu.Yd.
11+82.77	1730.2	1479.00	927.16	12+12.77	1023.8	315.89	1803.1	636.33	
	800.20 605.41	597.90	12+22.77	682		1633			
11+92.77	1539.0	1749.6			525.08		1289.62		
	551.41	699.60	12+45.77	550.8		1394.8			
12+02.77	1438.6	2028.2			147.38		410.16		
	313.93	461.16	12+53.77	443.9		1373.8			
12+08.77	1320.3	1757.7			290.58		1047.12		
	173.64	263.76	12+74.77	293.2		1318.7			
12+12.77	1023.8	1803.1			145.76		648.93		
	2444.65	2949.58	12+88.77	268.9		1184.2			
Below El. 542	<u>349.74</u>				153.90		782.59		
	2794.39		13+07.77	168.5		1040			
					6480		43742		
			13+19.77	123.1		928.3			
					57.61		471.17		
			13+34.77	84.2		767.9			
					41.80		470.28		
			13+53.77	34.5		568.6			
					18.58		529.41		
			13+82.77	0	1761.38		417.2		6723.03

Sta.	End Area Cu.Yd	End Area Cu.Yd
	Rock Rock	Earth Earth.
13+82.77	0	417.2
		6723 .03
		574.5
14+27.77		272.2
		7514
14+35.77		234.9
		127.70
14+51.77		196
		258.40
15+30	0	
	1761.38	7758.77
	2796.39	2969.58
	4555.77	10708.35

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sta.	sta.	Size of opening		Length	Cu. ft.	Cu.Yd
0+00	2+53	13'	14'	253'	46,046	
2+53	7+20	13'	15"	467'	91,065	
7+20	7+60	13'	18'	40'	9,360	
7+60	8+56	13'	15'	96'	18,720	
8+56	9+64	15'	18'	108'	29,160	
9+64	9+74	13'	15'	10'	1,950	
9+74	11+72.77	13'	14'	198.77'	36,176	
				11 72.77		
					232,477	8,610

Copied from Estimates Nos A & 5

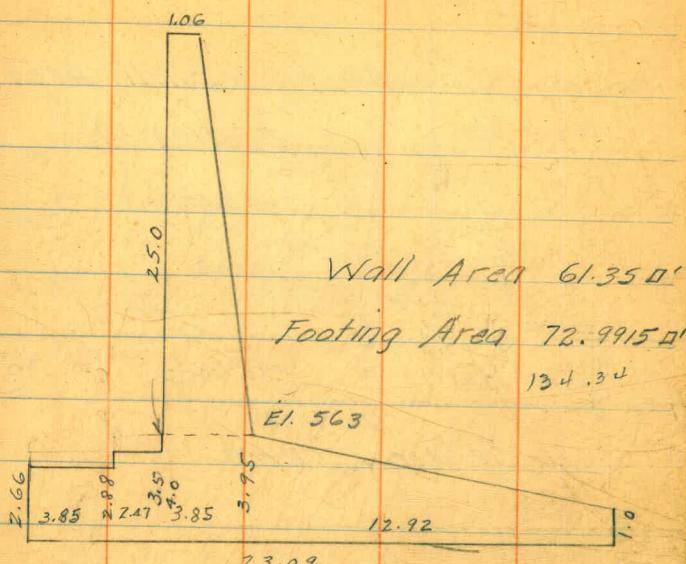
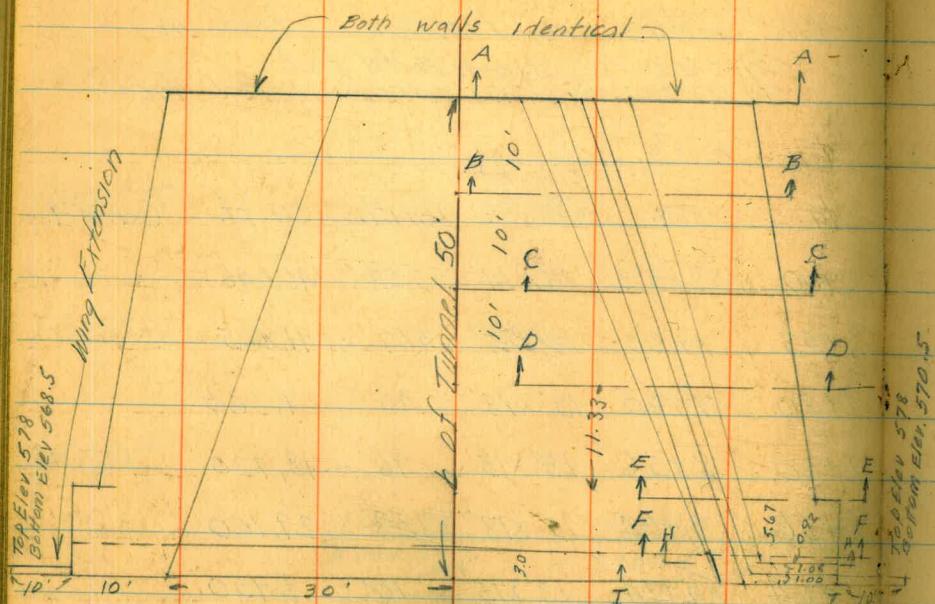
R.E.L

# EL CAPITAN DAM

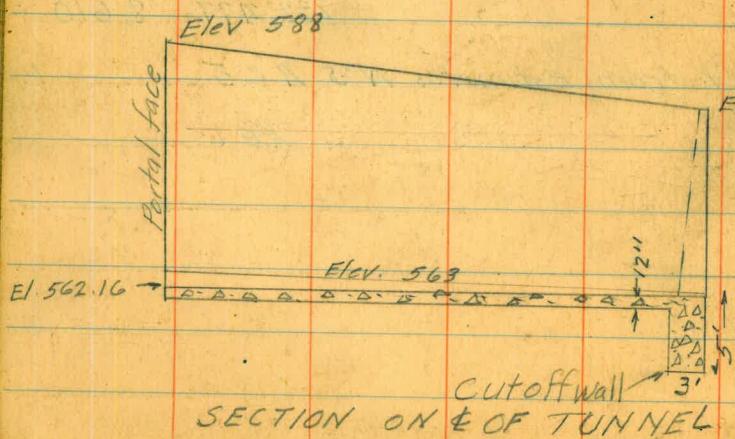
Concrete in tunnel entrance  
portal structure.

Ref. WD 390 and Field Book

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SECTION A-A



EAST CUTOFF WALL

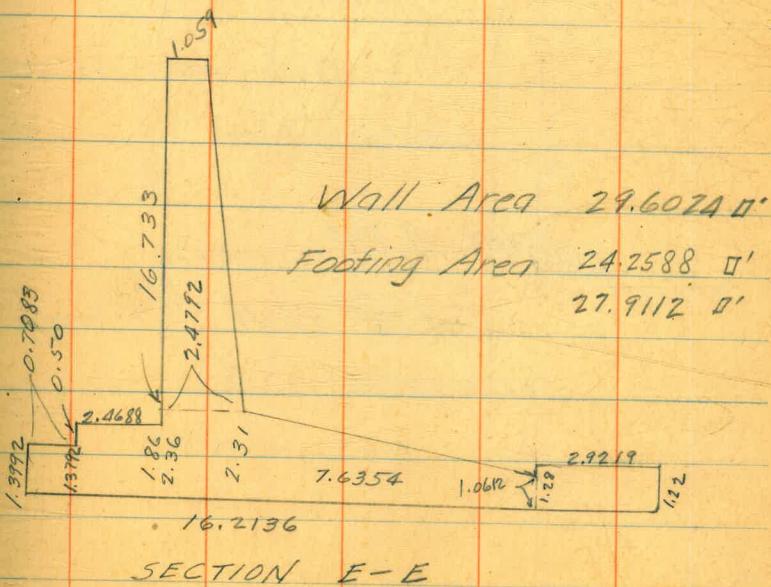
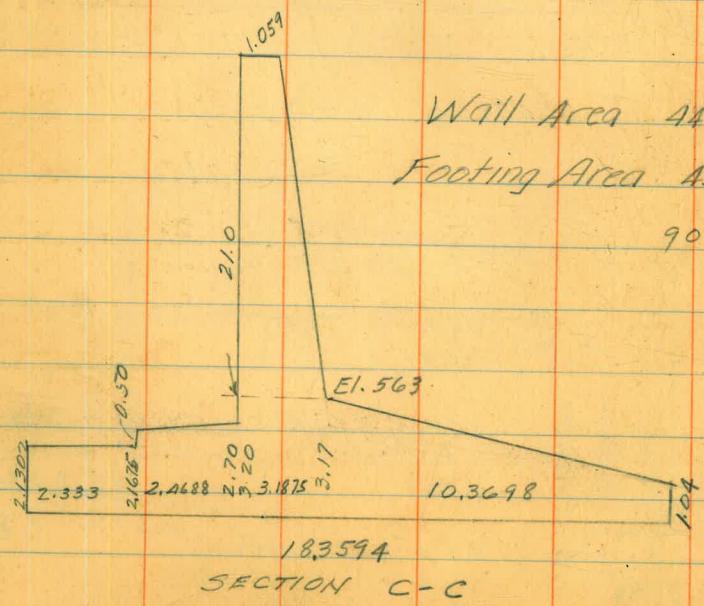
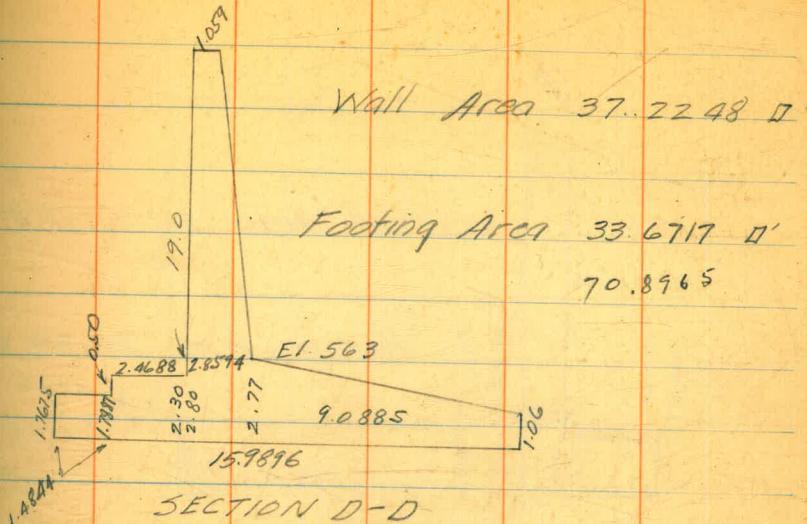
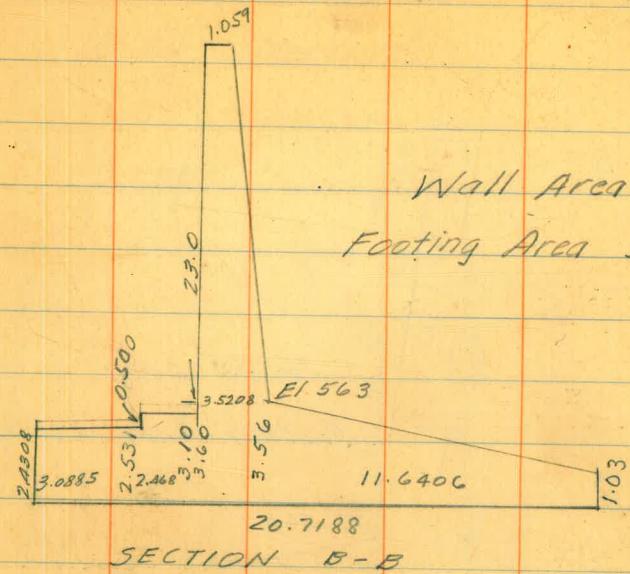


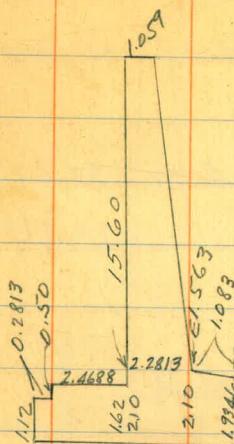
$$\text{GROSS Vol } 40 \times 3 \times 5 = 600 \text{ cuft.}$$

$$\text{Less } 85.232$$

$$\text{Vol. of } \frac{1}{2} \text{ of cutoffwall } \rightarrow 514.768$$

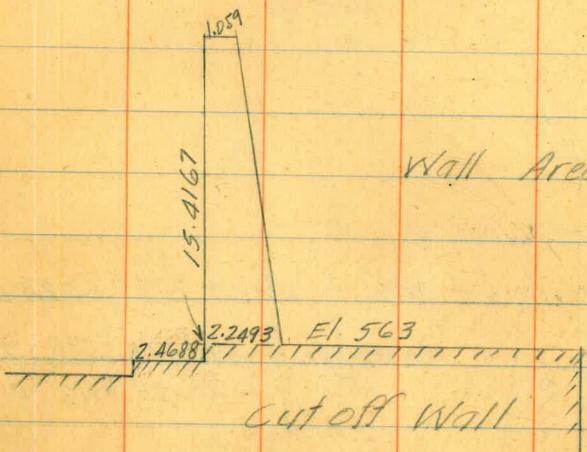
$$\begin{aligned} & 26.48 \\ & 27.53 \\ & 254.01 \\ & 27.00 \times 3 \times 1 = 81.00 \\ & 247 \times 3 \times 0.5 = 3.705 \text{ of Tunnel} \\ & \frac{84.705}{85.232} \\ & \text{Ramp from F-F to G-G } 0.533 \end{aligned}$$



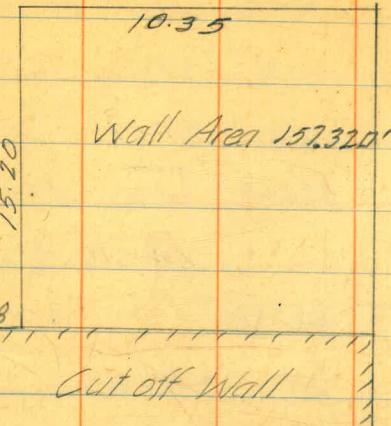


SECTION F-F

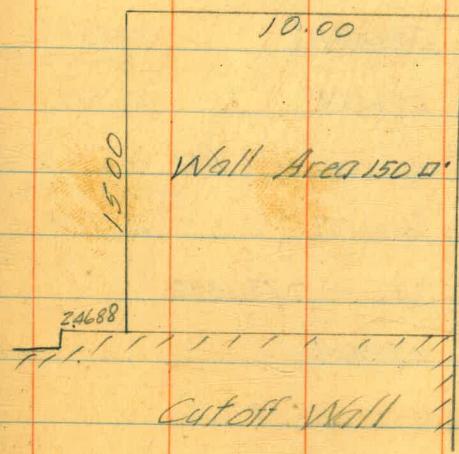
Wall Area 25.5007 ft'



SECTION G-G



SECTION H-H



SECTION I-I

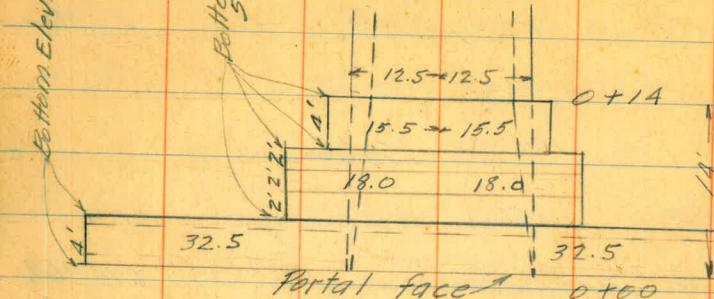
Concrete in retaining walls and footings (see sections Pg 7 to 9)

10

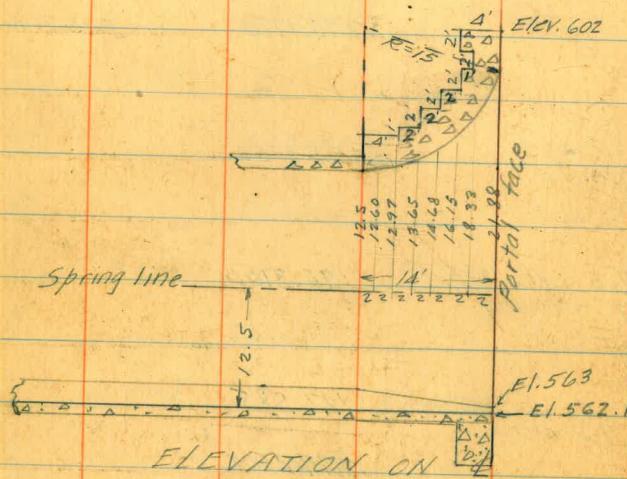
Bottom Elevation 557.16

Bottom Elevation 562

## EL CAPITAN DAM



PLAN OF INTAKE



ELEVATION ON

Concrete in intake and portal structure"

Gross volume of Portal collars

Cu. ft.

$$0+00 \quad 0+04 \quad 65 \times 44.84 \times 4 = 11,658.40$$

$$\text{Less } 65 \times 2. \times 1 \quad 130.00$$

$$11,528.40$$

$$0+04 - 0+06 \quad 36 \times 34 \times 2 = \underline{\underline{2,448.00}}$$

$$0+06 - 0+08 \quad 36 \times 32 \times 2 = \underline{\underline{2,304.00}}$$

$$0+08 - 0+10 \quad 36 \times 30 \times 2 = \underline{\underline{2,160.00}}$$

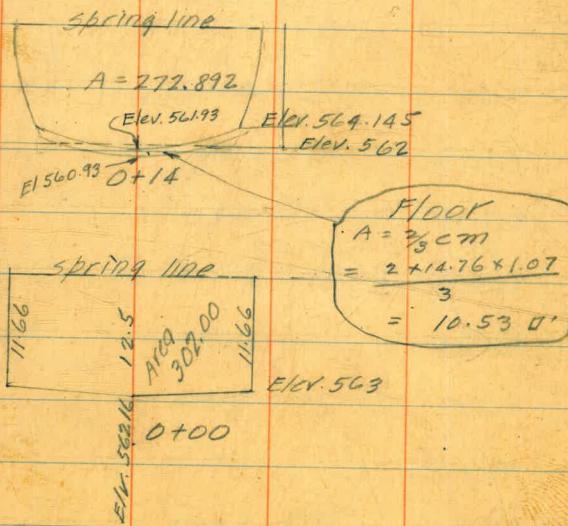
$$0+10 - 0+14 \quad 31 \times 29 \times 4 = \underline{\underline{35,96.00}}$$

$$22,036.40$$

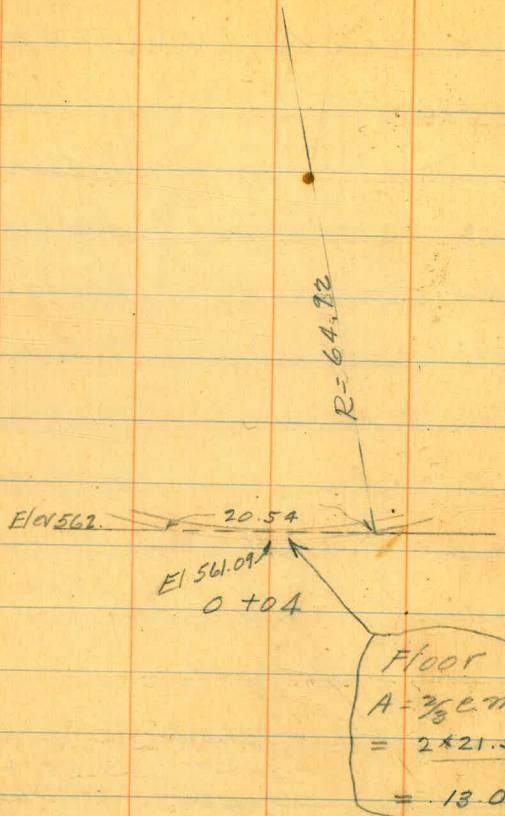
Subtractive Vol below Spring line.

$$1024.244$$

$$\frac{272.892}{302} \times 14 = 1024.244 \text{ Cu. ft}$$
$$\frac{272.892}{287.446} \times 14 = 1024.244 \text{ Cu. ft}$$



Subtractive Volume above spring line  
(see pg 11)



Floor Vol. below elev. 562

$$\frac{10.53 + 13.08}{2} \times 10 = 118.055 \text{ cu ft}$$

$A = 245.44$	$\gamma 3.65$	$0+14$	$A = 336.69$	$0+04$
$A = 247.94$	$\gamma 5.80$	$0+12$	$A = 390.44$	$0+02$
$A = 257.19$	$\gamma 9.38$	$0+10$	$A = 479.94$	$0+00$
$A = 274.19$	$\gamma 1.15$	$0+08$	$0+14$	$\frac{245.44}{2} = 122.72$
			$0+12$	$247.94$
			$0+10$	$257.19$
			$0+08$	$274.19$
$A = 299.94$	$\gamma 2.18$	$0+06$	$0+06$	$299.94$
			$0+04$	$336.69$
			$0+02$	$390.44$
			$0+00$	$\frac{479.94}{2} = 239.97$
			$\frac{247.94}{2} = 123.97$	$2169.08$
				$4338.16 \text{ cu ft}$

### Cut Off Wall

$$\text{End Area "A"} = 295.0624$$

$$\text{"B"} = 290.8054$$

$$\frac{5858.678}{2}$$

$$\times 3 = 878.8017 = 32.5482 \text{ cu yds}$$

Volume in Entrance Portal footing = 32.55 cu yds

### Floor Area

$$\text{at "A"} 55.0624 \text{ sq'}$$

$$2.4688 \text{ sq'}$$

$$\text{Area at "A"} = 57.5312 \text{ sq'}$$

$$57.5312$$

$$22.5312$$

$$80.1524$$

$$\frac{2}{2} \times 50 = 2015.6 \text{ cu ft} =$$

$$= 74.6518 \text{ cu yds.}$$

$$\text{at Portal Face } 20.0624 \text{ sq'}$$

$$2.4688$$

$$22.5312$$

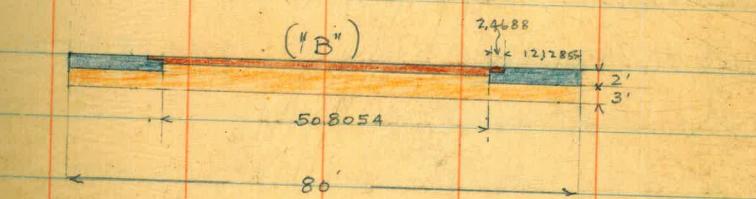
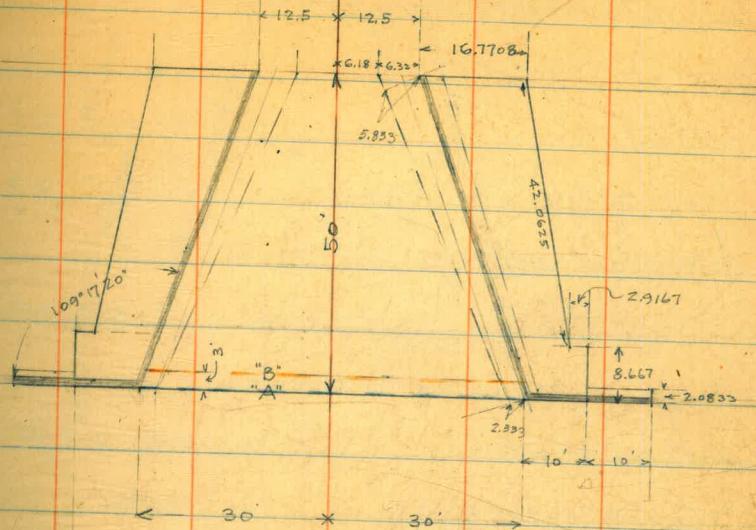
Volume in Entrance Portal floor = 74.65 cu yds

Note - Totals below include all concrete east of 0+14.

### SUMMARY

Walls	176.93	Cubic Yds.
Footings & Cutoff	187.69	
Portal Structure	506.44	
Floor	79.02	
Total in entr. Portal	950.08	
Plus $\frac{B_0 = 4.8 \text{ y}^3}{27}$	4.8	
See page 11. Portal floors $\frac{950.08 - 4.8}{27} = 86.20$		
Portal Structure: $86.8.68 \text{ y}^3$		

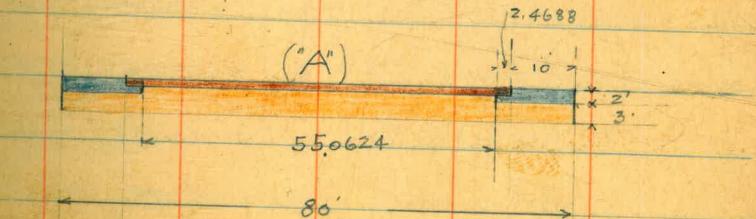
### EL CAPITAN DAM Entrance Portal Footing



$$\text{"B" Areas } 80 \times 3 = 240.0000$$

$$50.8054 \times 1 = 50.8054$$

$$\text{Area cut off wall at "B" } = 290.8054 \text{ sq'}$$



$$\text{"A" Areas } 80 \times 3 = 240.0000$$

$$55.0624 \times 1 = 55.0624$$

$$\text{Area cut off wall at "A" } = 295.0624 \text{ sq'}$$

0 10 20 30 40 50

Tunnel Outlet Approach

14

Tunnel Outlet Approach Concrete  
Apron Cut Off Wall

$$69.03' \times 3' \times 9' = 69.030 \text{ cu ft} \checkmark$$

$$\text{Floor } (50+25) \times 40 \times 1 = 1500 \text{ cu ft} = 55.555 \text{ cu ft} \checkmark$$

$$\text{Sump } \frac{1}{2} \times 38.5 \text{ ft}^3 = 1.425 \text{ cu ft} \checkmark$$

South wall footing under floor Section

$$\frac{38.763 \times 4.25 \times 3.25}{27} = 19.830 \text{ cu ft} \checkmark$$

South Wall footing under Ret. Wall and Counterforts

$$\frac{35.2344 \times 12.75 \times 3.25}{27} = 54.075 \text{ cu ft} \checkmark$$

$$\frac{(10.1354 + 7.1620) \times (9.75 \times 3.25)}{27} = 10.154 \text{ cu ft} \checkmark$$

$$\frac{(11.8646 + 10.3021) \times (5 \times 3.25)}{27} = 6.670 \text{ cu ft} \checkmark$$

South Retaining Wall

$$\frac{33.33 \times (1+1.5)}{2} = 41.66 \text{ cu ft}$$

$$\frac{29 \times (1+1.5)}{2} = 36.25 \text{ cu ft}$$

$$\frac{(41.66 + 36.25) \times 41.9063}{2} = 1632.46 \text{ cu ft} \checkmark$$

$$\frac{36.25 \times 18.765}{27} = 25.194 \text{ cu ft} \checkmark$$

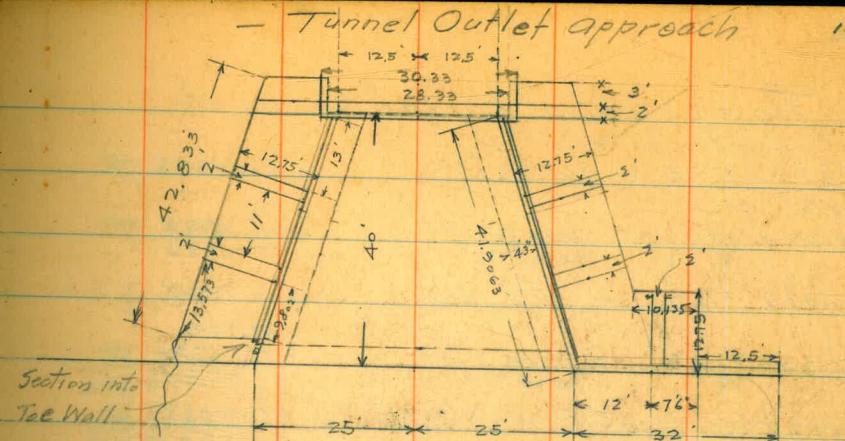
$$\text{Counterforts } A \frac{(11.25 \times 33.75) \times \frac{2}{27}}{2} = 14.062 \text{ cu ft} \checkmark$$

$$B \frac{(11.25 \times 33.56) \times \frac{2}{27}}{2} = 13.983 \text{ cu ft} \checkmark$$

$$C \frac{(11.25 \times 31.89) \times \frac{2}{27}}{2} = 13.287 \text{ cu ft} \checkmark$$

$$D \frac{(11.25 \times 30.42) \times \frac{2}{27}}{2} = 12.675 \text{ cu ft} \checkmark$$

$$E \frac{(11.25 \times 29) \times \frac{2}{27}}{2} = 12.083 \text{ cu ft} \checkmark$$



$$F \text{ Counterforts } \frac{(11.13 \times 33.37) \times \frac{2}{27}}{2} = 13.760 \text{ cu ft}$$

$$G \frac{(11.13 \times 33.37) \times \frac{2}{27}}{2} = 13.760 \text{ cu ft}$$

North Retaining Wall Footings under floor section

$$\frac{38.7736 \times 4.25 \times 3.25}{27} = 19.836 \text{ cu ft} \checkmark$$

$$\frac{(37.5937 + 37.801) \times (12.75 \times 3.25)}{27} = 57.855 \text{ cu ft} \checkmark$$

$$\frac{(10.3021 + 11.8646) \times (5 \times 3.25)}{27} = 6.670 \text{ cu ft} \checkmark$$

North Retaining Wall

$$33.37 \frac{(1+1.5)}{2} = 41.71 \text{ cu ft}$$

$$34 \frac{(1+1.5)}{2} = 42.50 \text{ cu ft}$$

$$\frac{(41.71 + 42.50) \times 37.801}{27} = 58.949 \text{ cu ft} \checkmark$$

Section into West Toe Wall

$$34 \times 1.75 \times 1 \frac{1.574}{27} = 1.574 \text{ cu ft} \checkmark$$

$$\text{Total Concrete } \frac{540.888}{27} = 55.555 \text{ cu ft} \checkmark$$

$$\text{Less floor approach Concrete } \frac{55.555}{27} = 486.333 \text{ cu ft} \checkmark$$

### Tunnel Outlet Portal Concrete

Cut off at face of Portal		
✓ 28.33 x 5 x 5 = 708.25 ft <sup>3</sup>	= 26.2340 y <sup>3</sup>	26.2340
Portal Wall		
✓ 28.33 x 35.83 x 2 = 2030.128 ft <sup>3</sup>	= 75.1898 y <sup>3</sup>	77.2884 ✓
36.83 ✓ 2086.788	77.2884 ✓	
✓ 30.33 x 3 x 2 = 182 ft <sup>3</sup>	= 6.7407 y <sup>3</sup> ✓	6.7407
✓ 28.33 x 28 x 3 = 2380 ✓ ft <sup>3</sup>	= 91.2856 ✓	91.2856 ✓
✓ 2464.71	= 88.1481 ✓	201.5487 ✓
Less Water way 518.328 x 5 = 2591.64 ft <sup>3</sup> =	196.3126 ✓	95.9867 ✓
	y <sup>3</sup>	y <sup>3</sup>
Less Invert 5 x 22.003 = 110.015 ft <sup>3</sup> =	100.3260 ✓	105.5620 ✓
	y <sup>3</sup>	y <sup>3</sup>
Total Tunnel/Outlet Portal Wall	96.2510 ✓ y <sup>3</sup>	4.0746
5' of Invert	4.0750 y <sup>3</sup>	101.4874 ✓

### Summary. Portal structure concrete

Includes all tunnel concrete west of Sta. 11+67.77.

✓ 485.333 y<sup>3</sup>

✓ 101.487

✓ 586.820 y<sup>3</sup>

✓ 1.425 Sump in portal floor.  
585.395 y<sup>3</sup> Total Item 29.

### Portal structure floor.

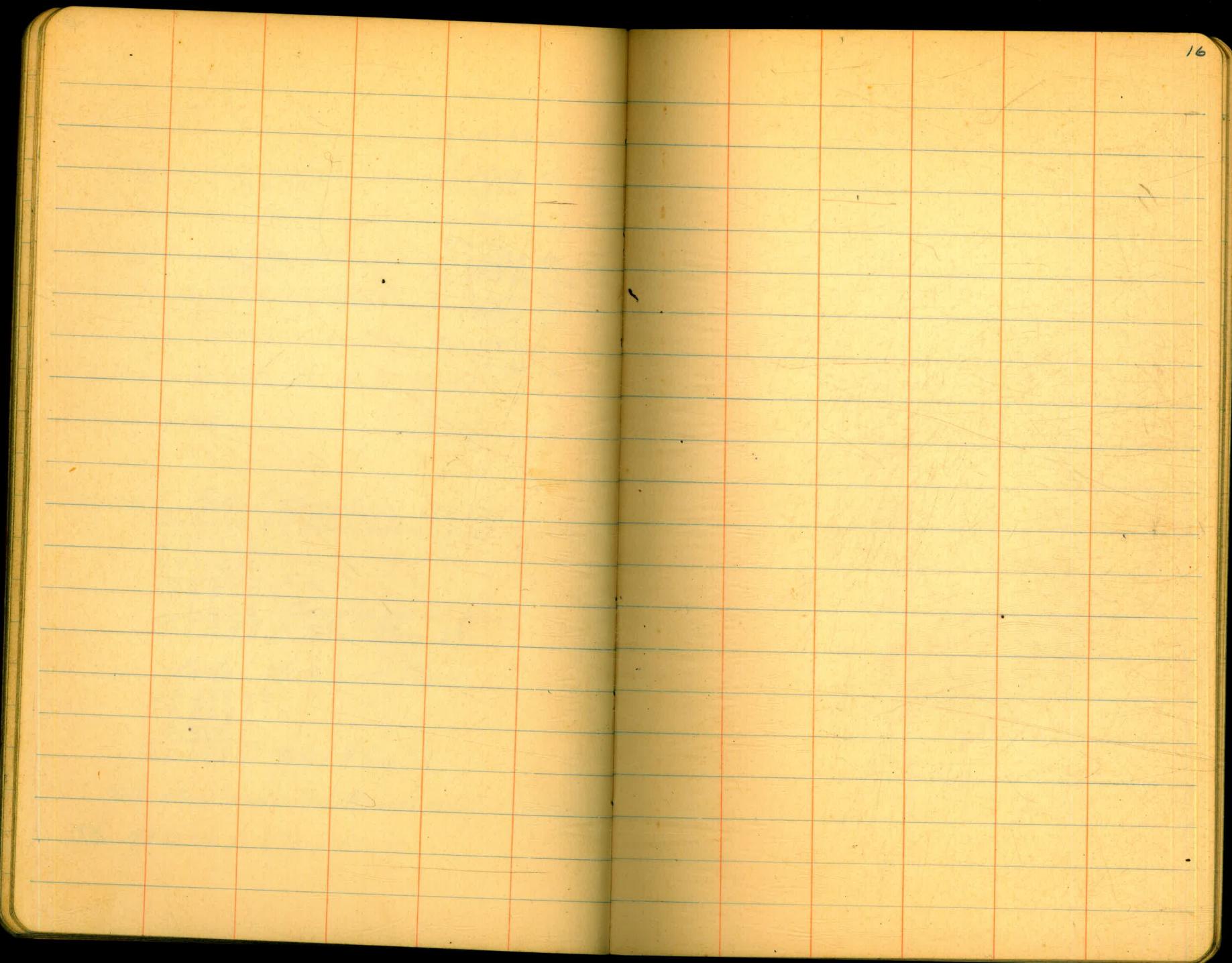
✓ 55.555 y<sup>3</sup>

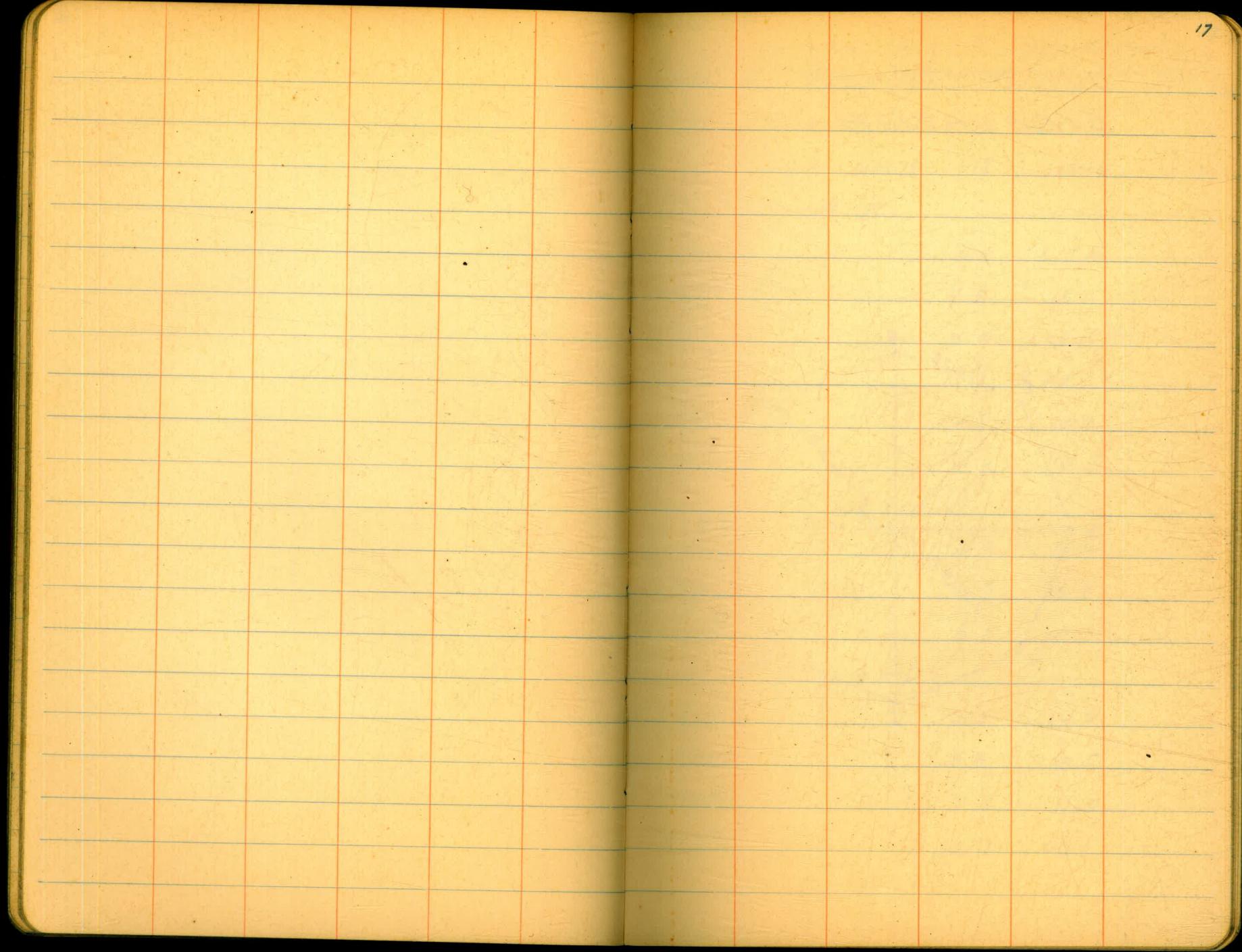
✓ 1.425

✓ 4.075 ✓

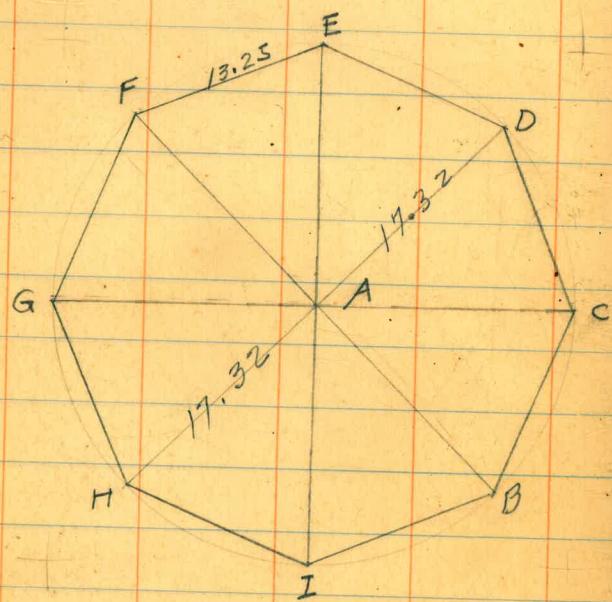
✓ 61.055 y<sup>3</sup> Total Item 28.

Note - Floor includes tunnel invert from  
Sta. 11+67.77 to Sta. 11+72.77.





OUTLET TOWER SHAFT  
EXCAVATION



X sections top of outlet tower  
shaft Feb 25-33 Simpson super  
Rammen.

18

BM 12.28 614.28 602.00

5.68 618.78 1.18 613.10

A-B

A=0+00	0.4	618.4
+02 $\frac{1}{2}$	0.8	180
+03	3.4	154
+07	1.7	17.1
+14	4.5	14.3
+17 $\frac{3}{4}$	4.3	14.5

A-C

0+01	4.2	14.6
+10	6.3	12.5
+17 $\frac{3}{4}$	8.1	10.7

A-D

0+01	4.5	14.3
+06	5.3	13.5
+10 $\frac{1}{2}$	7.8	11.0
+17 $\frac{3}{4}$	10.7	08.1

618.78

## A-E

0+02

49 613.9

+10

7.5 11.3

+17<sup>3</sup>

9.9 08.9

625.44

## A-I

36 621.8

7.1 18.3

6.2 19.2

## A-F

0+04

1.7 17.1

+05

5.4 13.4

+12

4.6 14.2

+17<sup>3</sup>

5.5 13.3

T.P. 8.82 625.44 2:16 616.62

## A-G

0+05

3.6 21.8

+12

6.8 18.6

+17<sup>3</sup>

5.9 19.5

## A-H

0+05

0.4 25.0

+07<sup>5</sup>

+0.3 25.7

+12<sup>5</sup>

2.0 23.4

+14<sup>5</sup>

5.6 19.8

+17<sup>3</sup>

+0.9 26.3

Original ground to Elev. 605

A - B

<u>C 13.4</u>	<u>83</u>	<u>C 13.0</u>	<u>5.85</u>	<u>C 10.4</u>	<u>15</u>	<u>C 12.1</u>	<u>75</u>	<u>C 9.3</u>	<u>32.7</u>	<u>C 9.5</u>
<u>0</u>		<u>2.5</u>		<u>3.0</u>		<u>7</u>		<u>14</u>		<u>17.3</u>

Area

191.55

<u>C 13.4</u>	<u>11.5</u>	<u>C 9.6</u>	<u>11</u>	<u>C 7.5</u>	<u>48</u>	<u>C 5.7</u>
<u>0</u>		<u>1</u>		<u>10</u>		<u>17.3</u>

136.70

1560.85 = 11.27 =

8X17.3 Average cut

<u>C 13.4</u>	<u>11.35</u>	<u>C 9.5</u>	<u>14.5</u>	<u>A - D</u>	<u>32.6</u>	<u>C 6.0</u>	<u>30</u>	<u>C 3.1</u>
<u>0</u>		<u>1</u>		<u>6</u>		<u>10.5</u>		<u>17.3</u>

119.40

Area of base = 848.32 ft'

<u>C 13.4</u>	<u>11.2</u>	<u>C 8.9</u>	<u>6.8</u>	<u>C 6.3</u>	<u>31.3</u>	<u>C 3.9</u>
<u>0</u>		<u>2</u>		<u>10</u>		<u>17.3</u>

120.4

848.32 X 11.27 =

27

<u>C 13.4</u>	<u>5.10</u>	<u>C 12.1</u>	<u>10.25</u>	<u>A - F</u>	<u>0.84</u>	<u>6.1</u>	<u>C 9.2</u>	<u>16.4</u>	<u>C 8.3</u>
<u>0</u>		<u>4</u>		<u>5</u>		<u>12</u>		<u>17.3</u>	

169.25

354.1 Cubic. Yds.

<u>C 13.4</u>	<u>15.5</u>	<u>C 16.8</u>	<u>10.6</u>	<u>C 13.6</u>	<u>7.4</u>	<u>C 14.5</u>
<u>0</u>		<u>5</u>		<u>12</u>		<u>17.3</u>

256.40

A - H

<u>C 13.4</u>	<u>0.20</u>	<u>C 20.7</u>	<u>C 18.4</u>	<u>C 19.8</u>	<u>21.3</u>
<u>0</u>	<u>5</u>		<u>7.5</u>	<u>12.5</u>	<u>14.5</u>

315.85

A - I

<u>C 13.4</u>	<u>16.8</u>	<u>13.3</u>	<u>14.2</u>
<u>0</u>	<u>5</u>	<u>11</u>	<u>17.3</u>

252.40

Total Area. 1560.85

X-Sections bottom of shaft  
After completion.

21

B.M -350 559.0 562.5

$A =$   
 $0 + 00$

+17

$+17^{\frac{3}{4}}$

$0 + 16\frac{5}{8}$

$0 + 17\frac{3}{4}$

$0 + 16\frac{5}{8}$

$0 + 17\frac{3}{4}$

$0 + 02$

+16

$+17^{\frac{3}{4}}$

$0 + 09$

+17

$+17^{\frac{3}{4}}$

A-B

6.5	552.5
4.9	54.1
3.5	55.5

A-C

5.7	53.9
3.5	55.5

A-F

4.7	54.3
3.5	55.5

A-G

5.6	53.4
4.4	54.6
3.5	55.5

A-H

4.9	54.1
5.4	53.6
3.5	55.5

559.0

A-I

$0 + 16$

$+17^{\frac{3}{4}}$

5.1	553.6
3.5	55.5

Excavation from Elevation 605 to bottom.

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-B

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-C

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-D

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-E

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-F

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-G

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-H

$\begin{array}{r} 52.5 \\ \hline 0 \end{array}$  A-I

Area.  
893.96

900.13

907.91

894.94

896.43

934.03

889.63

896.78

7213.81

outlet tower shaft excav  
(cont'd)

Total area = 7213.81

$$\text{Average height} = \frac{7213.81}{8 \times 17.32}$$

$$= 52.06'$$

Area of base = 848.32 ft'

$$\text{Vol.} = \frac{848.32 \times 52.06}{27}$$

$$= 1635.68 \text{ cu.yd.}$$

From p. 20

Total shaft excav. 1719.78 Cu.Yd.

$$\begin{array}{r} 193.35 \\ + 9.63 \\ \hline 1922.76 \end{array}$$

22

EXCAVATION BETWEEN  
TOWER SHAFT AND TUNNEL

LINING

Elev.	Planimeter Reading	Area	Mean Height	Cu.Yd.
561	3595	524.98		
562	3532	515.78	520.38	520.38
563	3436	501.76	508.77	508.77
565	3123	456.05	478.90	957.80
			520.57	2102.85
570	2637	385.08	376.90	1130.70
573	2525	368.72		
				5220.50
				193.35 Cu.Yd.

Note: Areas were measured by  
planimeter from drawing to scale

of 1" = 3'

Ref. WD. 423 WD. 414 W.D. 383

EXCAVATION FOR ENTRANCE  
ADDIT

Elev 561 to Elev 573

$$16.1 \times 6 \times 12 = 1159.2 \text{ Cu.Pt}$$

Note - This quantity included in <sup>47.93 Cu.Yd</sup>  
193.35 Cu.Yd  
on previous page.

Elev 573 to Elev 576

$$14.45 \times 6 \times 3 = 260.1 \text{ Cu.Pt}$$

$$= 9.63 \text{ Cu.Yd}$$

OUTLET TOWER BACK-  
FILL Elev 561.5 to Elev 605

Volumes displacing backfill.

Concrete on south Elev 561.5  
to Elev 573

Bottom area	155.96 ft'
Top "	159.46 ft'
Mean	157.71
height =	11.5 Pt.

$$157.71 \times 11.5 = 1813.66 \text{ Cu.Pt}$$

Well displacement Elev 561.5  
to Elev 576

Bottom area	42.20 ft'
Top "	43.08 ft'
Mean	42.64
Height =	14.5'

$$42.64 \times 14.5 = 618.28 \text{ Cu.Pt}$$

Elev 576 to Elev 605

Bottom area =	37.24 ft'
Top "	41.18 ft'
Height =	29.'

(Jack Pill cont)

$$39.21 \times 29 = 1173.09 \text{ cu. ft.}$$

Displacement of barrel of  
tower Elev 561.5 to Elev 605

$$\frac{\pi}{3} [(10.05)^2 + (9.40)^2 + (10.05 \times 9.40)] = 12,929.37 \text{ cu. ft.}$$

Volume of octagon shaft.

Elev 561.5 to Elev 605

$$\text{Area of base} = 848.32 \text{ ft}^2$$

$$\text{Height} = 43.5'$$

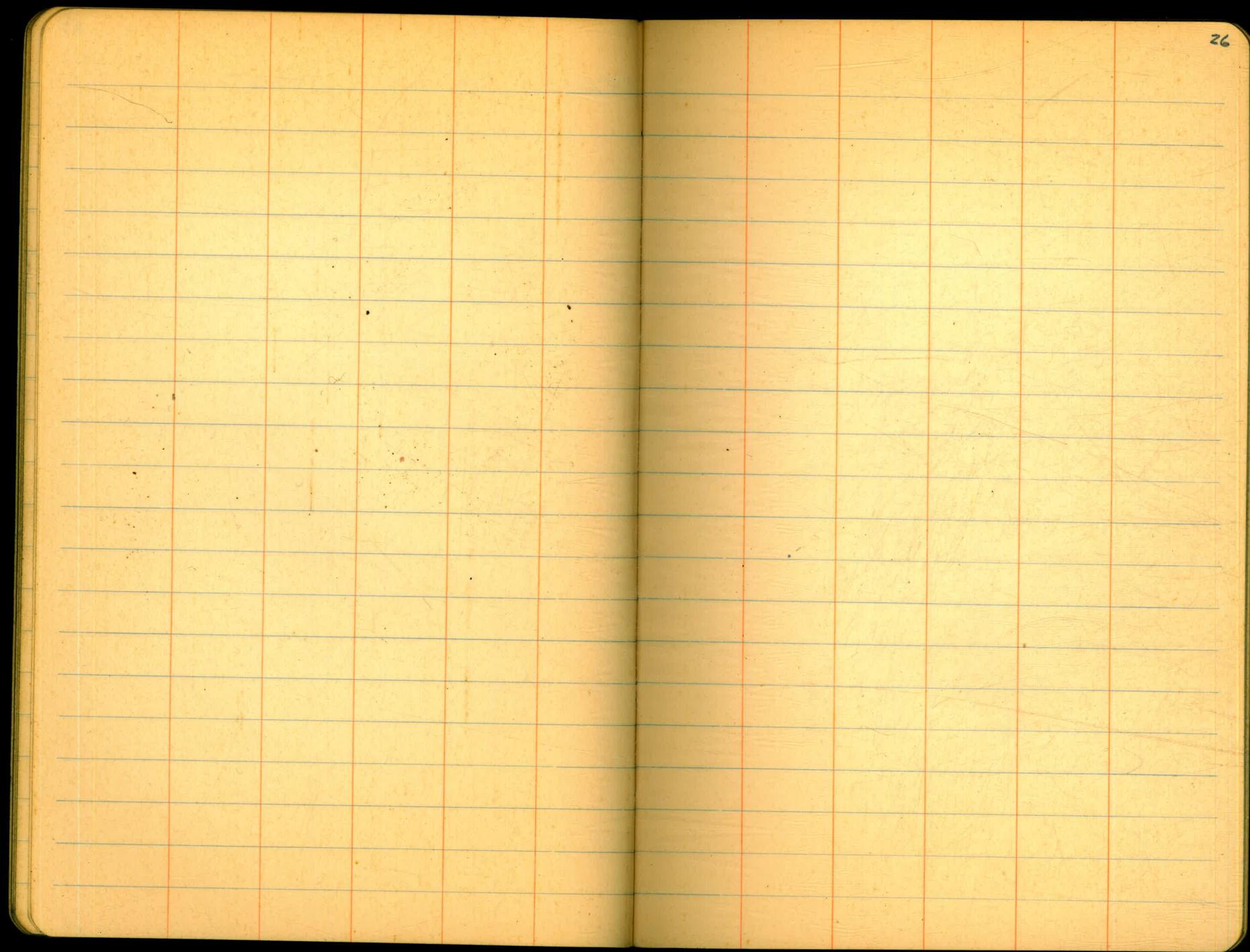
$$848.32 \times 43.5 = 36,901.92 \text{ cu. ft.}$$

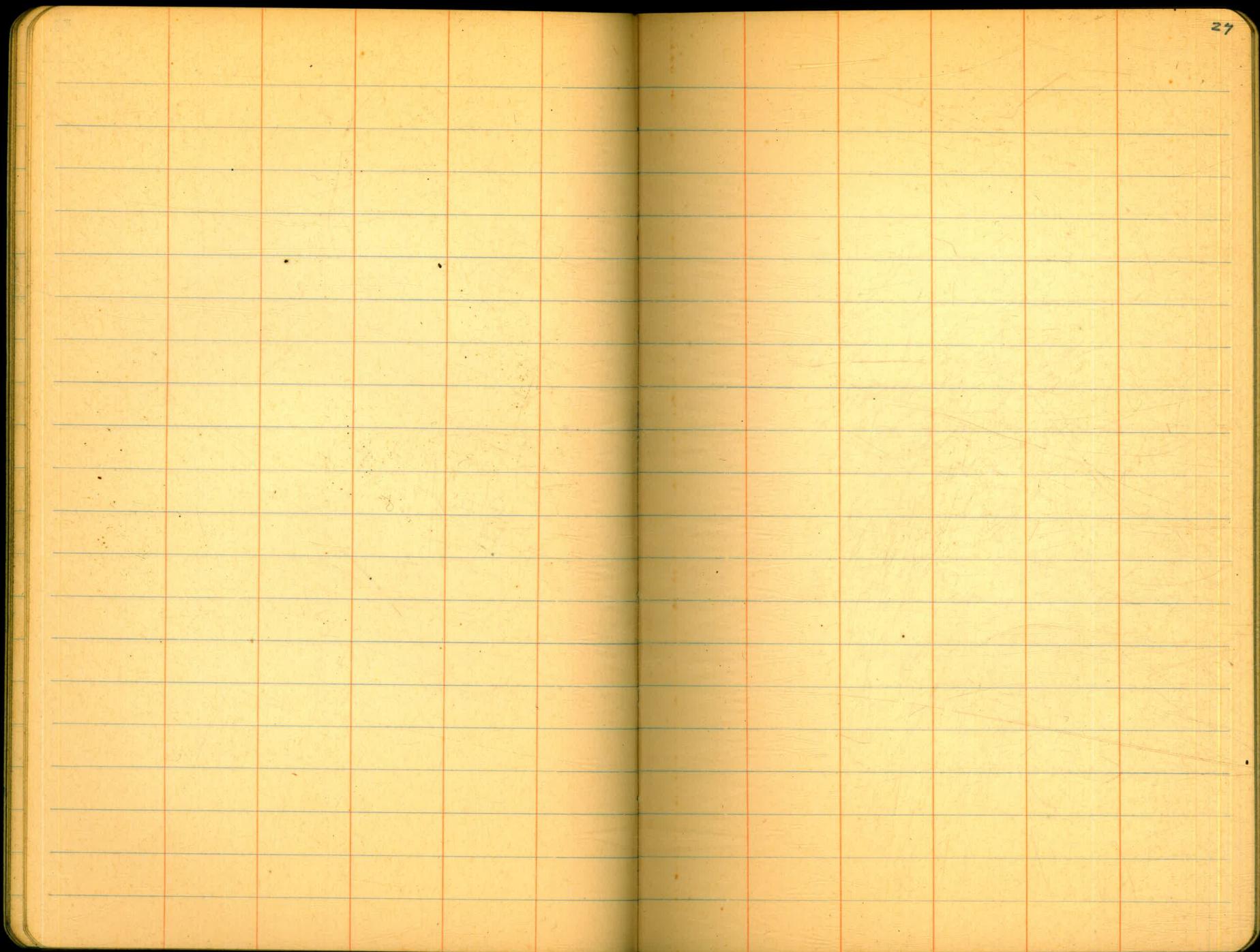
$$\text{Less total displacement } \underline{16,534.40}$$

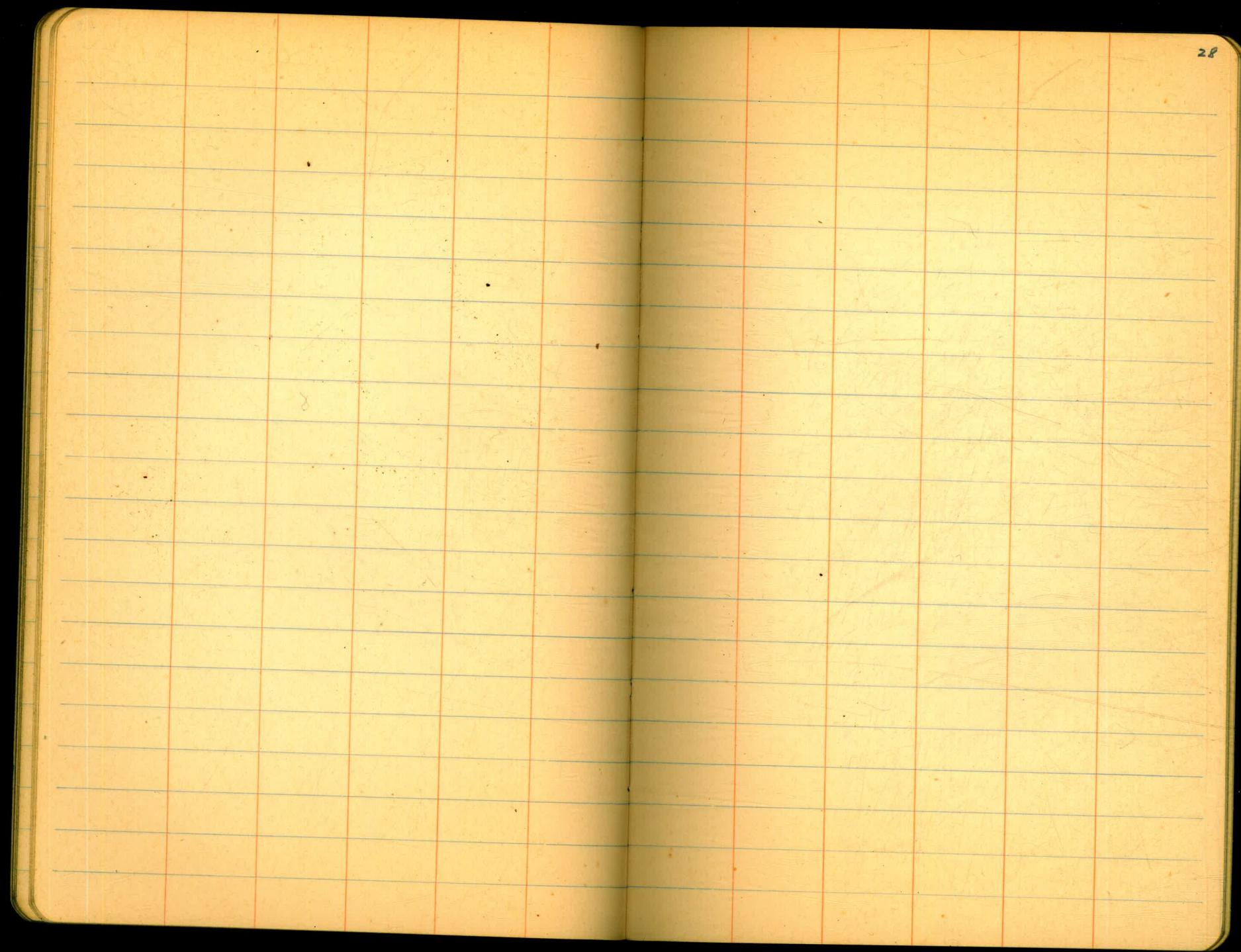
$$20,367.52$$

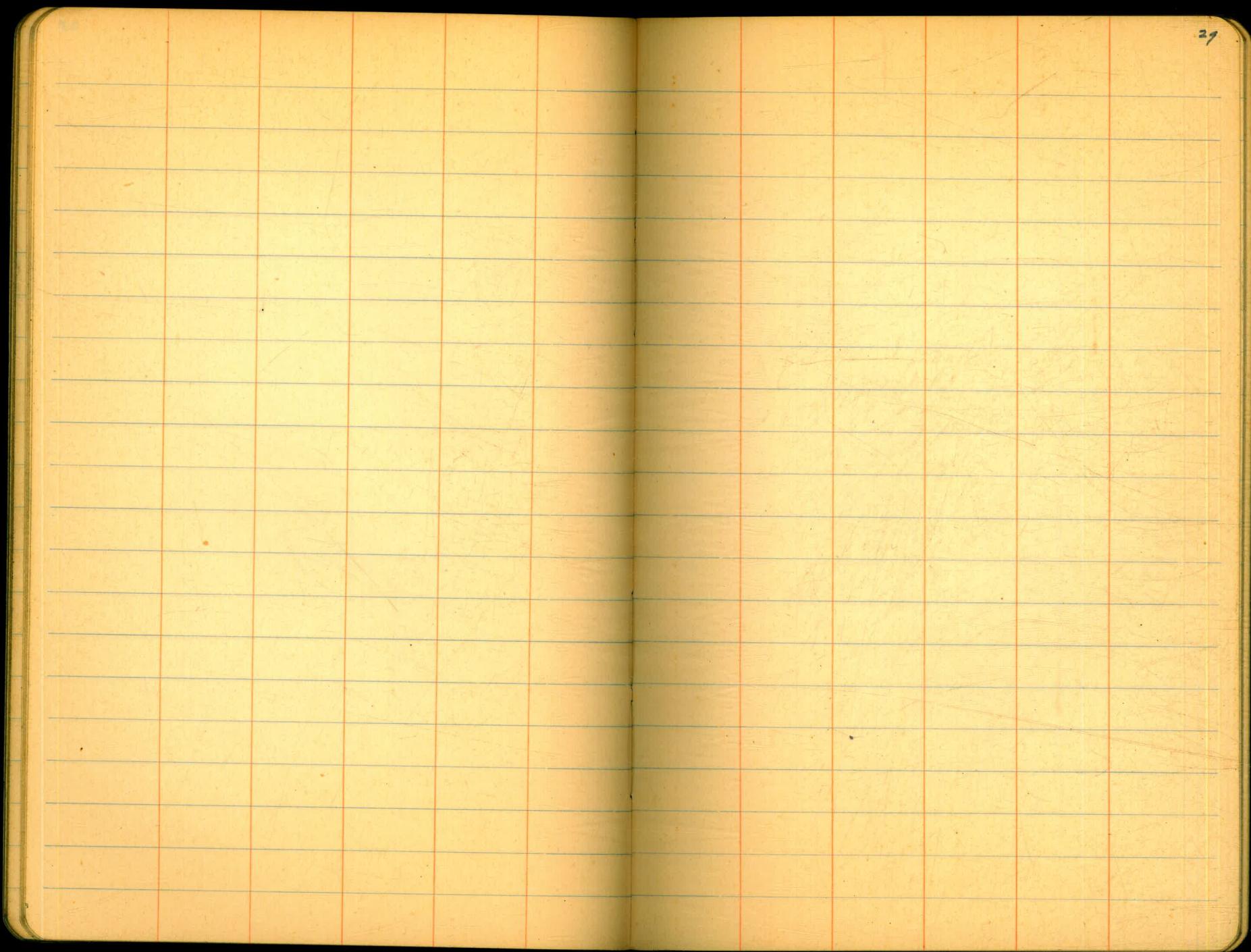
$$= 754.35 \text{ cu. yd.}$$

OUTLET TOWER CONCRETE



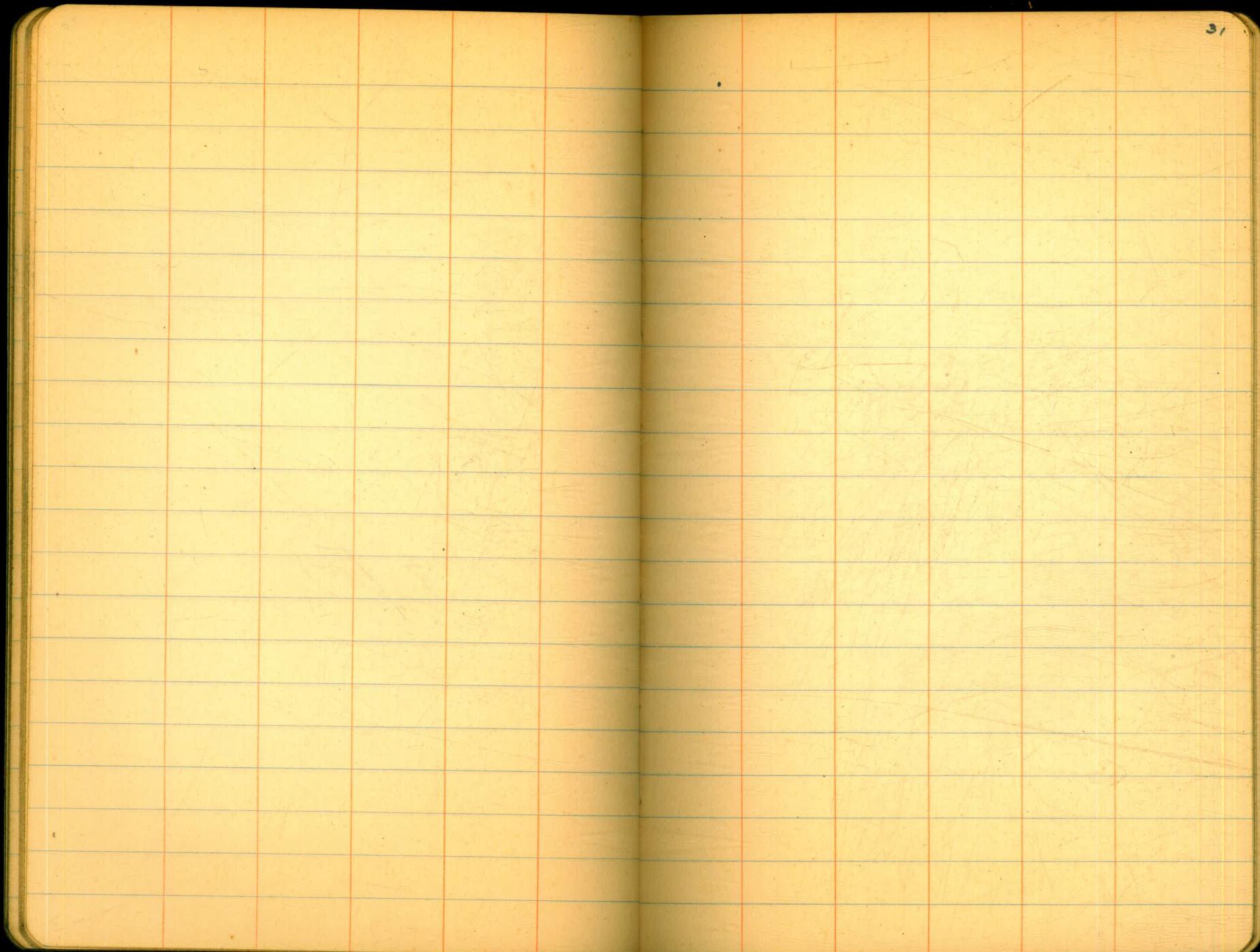


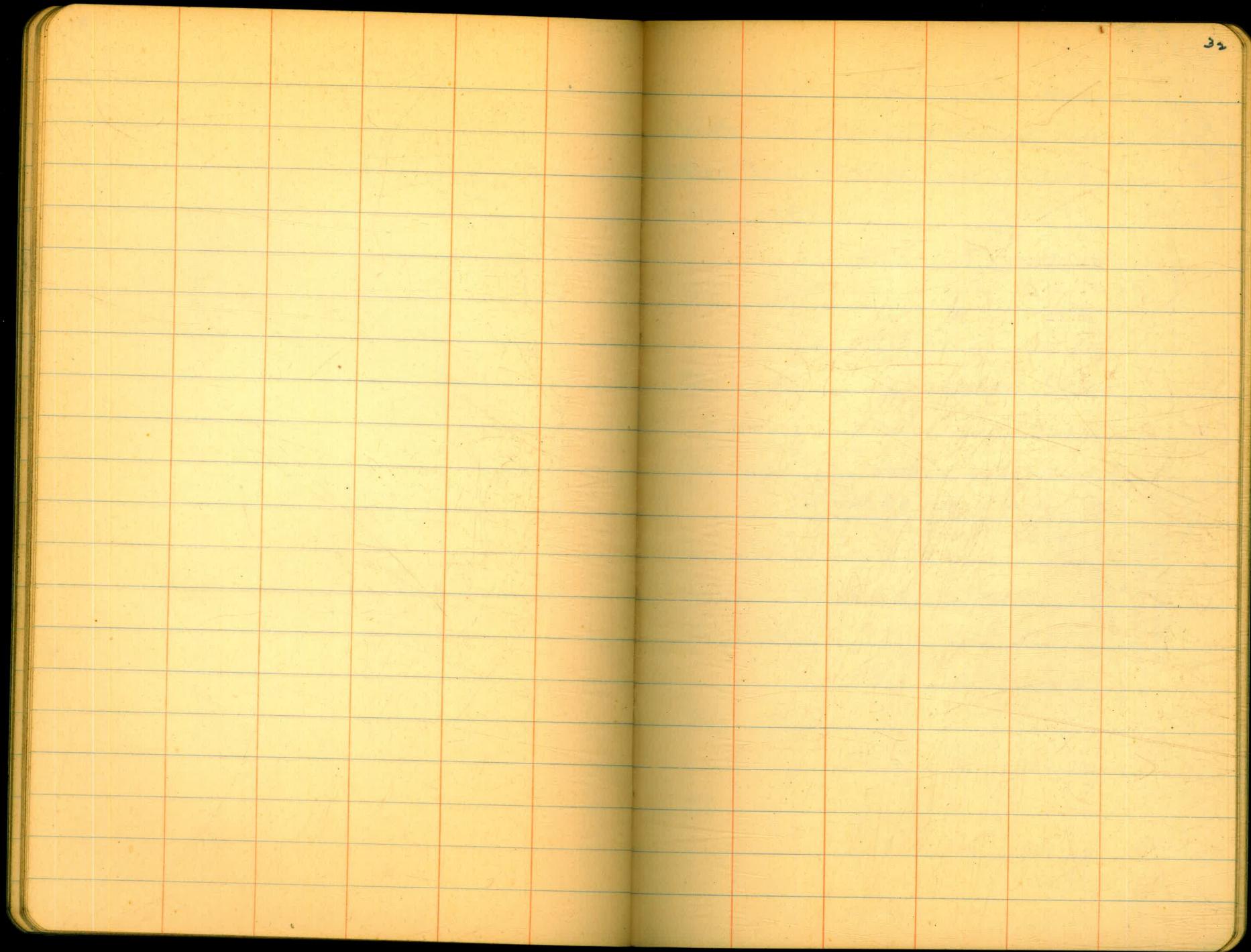




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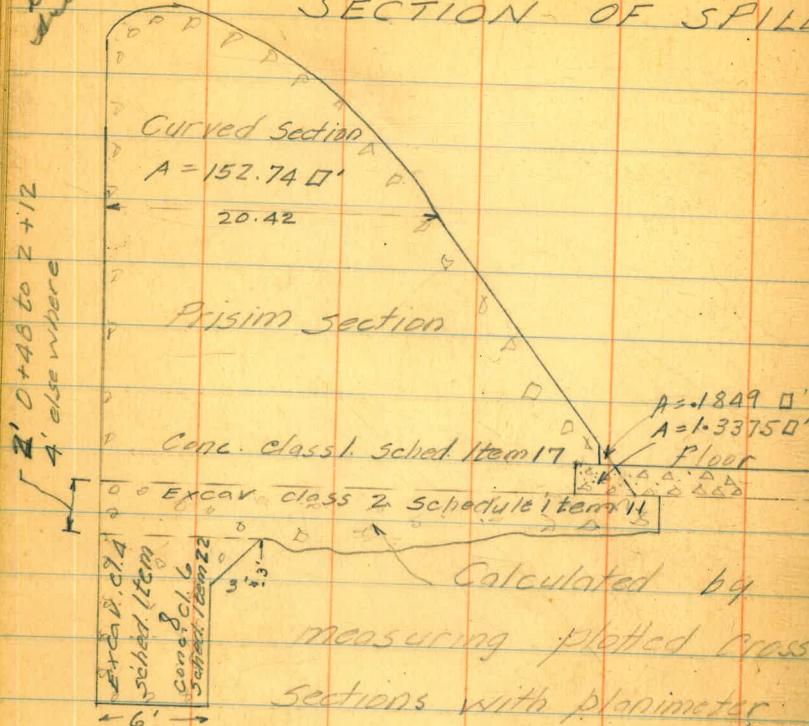




Note 1/2/23  
Excav. 20% above  
the calculated from bed  
to begin.

# CONCRETE AND EXCAVATION IN OGEE

## SECTION OF SPILLWAY sta. Area II Mean Dist. Cu.ft.



Concrete cl. 6 Schedule Item 22

Excavation cl. 4 schedule item 8

sta.	Area II	Mean Dist.	Cu.ft.
0+00	57.9'		
+08	58.8' 8'	470.40'	
+10	59.7'		
+20	59.4' 12'	712.80'	
+26	60.0' 6'	360.00'	
+31	60.9'		
+31	56.1'		
+42	58.5' 5'	292.50'	
+42	54.6' 11'	600.60'	
+46	53.1'		
+46	53.1'	212.40'	
+48	53.1'		
+48	57.6' 2'	115.20'	
+54	62.1'		
+64	65.1' 16'	1041.60'	
+64	66.9' 10'	669.00'	

Sta	Area	Mean	dist.	Cu.Pt.
0+74		65.7		
		65.1	8	520.80
+82		64.5		
		67.8	14	949.20
+96		71.1		
		67.5	24	1620.00
1+20		63.9		
		63.9	9	575.10
+29		63.9		
-		63.6	11	699.60
+40		63.3		
		61.8	15	929.00
+55		60.3		
		59.4	13	772.20
+68		58.5		
		61.2	24	1468.80
+92		63.9		
		65.7	24	1576.80
2+16		67.5		
		66.6	24	1598.40

Sta	Area	Mean	dist.	Cu.Pt.
2+40		65.7		
		72.3	24	1735.20
+64		78.9		
		69.6	6	417.60
+70		60.3		
		60.3	6	361.80
+76		60.3		
		63.3	7	443.10
+83		66.3		
		66.0	5	330.00
+88		65.7		
		65.4	5	327.00
+93		65.1		
		64.5	7	457.50
3+00		63.9		
		62.7	12	752.40
+12		61.5		
		57.6	7	403.20
+19		53.7		
		52.8	4	211.20

Sta Area Mean dist Cst Pt.

3+23 51.9'

51.0' 4 204.00'

+27 50.1'

50.1' 4 200.40'

+31 50.1'

51.6' 9 464.40'

+40 53.1'

52.5' 6 315.00'

+46 51.9'

51.3' 4 205.20'

+50 50.7'

52.2' 10 522.00'

+60 53.7'

55.5' 2 111.00'

+62 57.3'

54.9' 6 329.40'

+68 52.5'

53.4' 6 320.40'

+74 54.3'

53.1' 4 212.40'

Sta Area Mean dist Cst Pt.

3+78 51.9'

52.2' 5 261.00'

+83 52.5'

51.6' 5 258.00'

+88 50.7'

51.6' 11 567.60'

+99 52.5'

53.1' 5 265.50'

+104 53.7'

50.7' 4 202.80'

+108 47.7'

46.8' 5 234.00'

+113 45.9'

50.4' 7 352.80'

+120 54.9'

56.4' 12 676.80'

+132 57.9'

59.7' 11 656.70'

+143 61.5'

62.1' 3 186.30'

Sta Area Mean dist Cu.Pt.

4+46 62.7'

61.8' 4' 247.20'

+50 60.9'

59.7' 6' 358.20'

+56 58.5'

58.8' 4' 235.20'

+60 59.1'

58.2' 6' 349.20'

+66 57.3'

57.6' 4' 230.40'

+70 57.9'

55.5' 5' 277.50'

+75 53.1'

51.6' 5' 258.00'

+80 50.1'

52.8' 5' 264.00'

+85 55.5'

54.0' 10' 540.00'

+95 52.5'

51.6' 9' 464.40'

Sta Area Mean dist Cu.Pt. cu.yd.

5+04 50.7'

5+04 297.8'

292.2' 6' 1753.20'

+10 286.6'

~~32,138.40 1190.31~~

Superseded by calculated sheets ~~32,144.40 - 1190.53~~

Concrete class 1 (Schedule Item 17) below line of floor  
subgrade produced across area.  
This same volume is excavation  
class 2 (Sched. Item 11)

Sta.	Area	Mean	dist	Cuft
0+00	79.1			
	82.35	8	658.80	
+08	85.6			
	92.65	12	1111.80	
+20	99.7			
	94.55	6	567.30	
+26	89.4			
	100.85	5	504.25	
+31	112.3			
	103.55	11	1139.05	
+42	94.8			
	95.85	4	383.40	
+46	96.9			
	83.75	2	167.50	

Sta.	Area	Mean	dist	Cuft
0+48	70.6			
	69.0	16	1104.00	
+64	67.4			
	71.25	10	712.50	
+74	75.1			
	76.5	8	612.00	
+82	77.9			
	80.5	14	1127.00	
+96	83.1			
	71.8	24	1723.20	
+120	60.5			
	57.9	9	521.10	
+29	55.3			
	56.1	11	617.10	
+40	56.9			
	59.95	15	899.25	
+55	63.0			
	62.35			
	60.75	13	810.55	
	789.75			
+68	58.5			
	61.7	68.80	1651.20	
	67.2	24	1612.80	

Sta Area Mean dist Cn. ft.  
1+92 75.9 ✓

688' 24 1651.20 ✓

2+16 61.7 ✓

62.95' 24 1510.80 ✓

+40 64.2 ✓

55.3' 24 1327.20 ✓

+64 46.4 ✓

57.1' 6 342.60 ✓

+70 67.8 ✓

100.7' 6 604.20 ✓

+76 133.6 ✓

136.4' 7 954.80 ✓

+83 139.2 ✓

155.55' 5 777.75 ✓

+88 171.9 ✓

166.65' 5 833.25 ✓

+93 161.4 ✓

161.0' 7 1127.00 ✓

3+00 160.6 ✓

142.25' 12 1707.00 ✓

Sta Area Mean dist Cn. ft.  
3+12 123.9 ✓

134.8' 7 943.60 ✓

+19 145.7 ✓

141.65' 4 566.60 ✓

+23 137.6 ✓

136.0' 4 544.00 ✓

+27 134.4 ✓

132.15' 4 528.60 ✓

+31 129.9 ✓

128.7' 9 1158.30 ✓

+40 127.5 ✓

124.3' 6 745.80 ✓

+46 121.1 ✓

119.85' 4 479.40 ✓

+50 118.6 ✓

108.15' 10 1081.50  
108.65' 10 1086.50

+60 97.7 ✓

96.05' 2 192.10 ✓

+62 94.4 ✓

94.0' 6 564.00 ✓

Sta	Area	Mean	dist	Cu.Ft.
3+68	93.6			
		92.8	6	556.80
+74	92.0			
		95.45	4	381.80
+78	98.9			
		90.0	5	450.00
+83	81.1			
		79.9	5	399.50
+88	78.7			
		87.35	11	960.85
+99	96.0			
		95.4	5	477.00
4+04	94.8			
		92.2	4	368.80
+08	89.6			
		90.6	5	453.00
+13	91.6			
		99.7	7	697.90
+20	107.8			
		113.45	12	1361.40

Sta	Area	Mean	dist	Cu.Ft.
4+32	119.1			
		125.15	11	1376.65
+43	131.2			
		136.25	3	408.75
+46	141.3			
		131.2	4	524.80
+50	121.1			
		122.1	6	732.60
+56	123.1			
		123.1	4	492.40
+60	123.1			
		129.35	6	776.10
+66	135.6			
		142.45	4	569.80
+70	149.3			
		147.9	5	739.50
+75	146.5			
		143.9	5	719.50
+80	141.3			
		136.85	5	684.25

Sta Area Mean dist. Cufft. Ch. Rd  
4+85 132.4

133.0 10 1330.00 ✓

+95 133.6 ✓

155.0 9 1395.00 ✓

5+04 176.4 ✓

5+04 114.2 ✓

115.6 6 693.60 ✓

+100 117.0 ✓

48,445.50 1794.28

48,499.70 - 1796.28

20.42  
EI 739.46

A = 171.13 ✓  
74.6

EI 732  
25.46

0+00

20.42  
EI 739.46

A = 353.32 ✓

EI 725.42  
29.91 2+40

20.42  
EI 739.46

A = 564.68 ✓

20.62

EI 718.84

34.35

4+80

### CURVED SECTION

$$510 \times 152.74 = 77897.40$$

### Prism section 8+00 to

4+80

$$\frac{480}{6} \times [171.13 + 4(353.32) + 564.68]$$

$$= 171,927.20 ✓$$

4+80 to 4+95 ✓

$$\frac{15}{6} \times [564.68 + 4(572.27) + 579.89]$$

$$= 8,584.14$$

4+95 to 5+10

$$\frac{15}{6} \times 579.89 + 4(593.08) + 606.38$$

$$= 8,896.50$$

267,305.15  
20

(over)

20.42  
EI 739.46

A = 579.89 ✓

EI 718.40

34.65

4+95

20.42  
EI 739.46

A = 572.27 ✓

34.50

4+87.50

Less piece of floor 267, 305.15 Cu.Ft.

and triangular piece left  
out

$$1.5224 \times 510 = 776.42$$

$$266,528.73$$

$$= 9,871.43 \text{ Cu.Ft.}$$

From Page 40

$$1,794.28 \text{ " "$$

$$11,665.72 \text{ " "}$$

$$19.05$$

$$11,684.76 \text{ " "}$$

204.2  
EI 739.46

$$A = 606.38$$

EI 717.64

35.16

5+10

20.42

$$A = 593.08$$

34.905

5+02

Mass concrete class 1 (Schedule

Item 17) back of 0+00 in

Ogee Areas measured by  
planimeter from cross-sections  
(Ref. book A51 P.47)

0+00 377.5

260.8 1.5 391.20

0-1 144.1

95.25 1 95.25

0-2 46.4

23.2 1.2 27.84

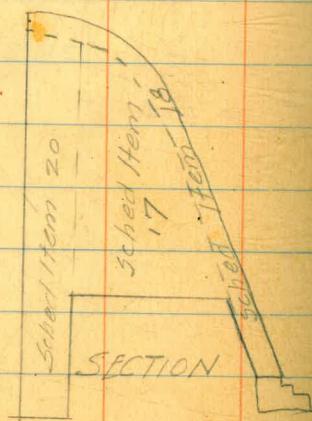
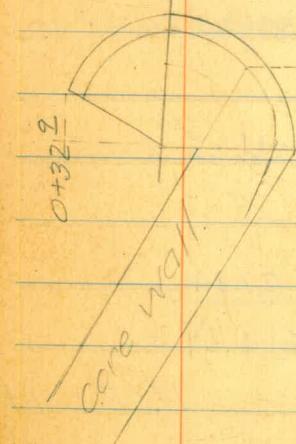
0-3 0.0

514.29 =

19.05 Cu.Ft.

Spillway  
STA 5+36

Bottom of warped wall



SPILLWAY SIDELINING IN  
WARPED SECTION STA 5+10 TO 5+36

Schedule Item 18

sta	area	mean dist.	cu. ft.
0+00	97.58		
	97.58	31	292.74 ✓
0+03	97.58		
0+03	104.85		
	108.47	31	325.41 ✓
0+06	112.09		
	114.51 ✓	21	229.02 ✓
0+08	116.93		
	118.34 ✓	2 ✓	236.68 ✓
0+10	119.75		
	122.57 ✓	2.	245.14 ✓
0+12	125.40		
	126.41 ✓	2 ✓	252.82 ✓
0+14	127.42		
	129.23 ✓	2 ✓	258.46 ✓
0+16	131.04		
	132.45 ✓	1 3 ✓	172.18 ✓
0+17 3	133.87		

Areas measured by planimeter

from plotted cross-sections

0+16 131.04

0+17 3 133.87

sta area mean dist cu.ft.  
0+17<sup>3</sup> 133.87

132.45 1<sup>2</sup> 158.94 ✓

0+18<sup>5</sup> 131.04

130.64 0<sup>5</sup> 78.38 ✓

0+19<sup>10</sup> 130.24

126.81 2<sup>9</sup> 367.75 ✓

0+22 123.38

123.58 1<sup>4</sup> 173.01 ✓

0+23<sup>4</sup> 123.79

122.39 1 122.39 ✓

0+24<sup>4</sup> 121.00

64.61 1<sup>1</sup> 71.07 ✓

0+25<sup>5</sup> 8.22

7.61 0<sup>5</sup> 3.80 ✓

0+26 7.00

7.35 1<sup>2</sup> 8.82 ✓

0+27<sup>2</sup> 7.70

8.40 2<sup>3</sup> 19.32 ✓

0+29<sup>5</sup> 9.10

sta area mean dist cu.ft.

0+29<sup>5</sup> 9.10

13.82 2<sup>4</sup> 33.17 ✓

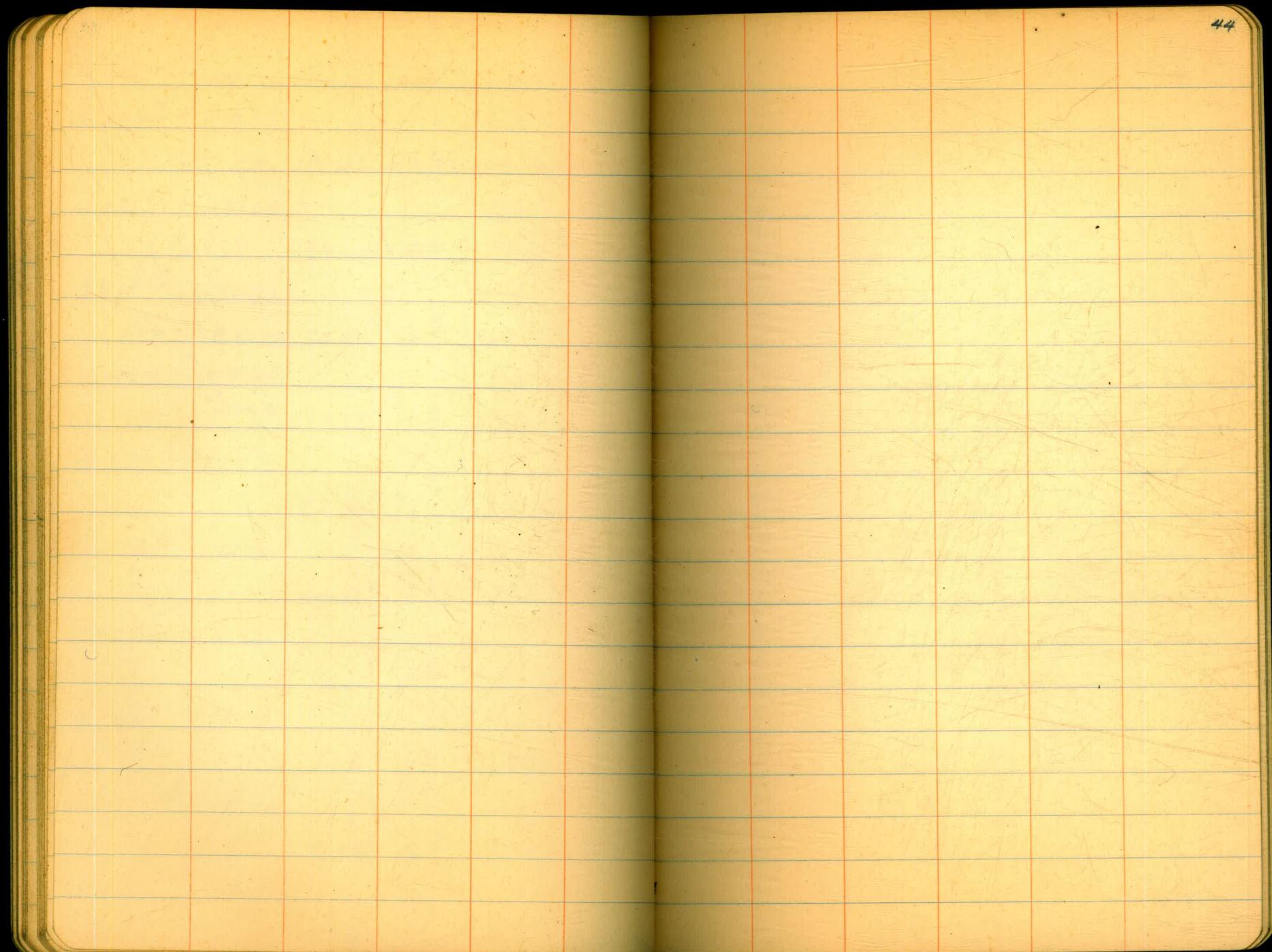
0+31<sup>9</sup> 18.55

9.27 1 9.27 ✓ 1282

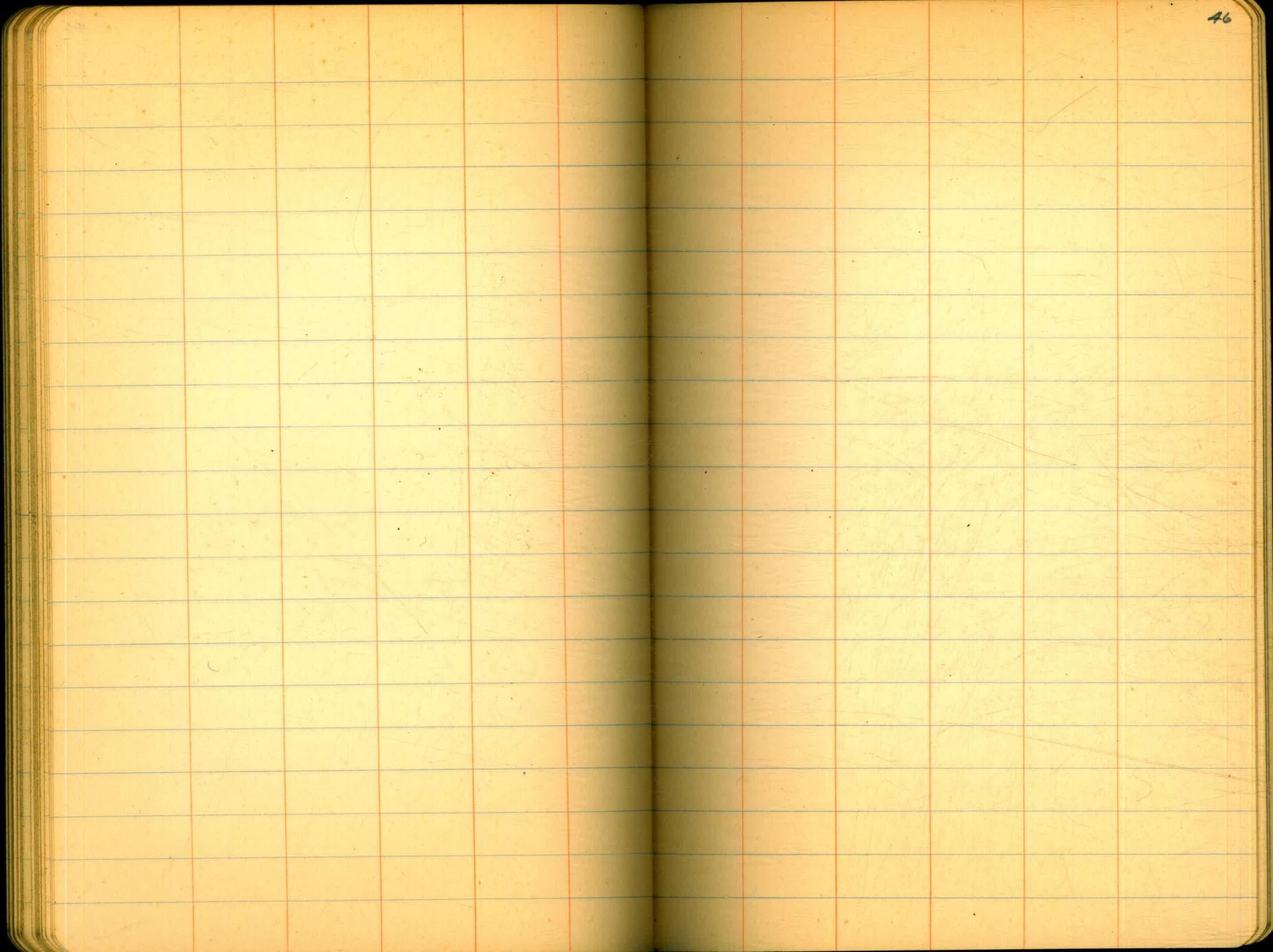
0+32<sup>9</sup> 0.00

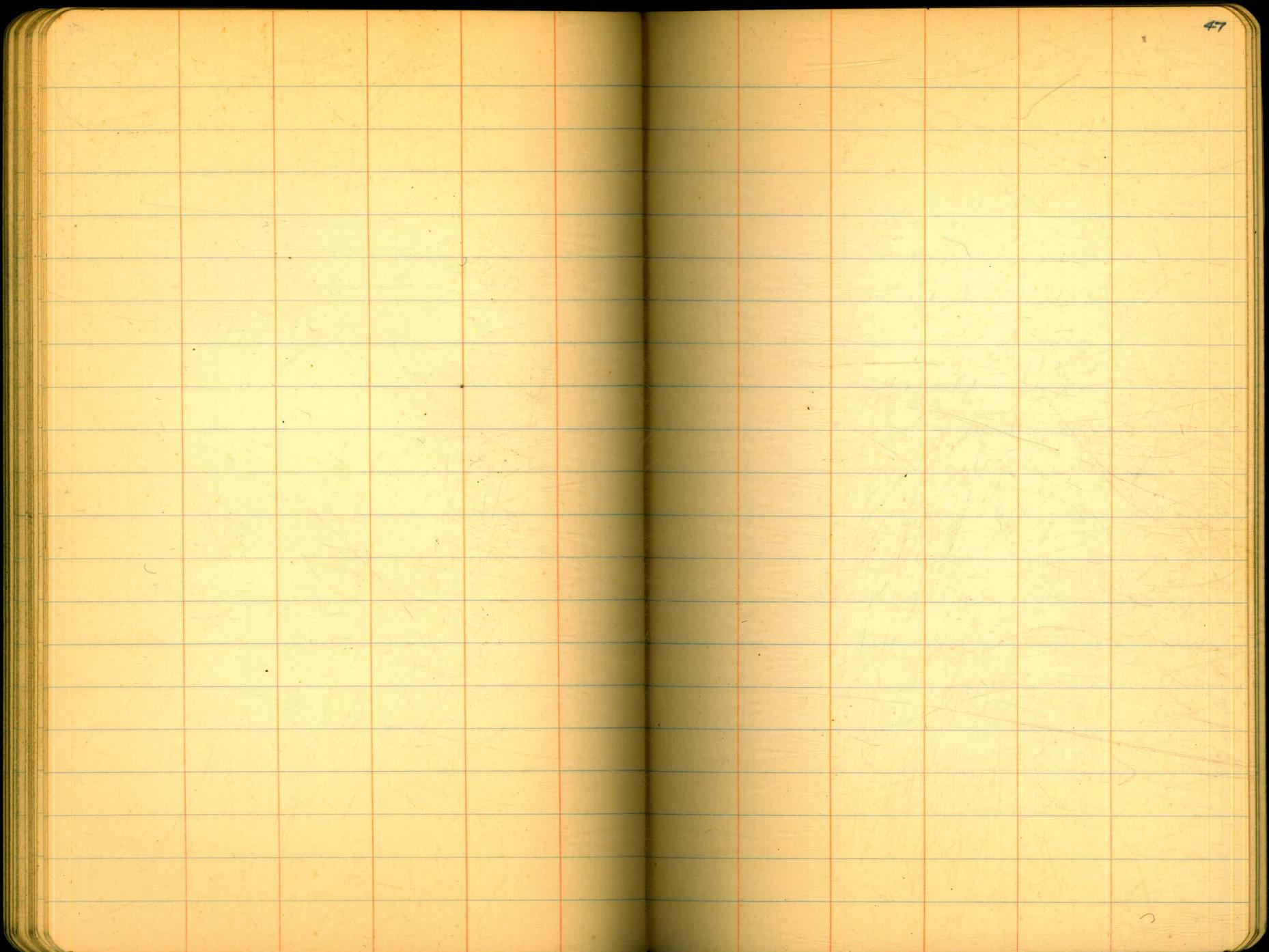
30.58 37 =

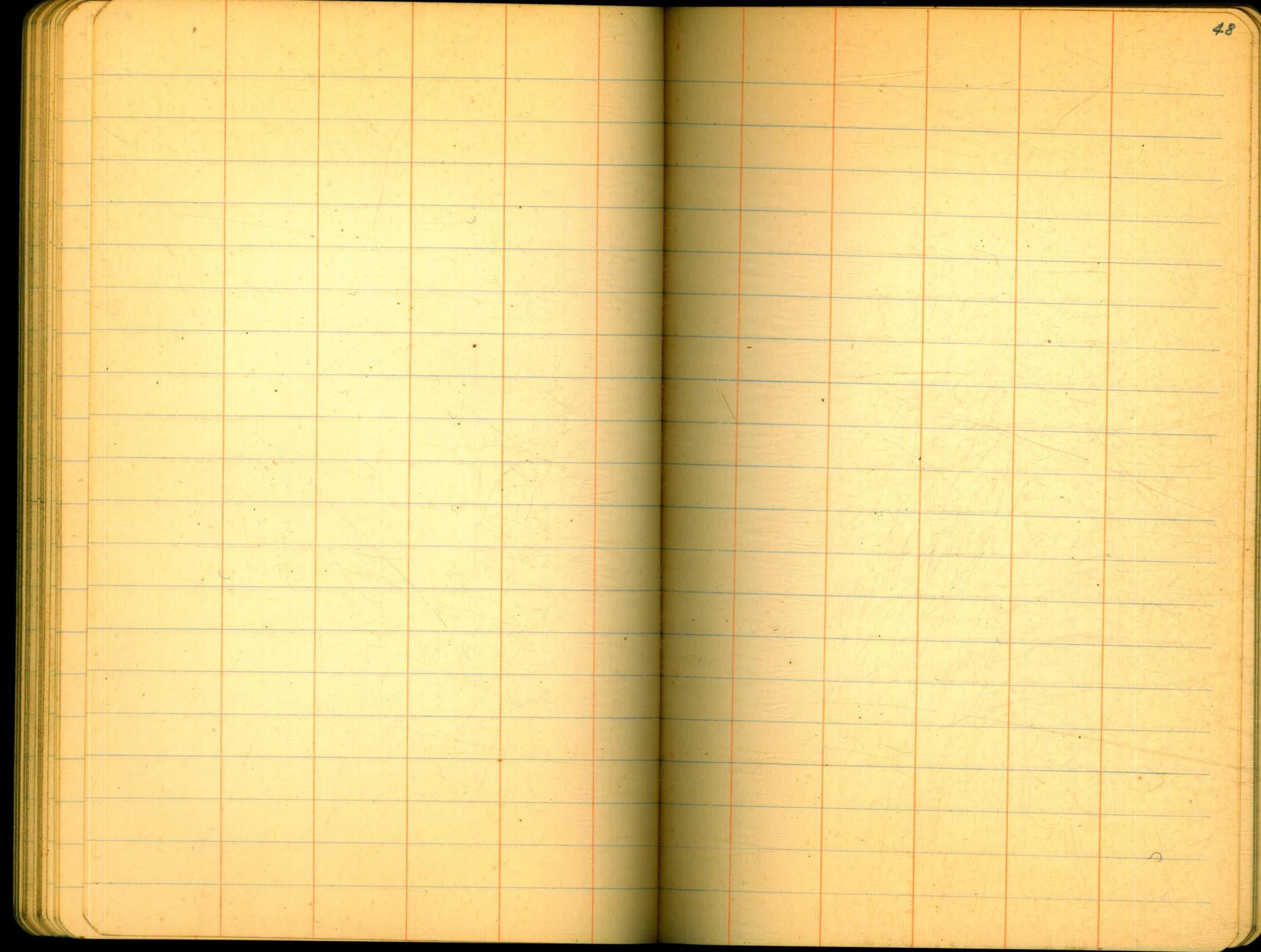
113.27 Cu. Yd.  
G.W.G.

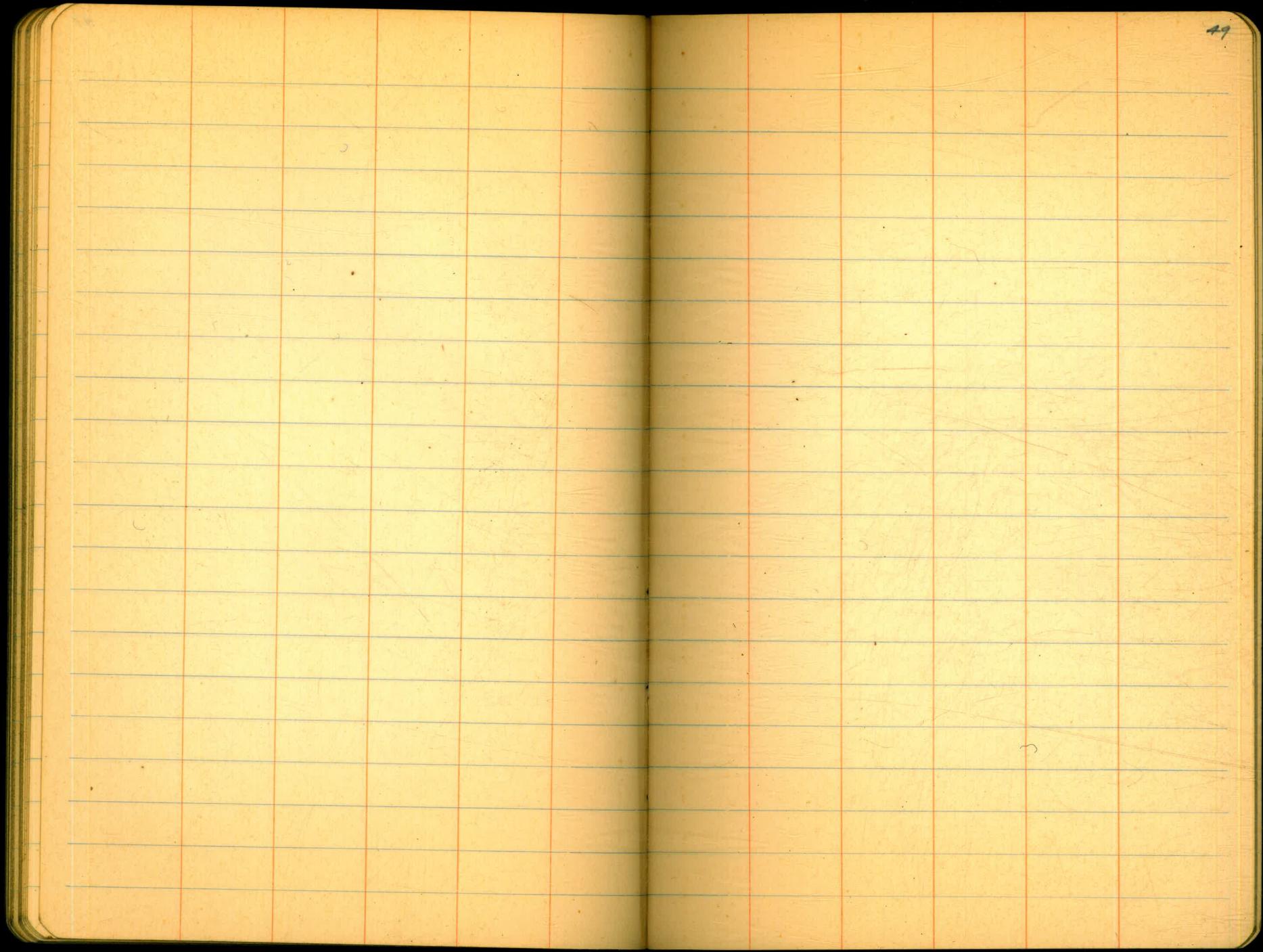


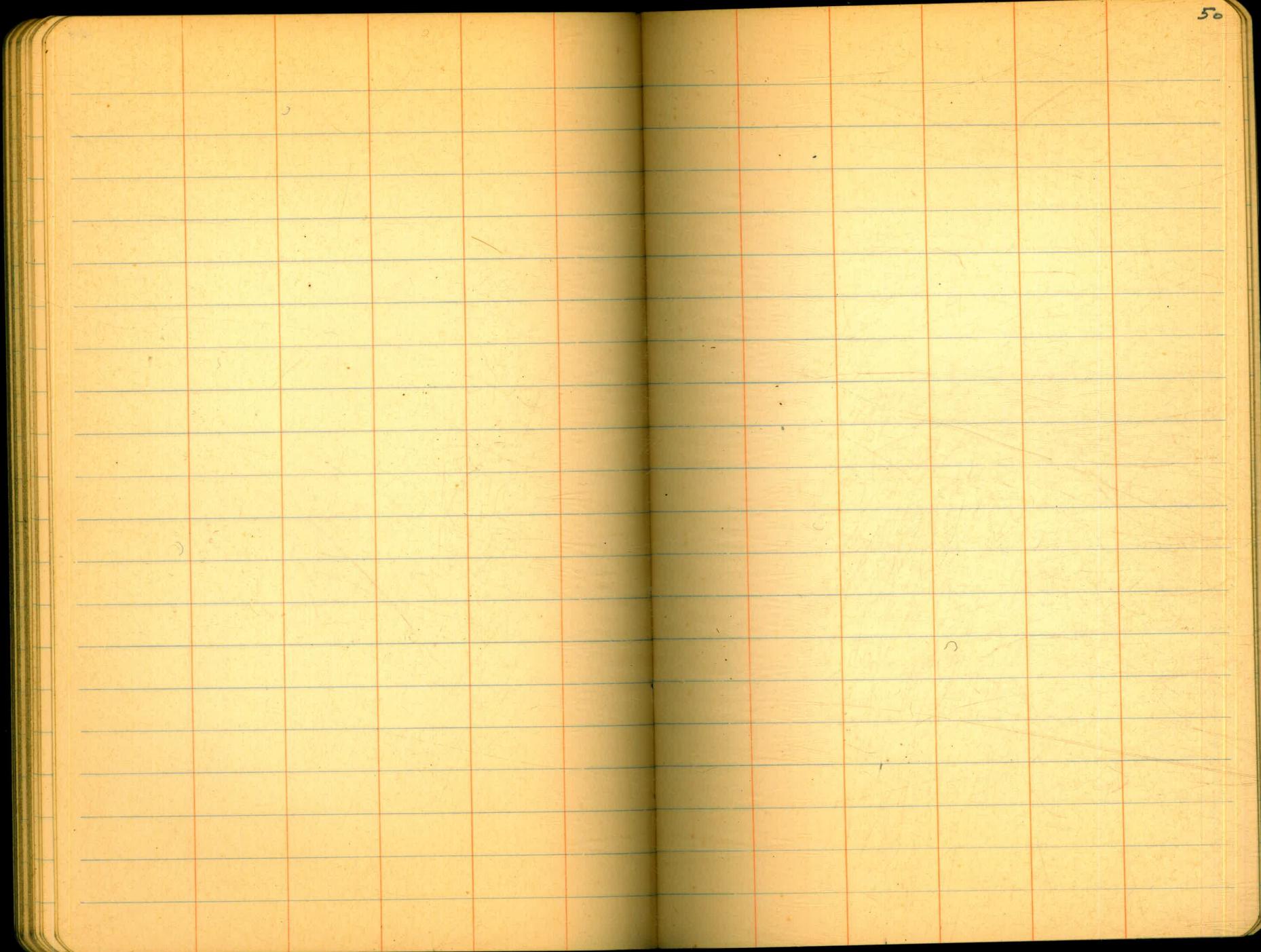


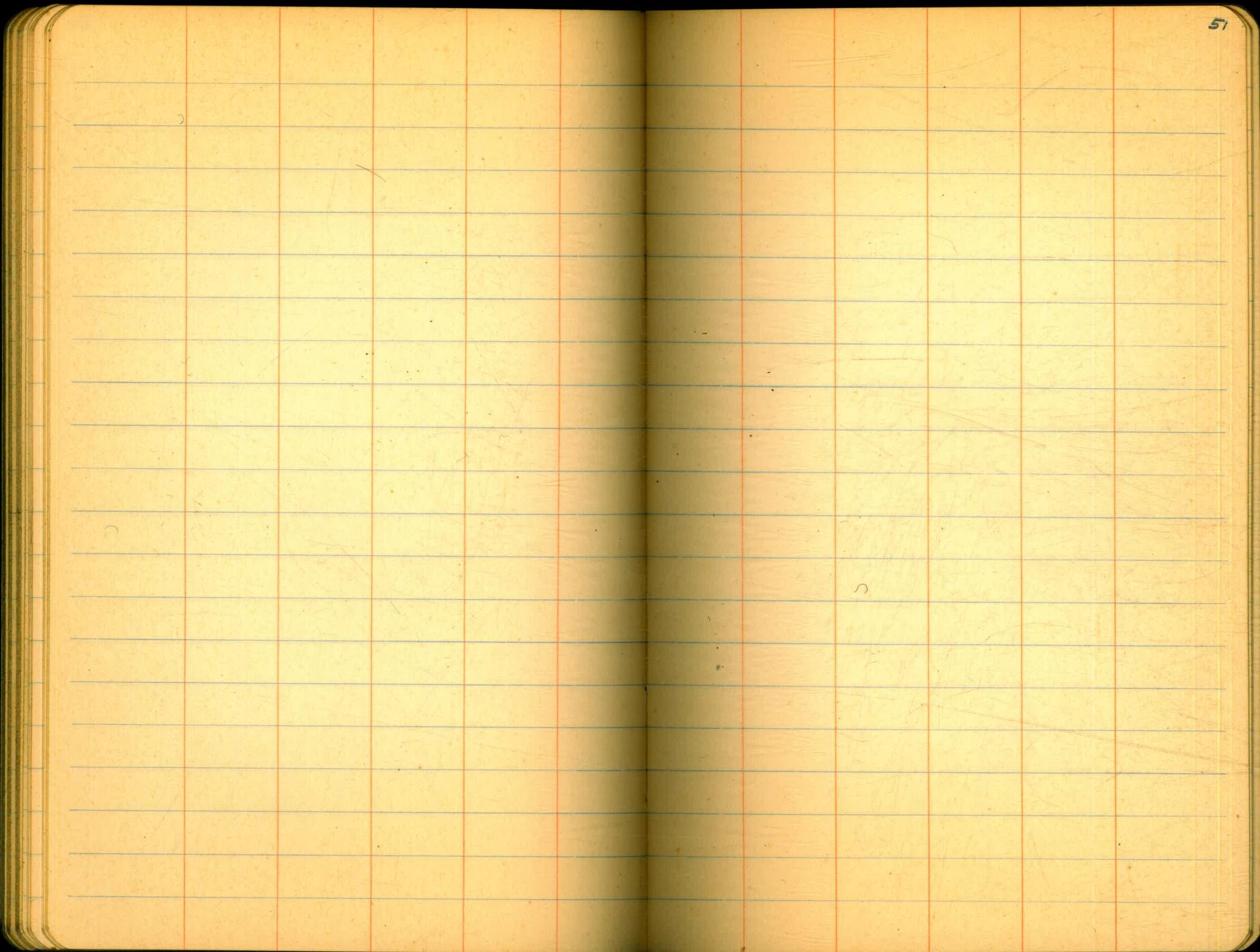


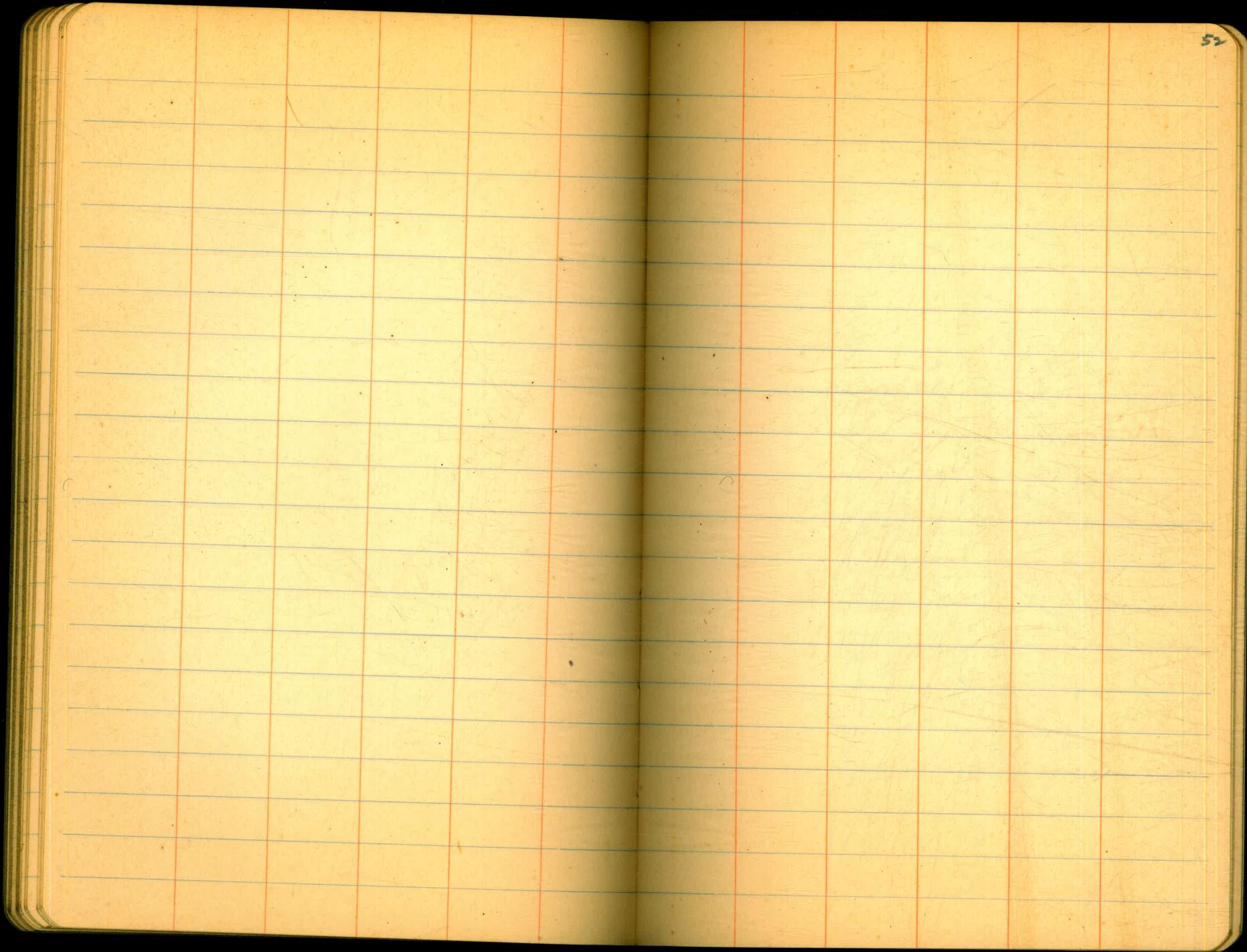


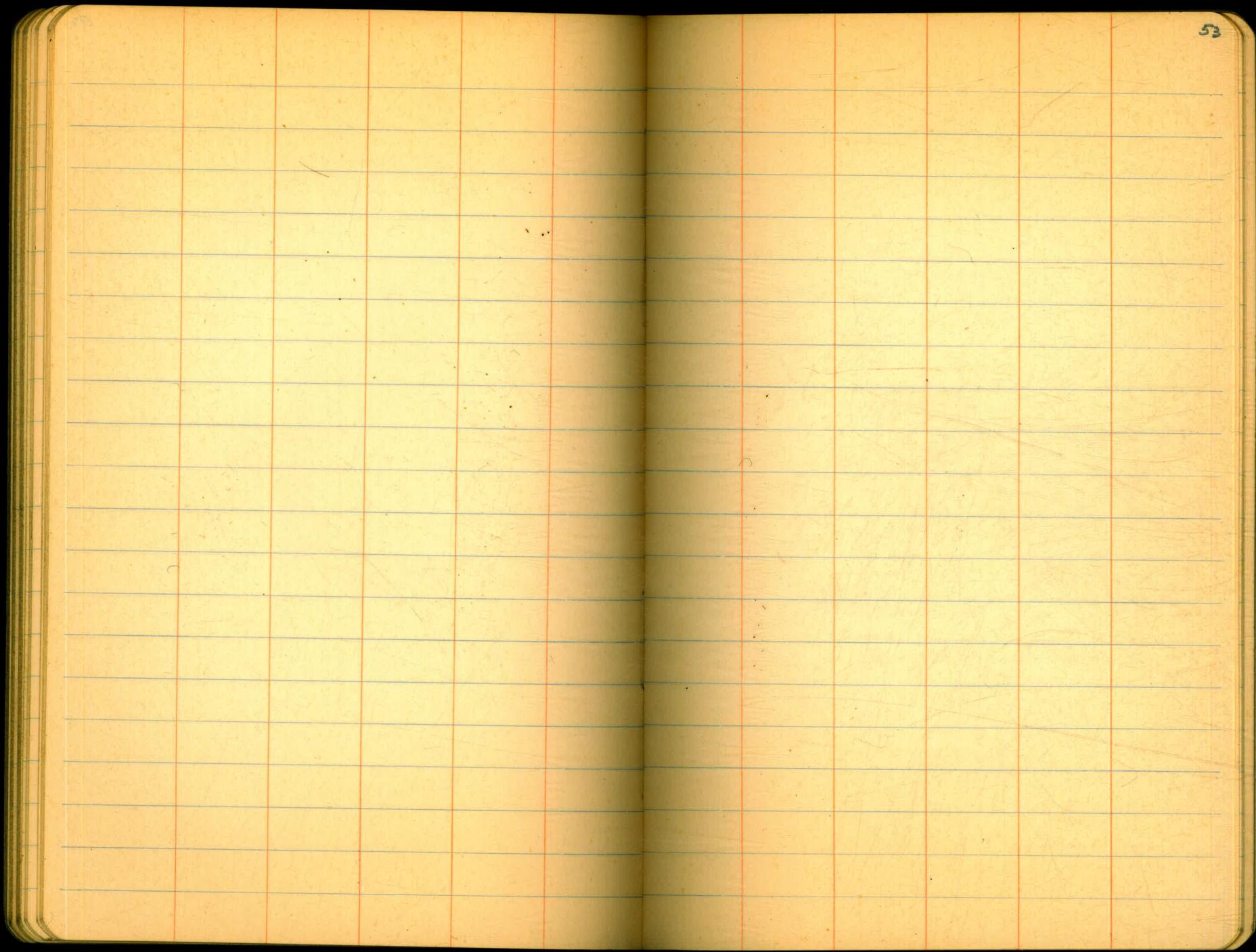


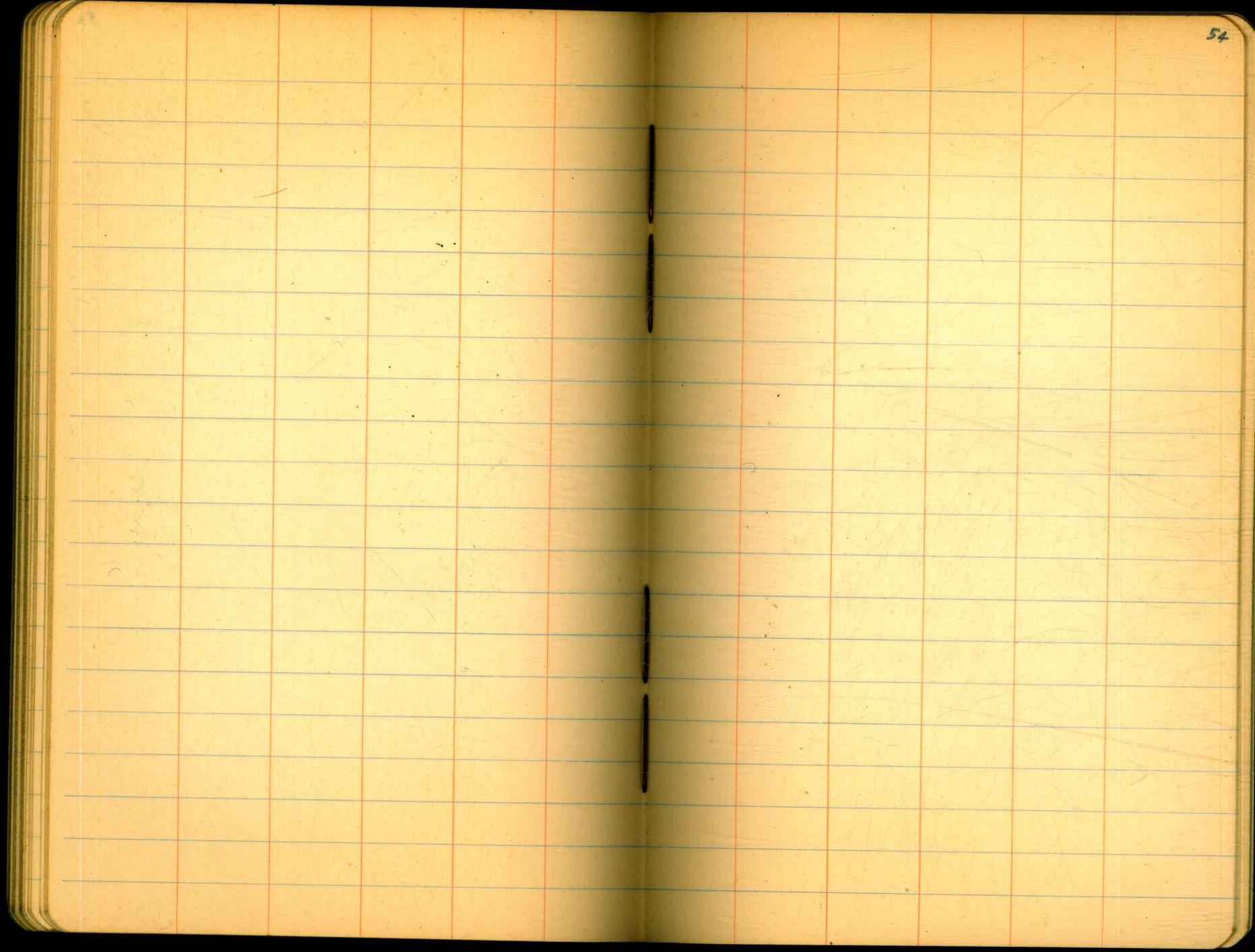


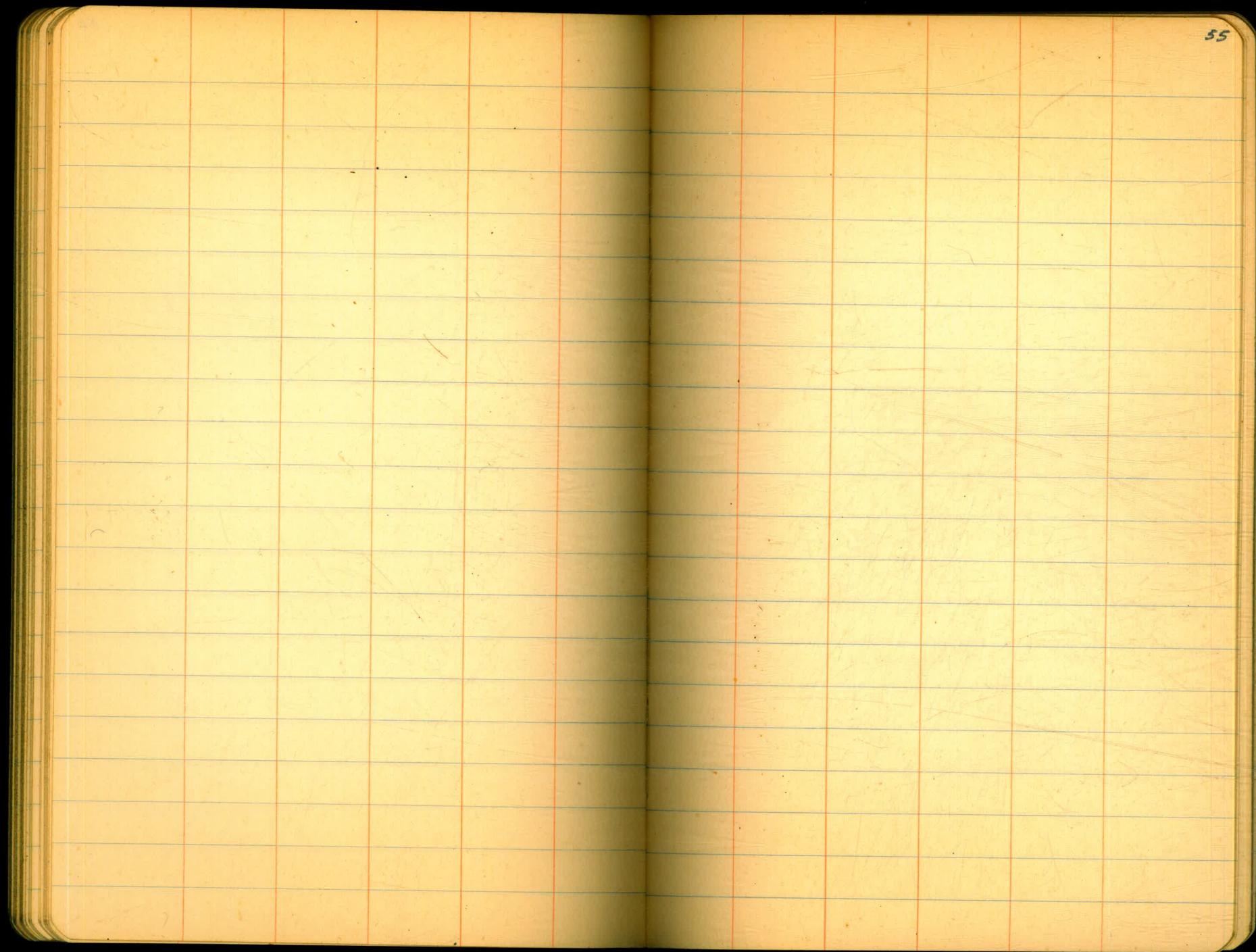


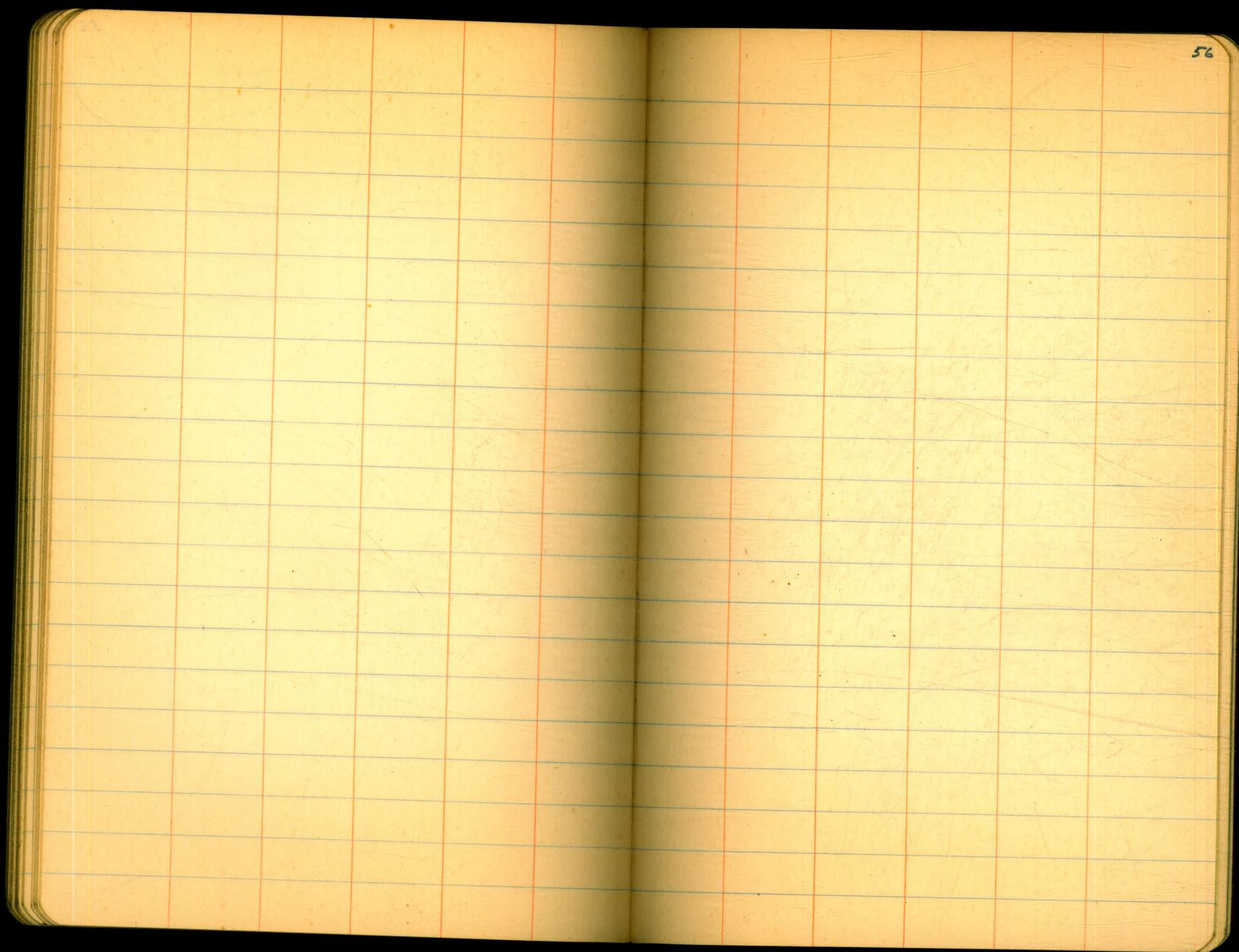


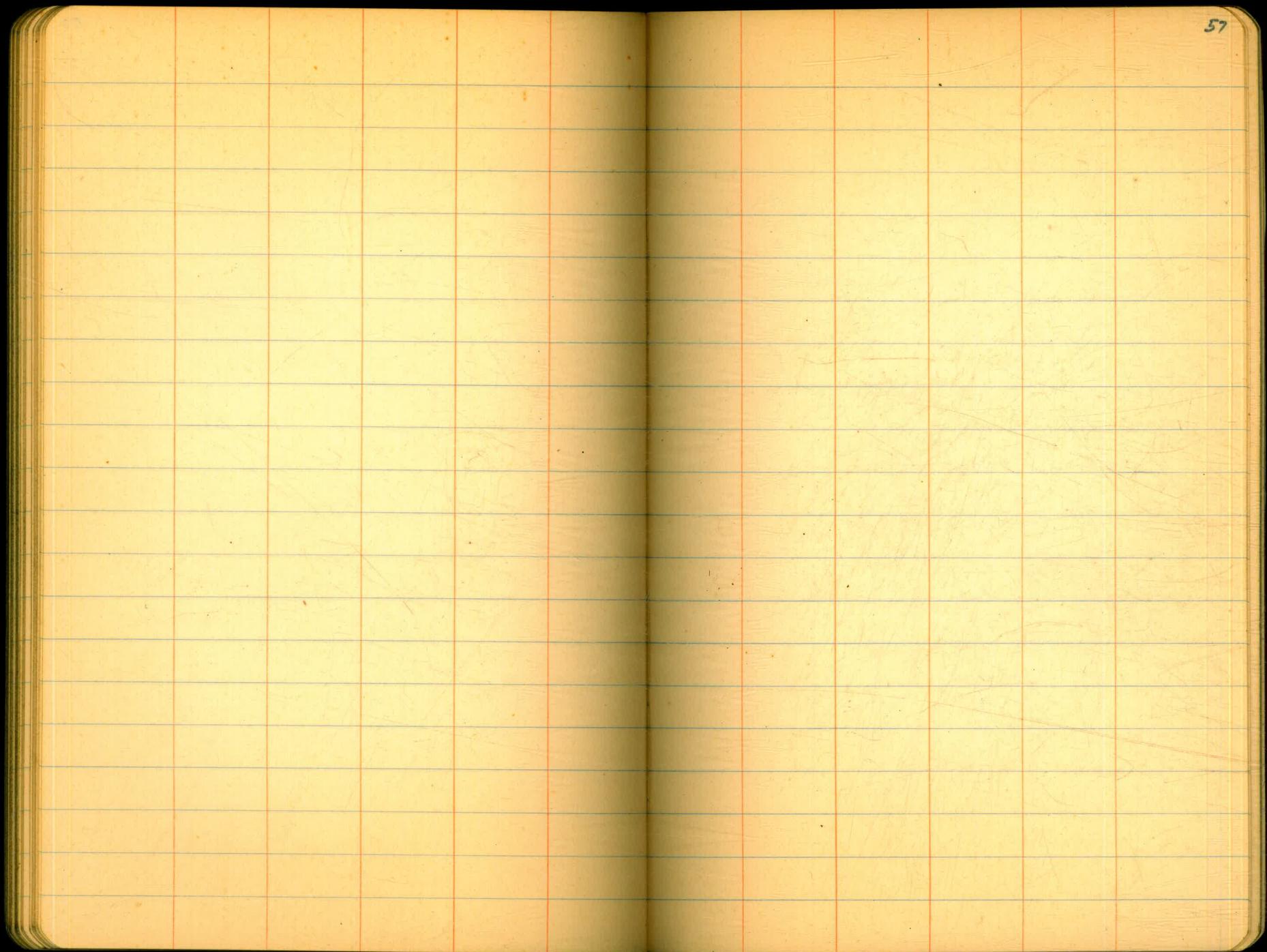


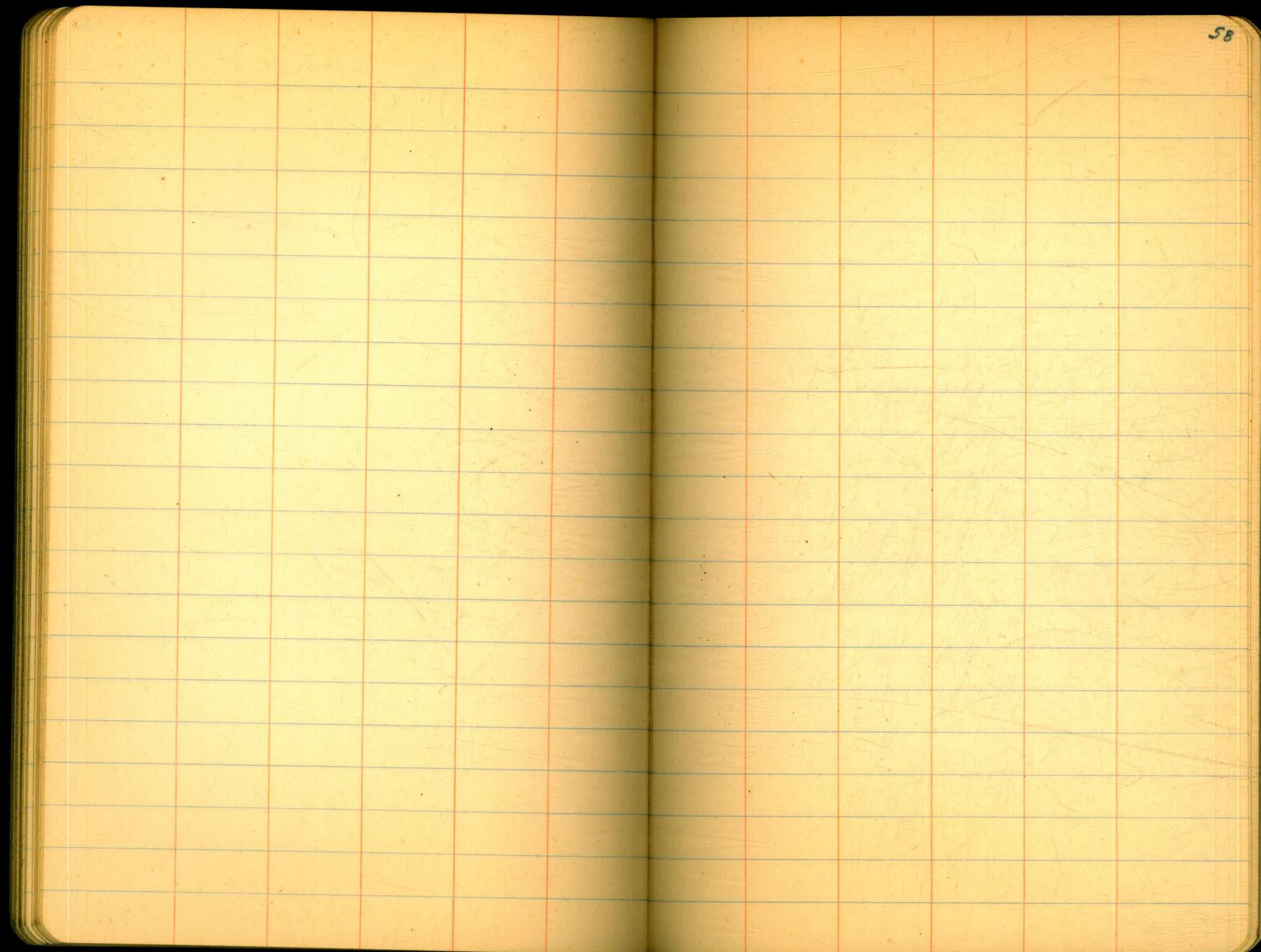


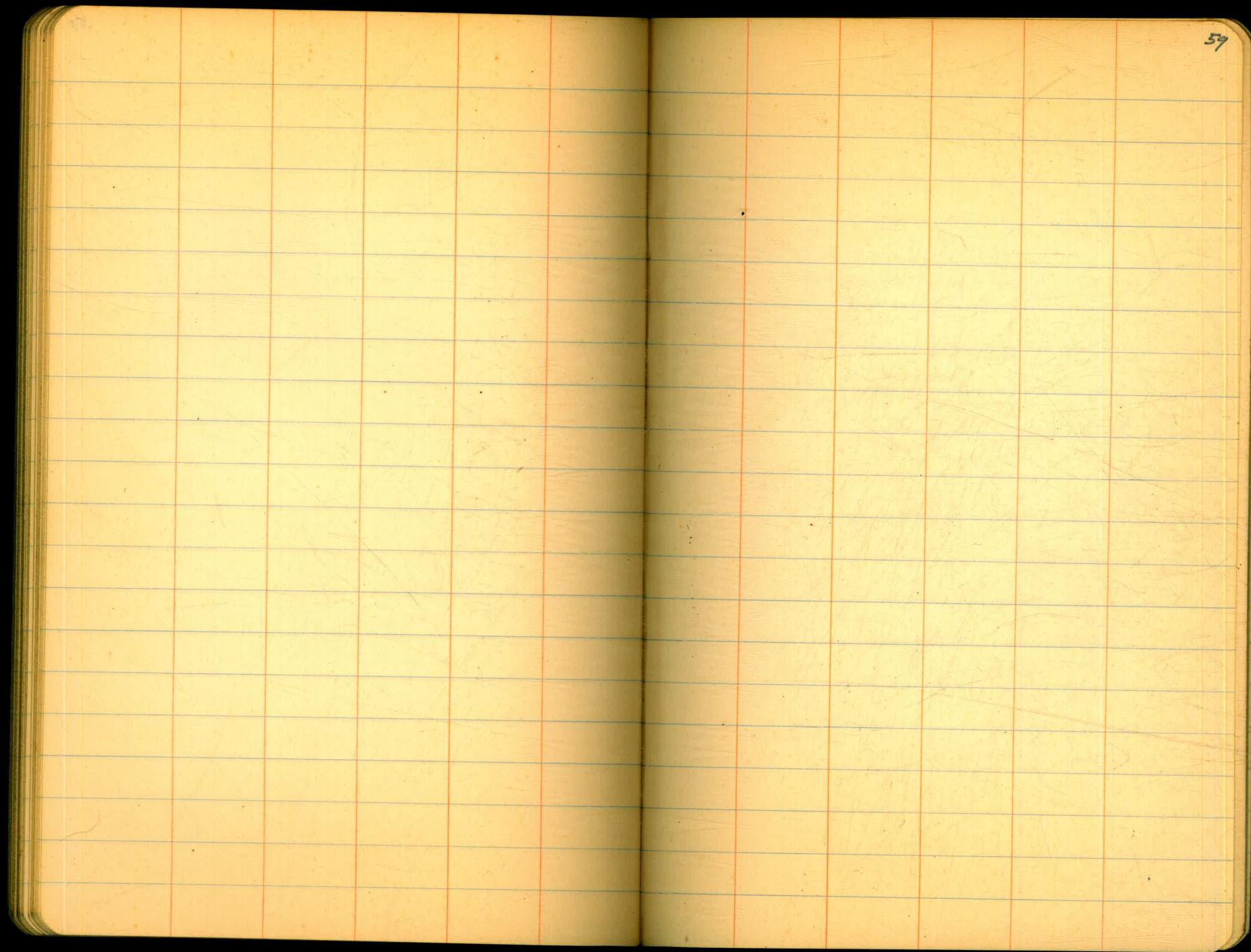


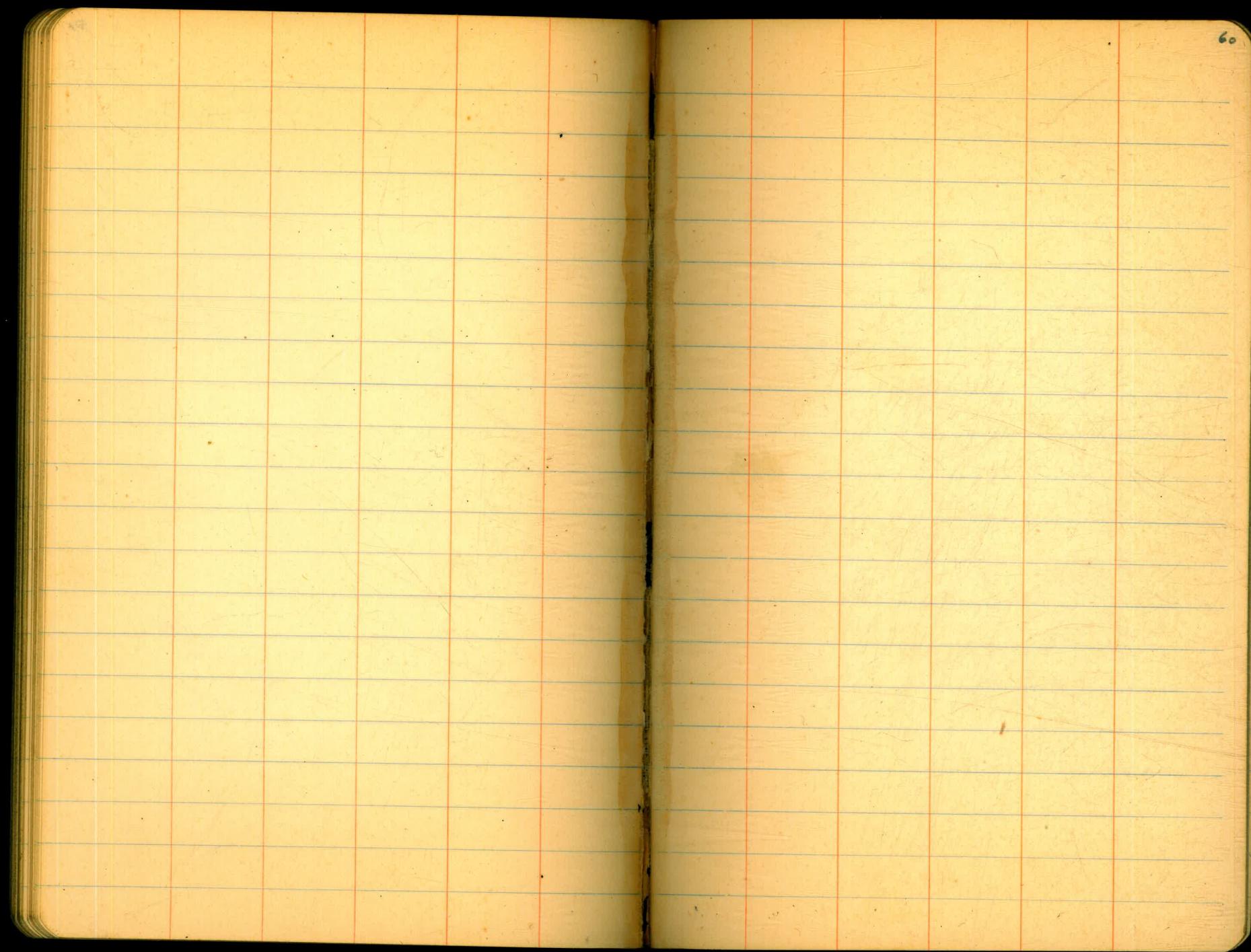


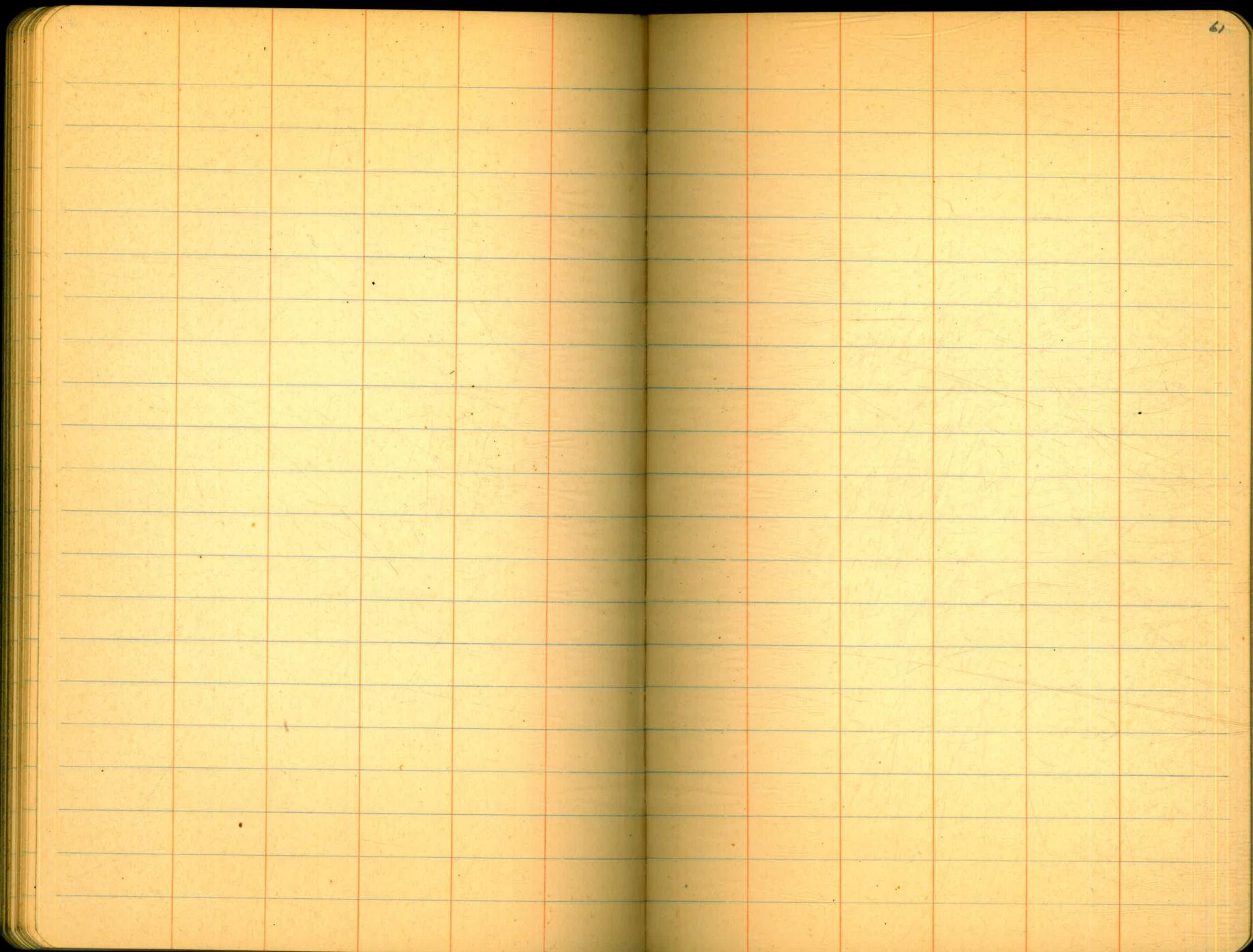


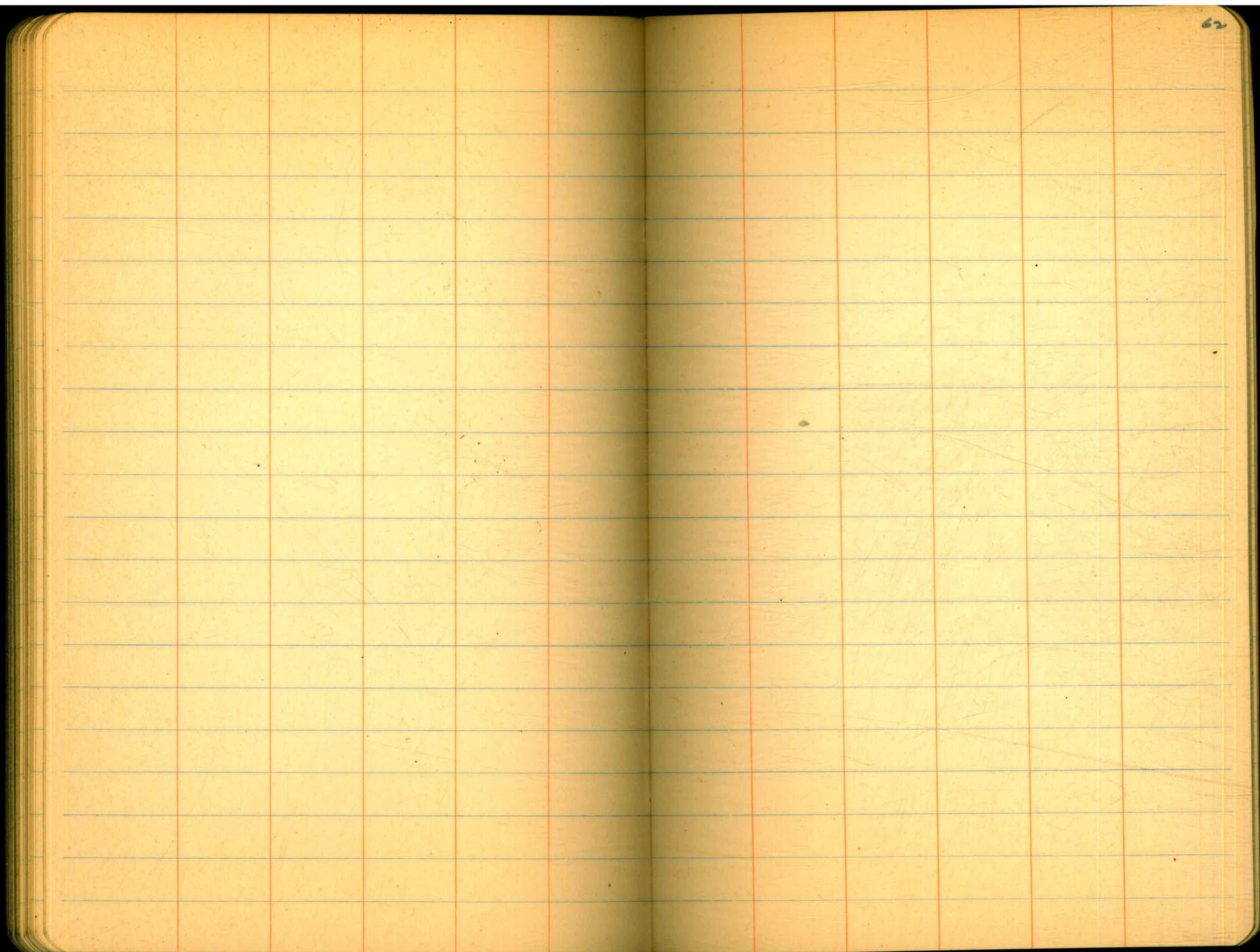


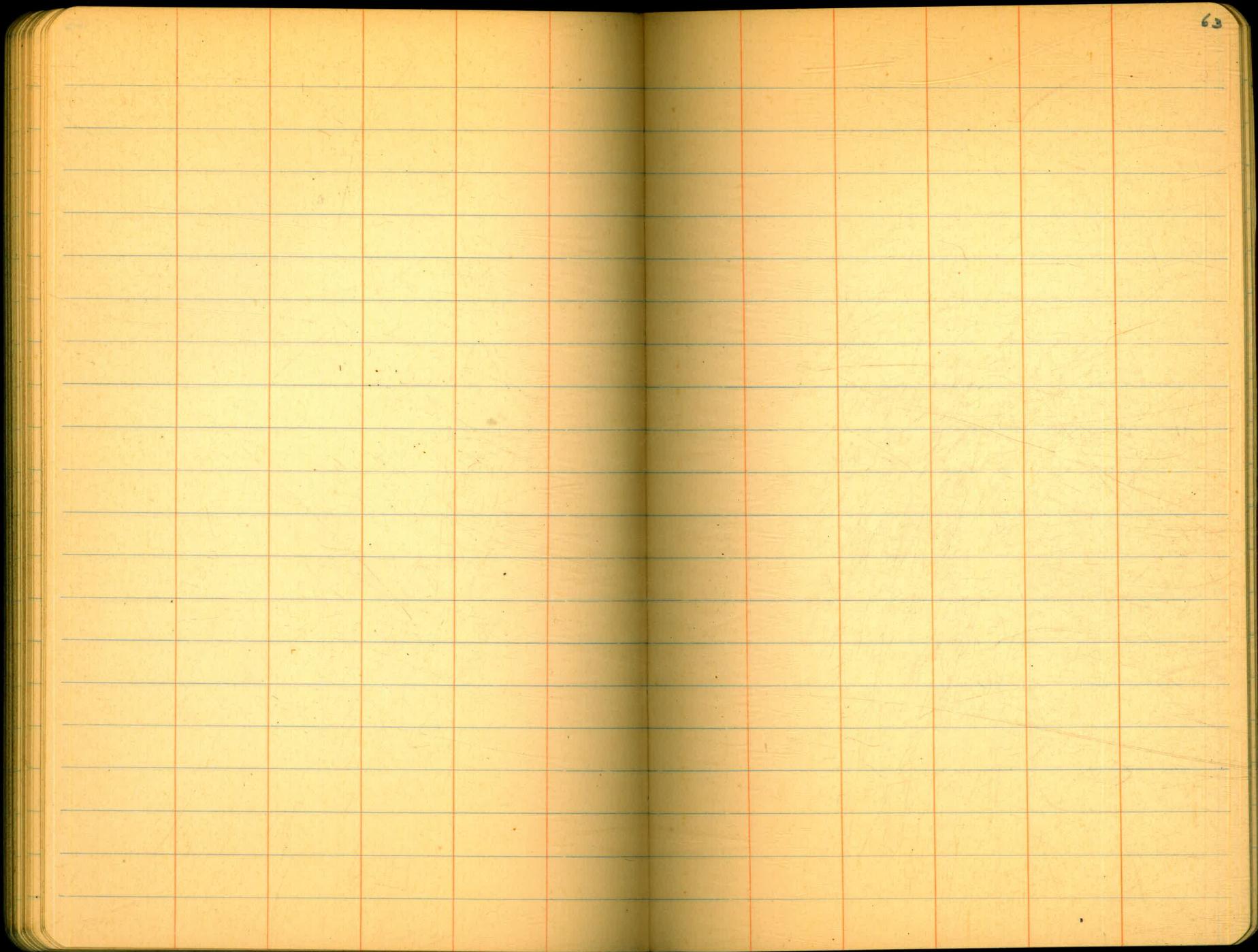


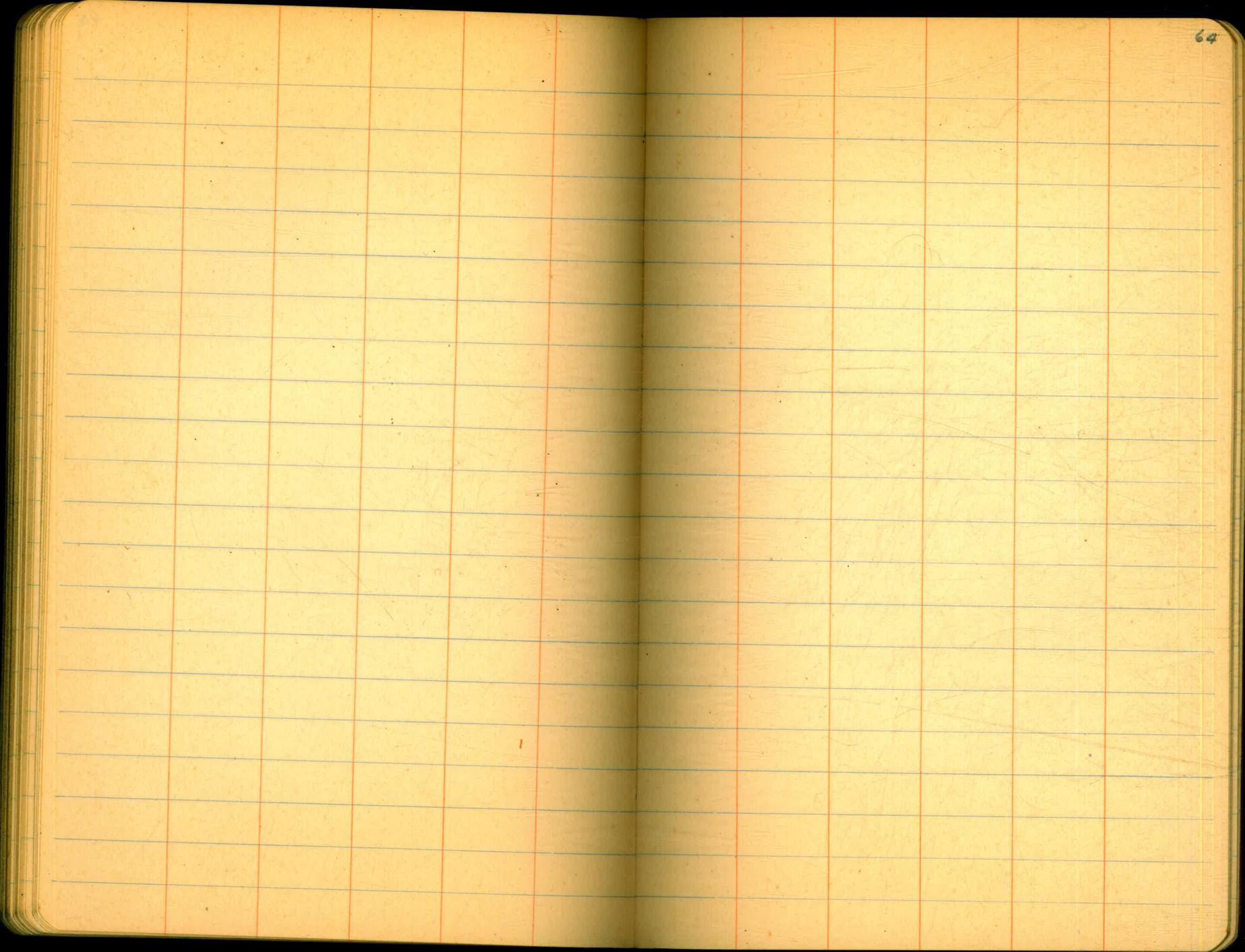


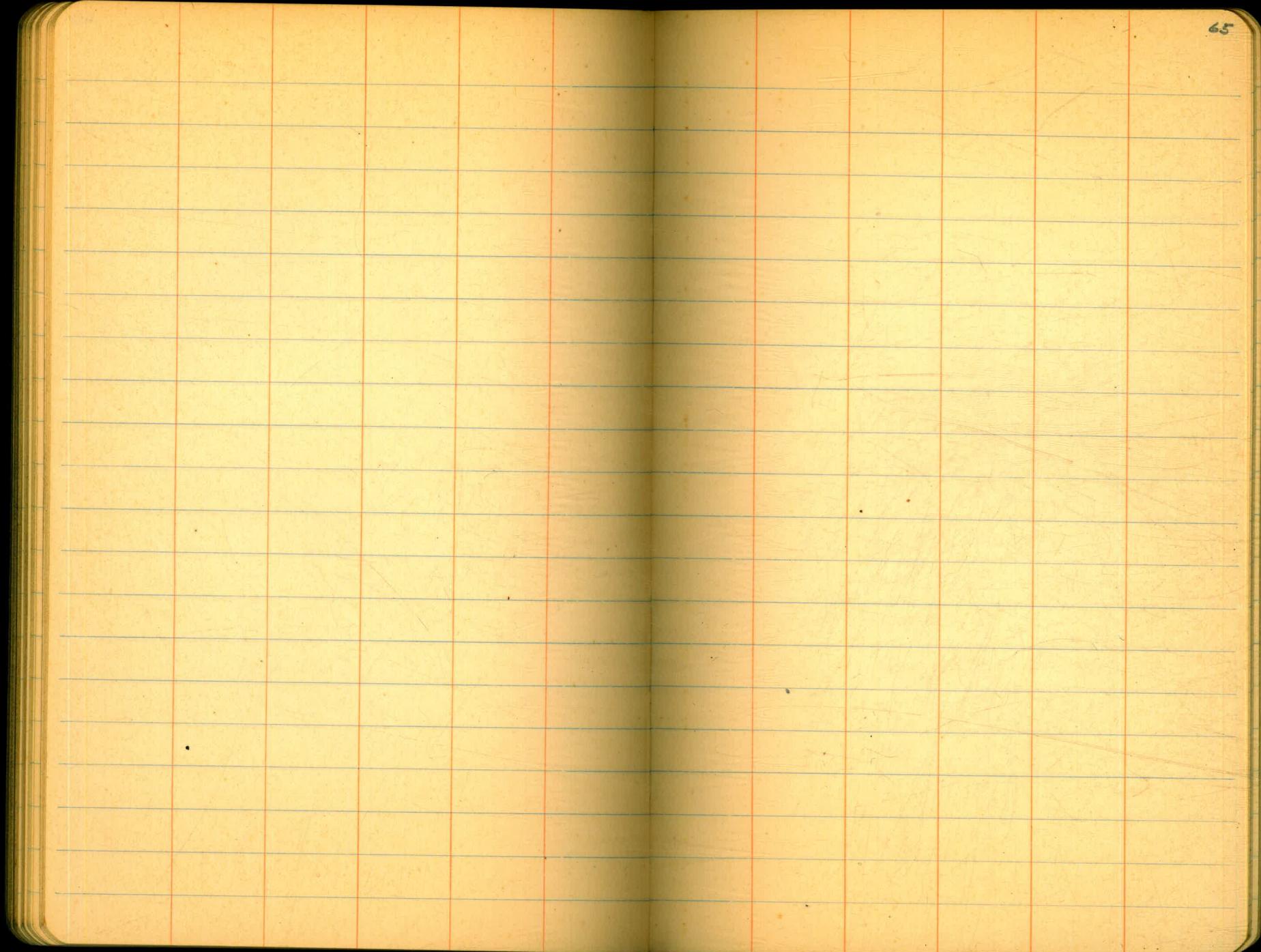


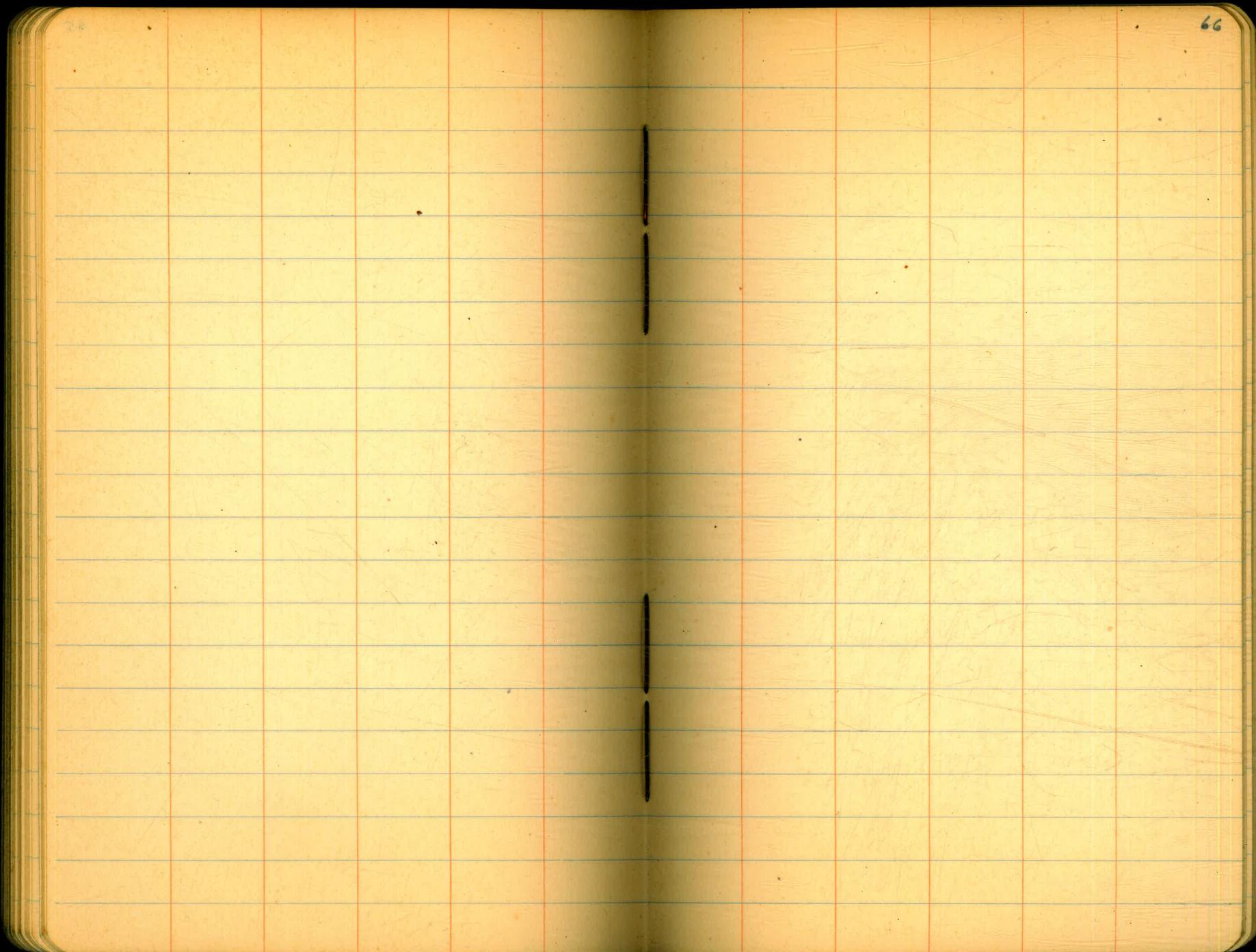


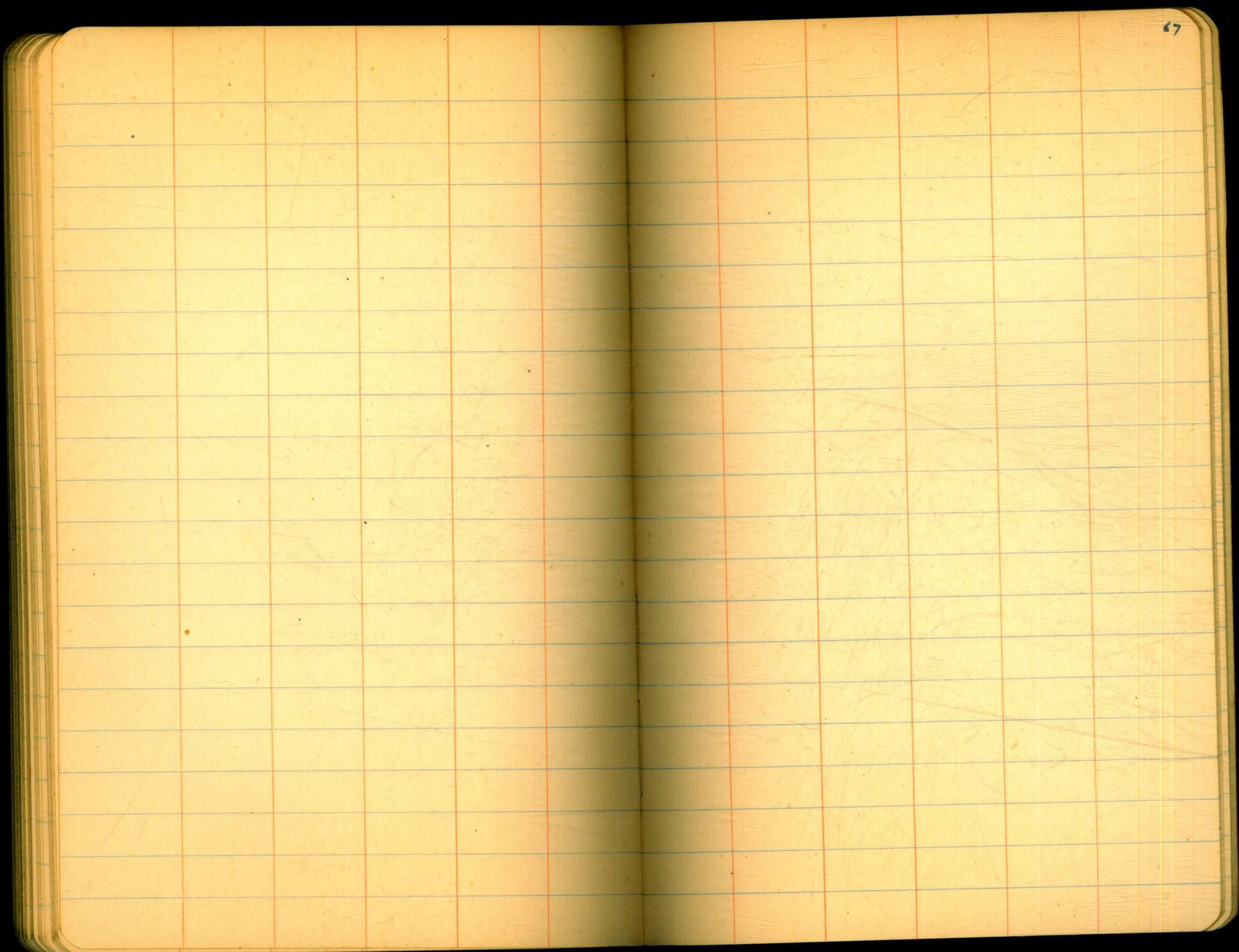


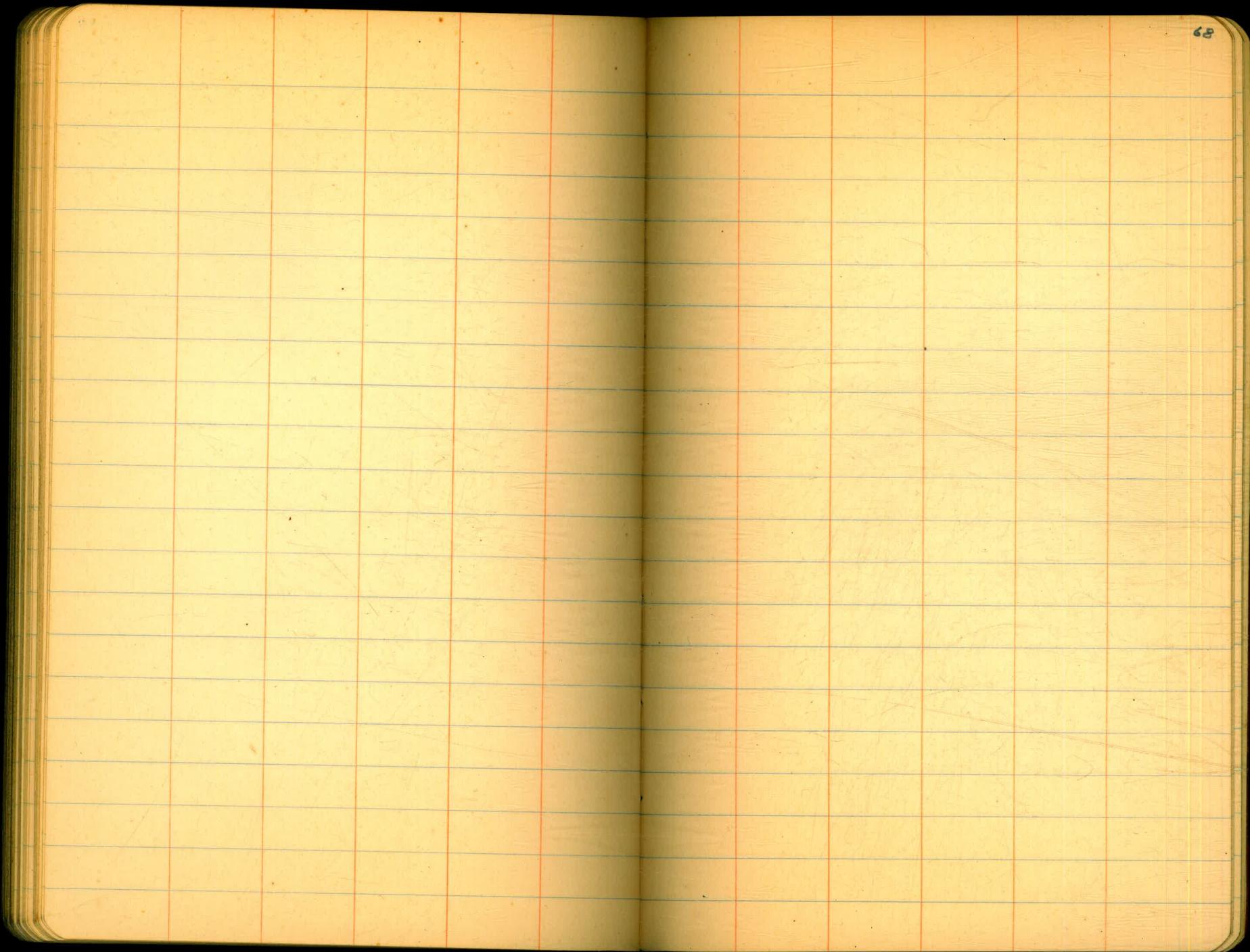


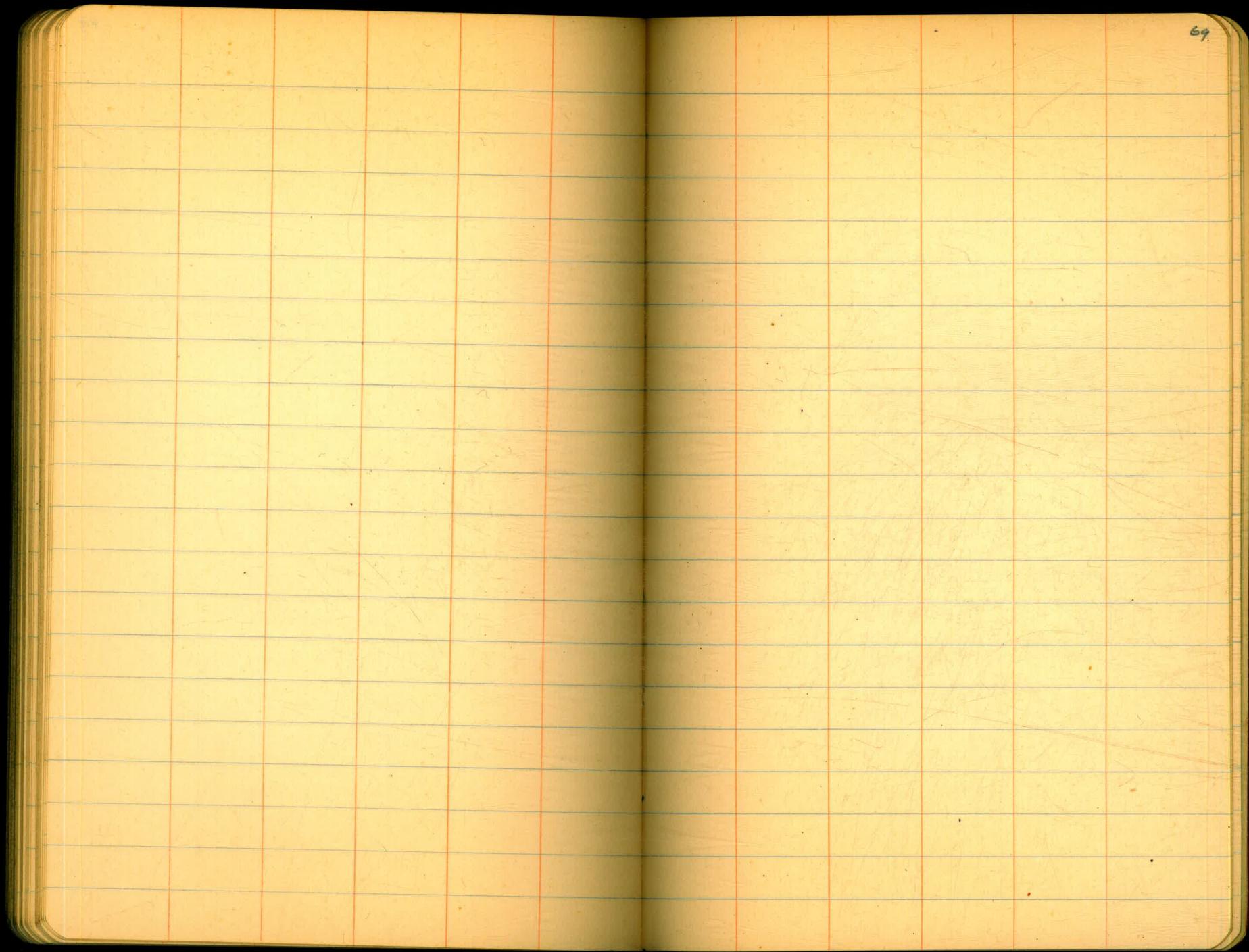


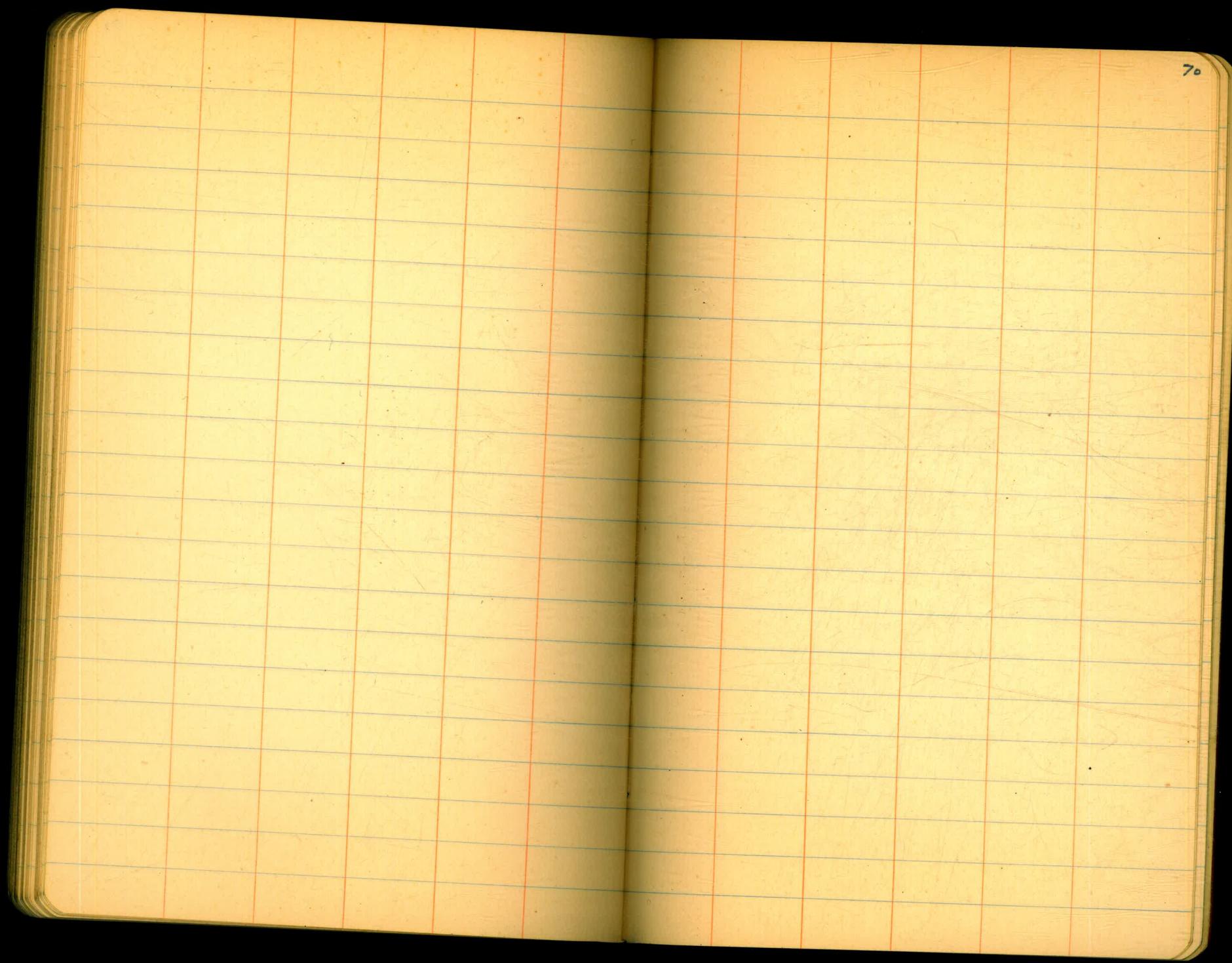


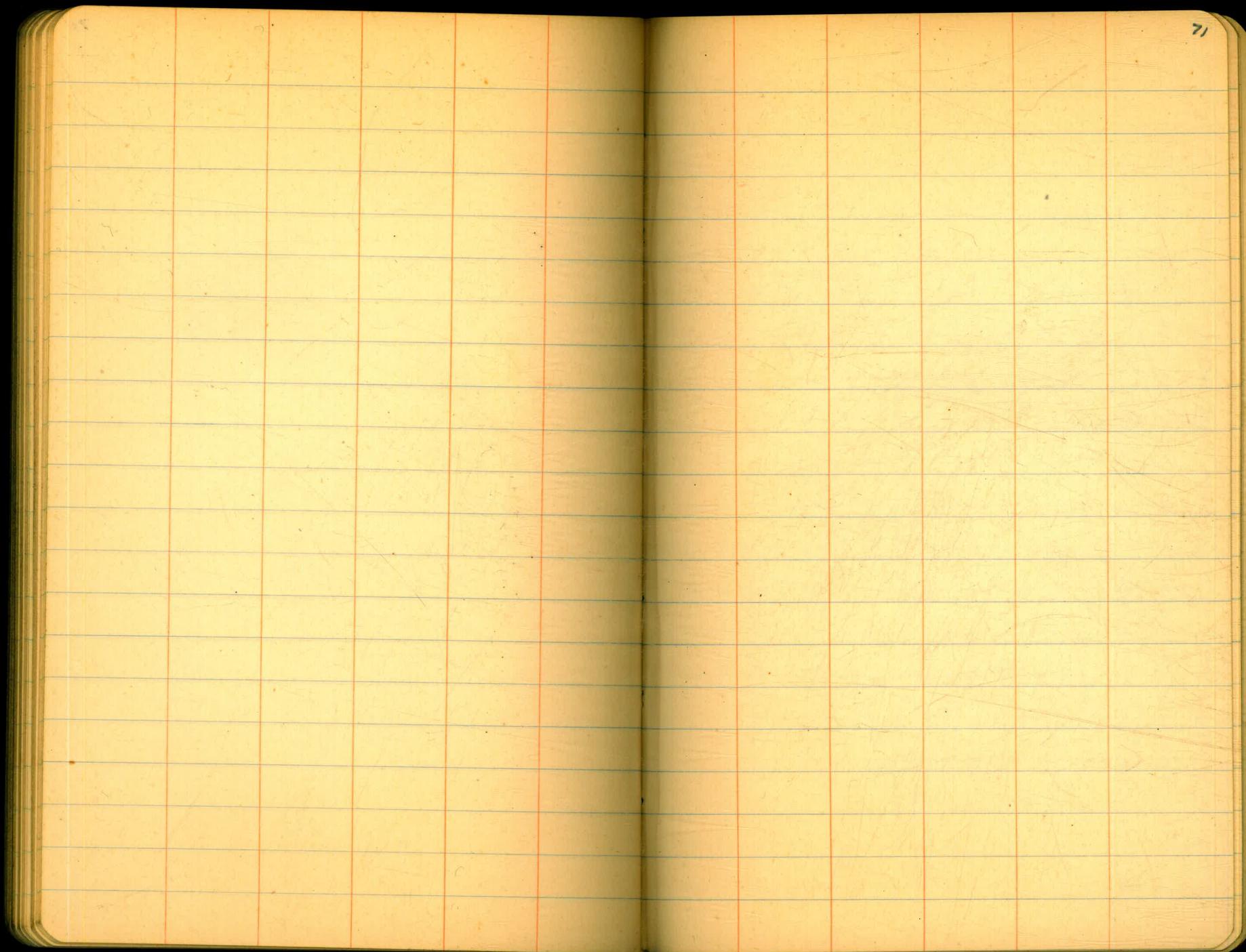


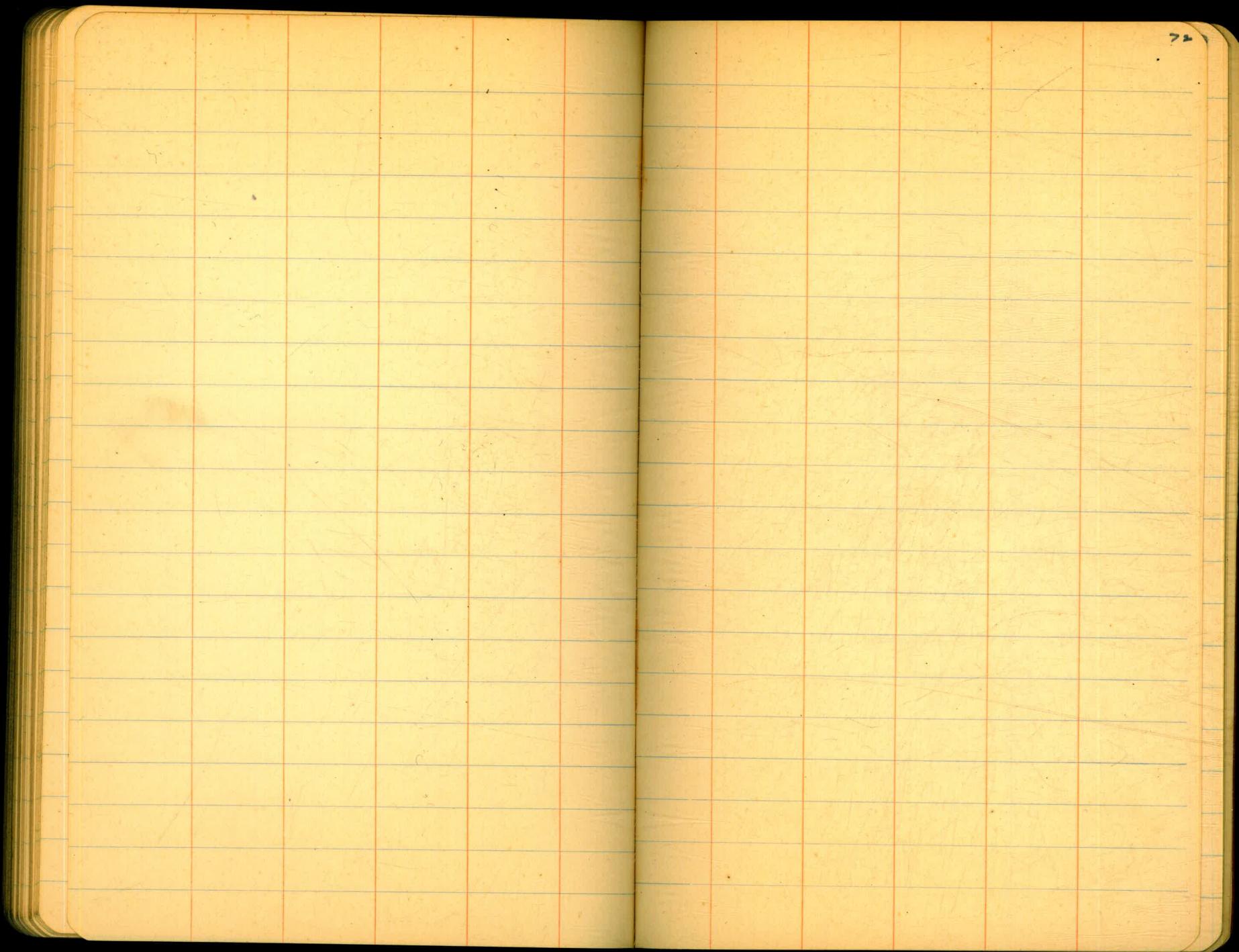


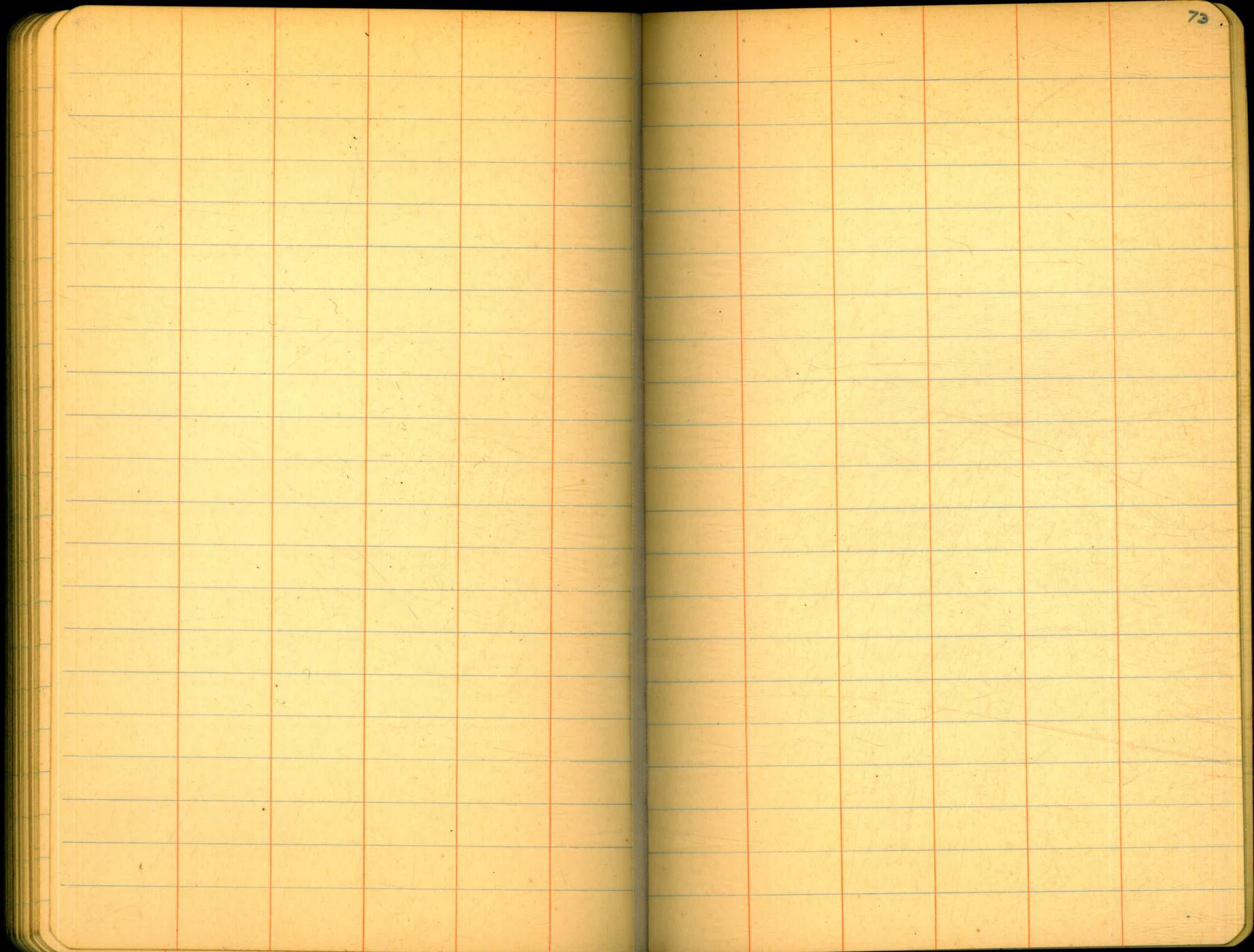


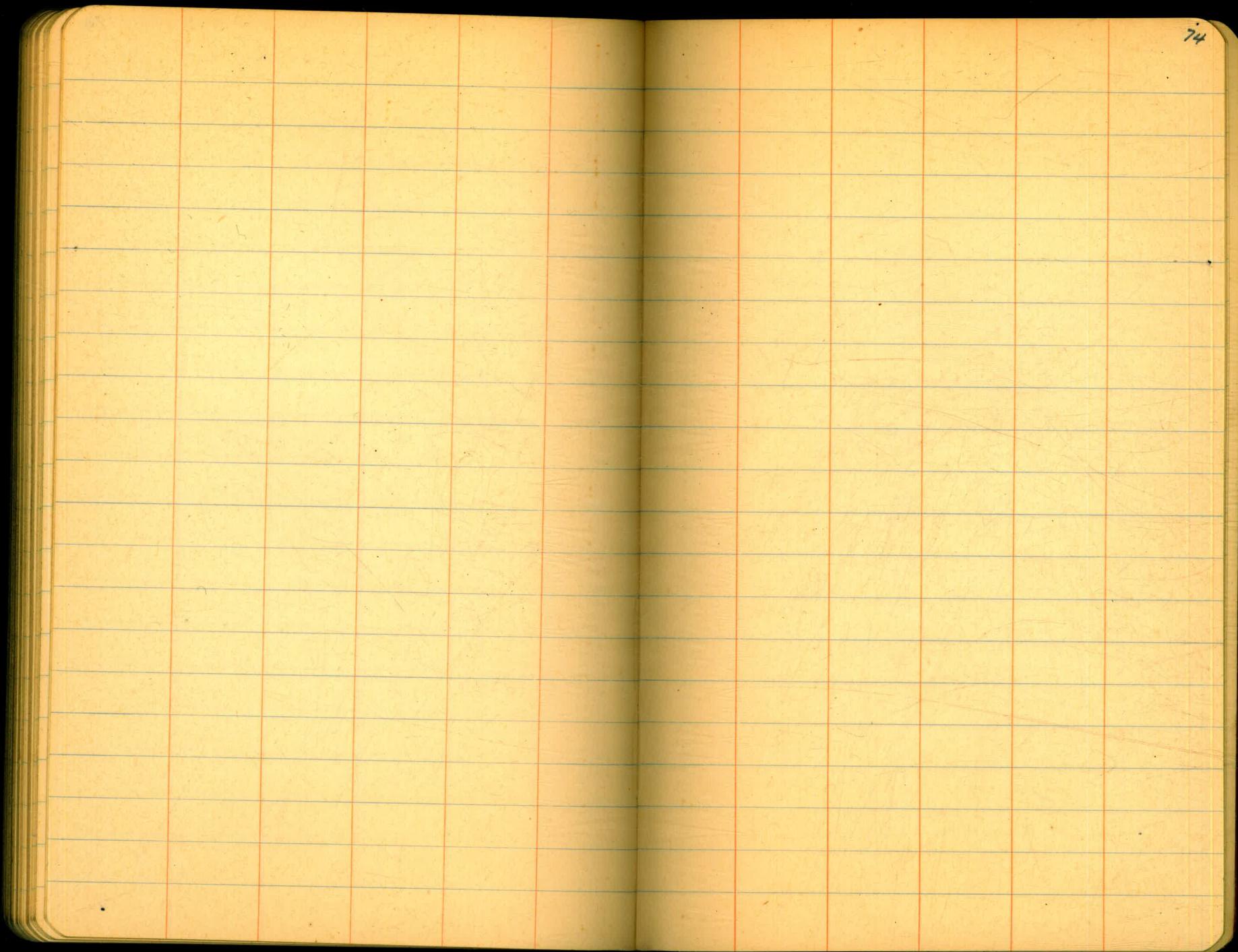


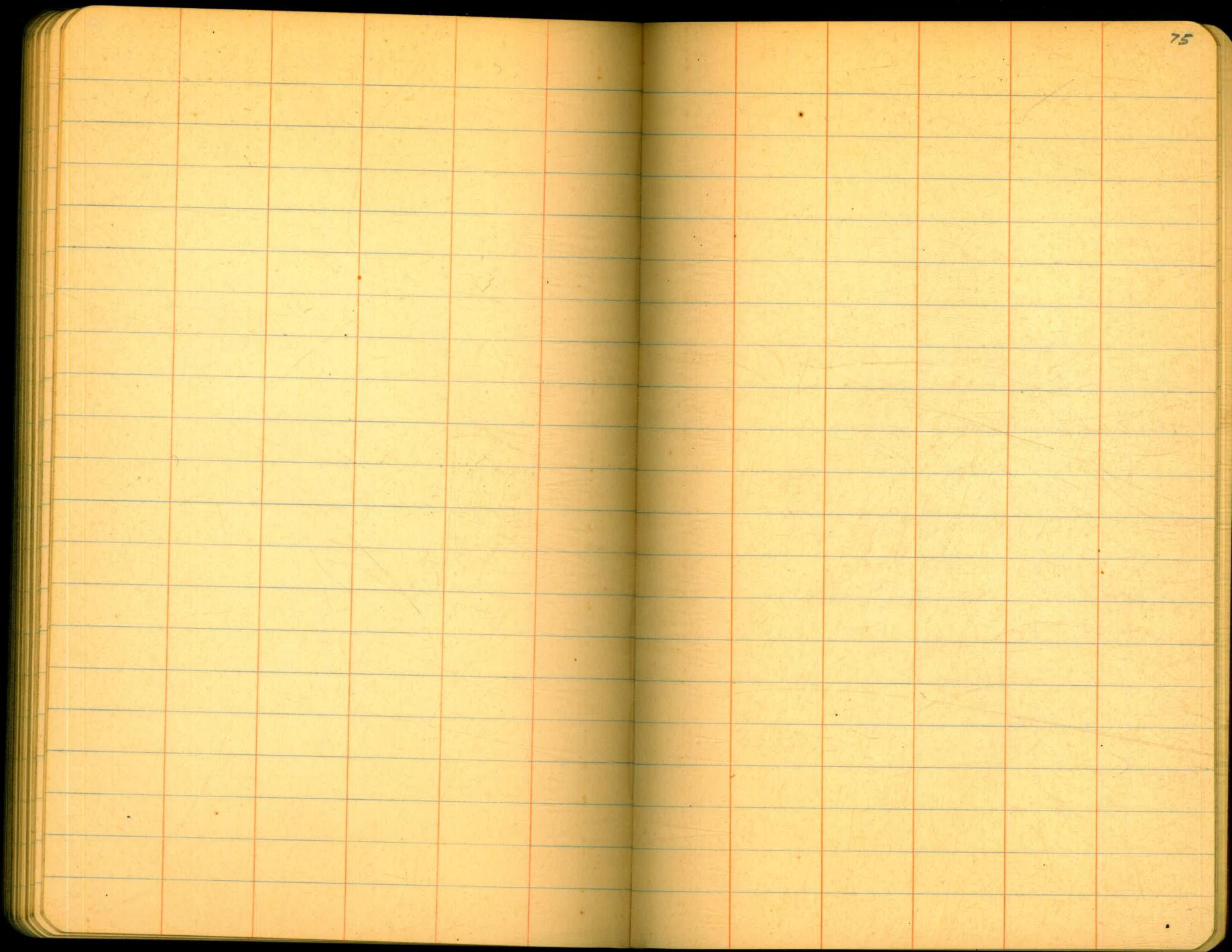


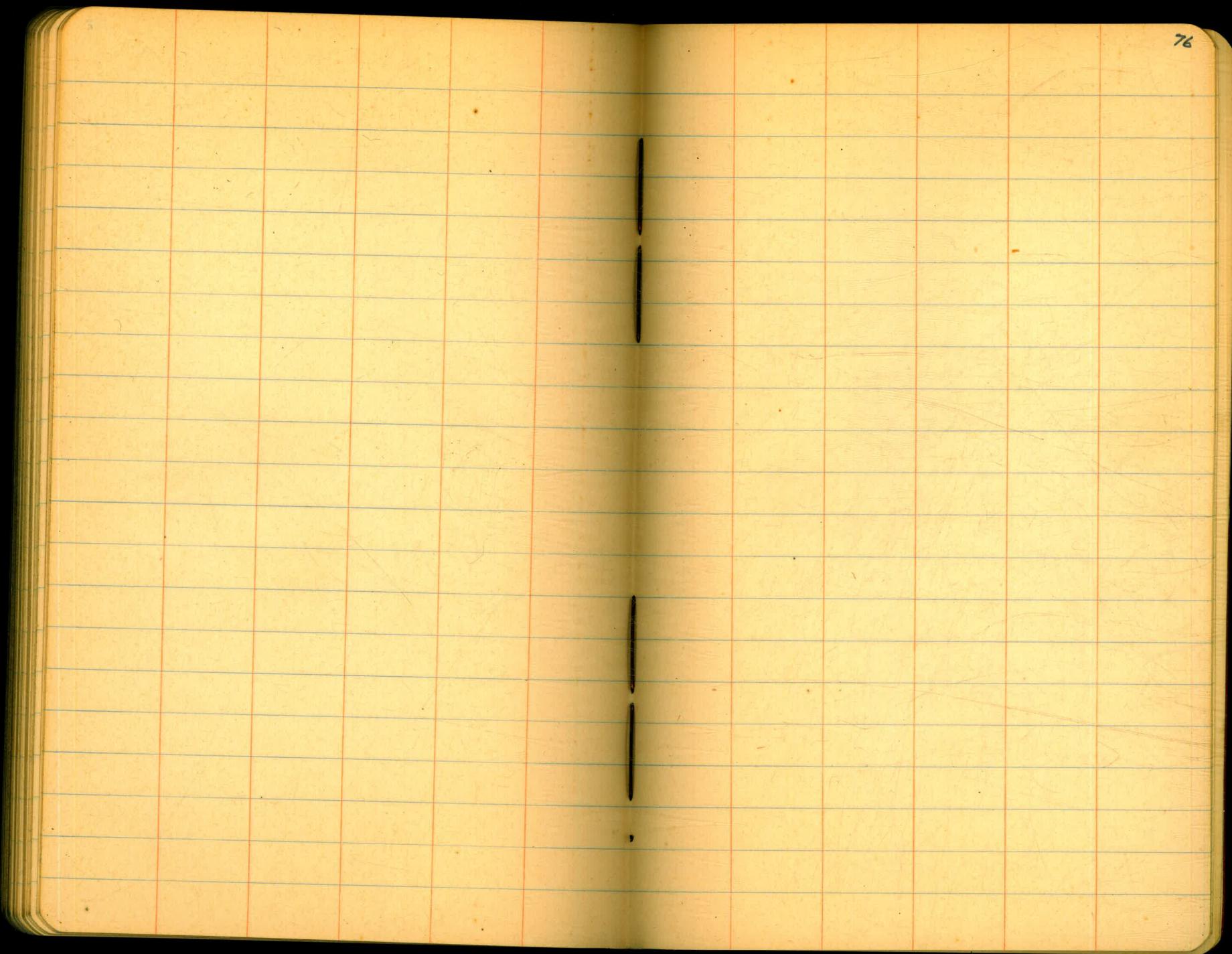


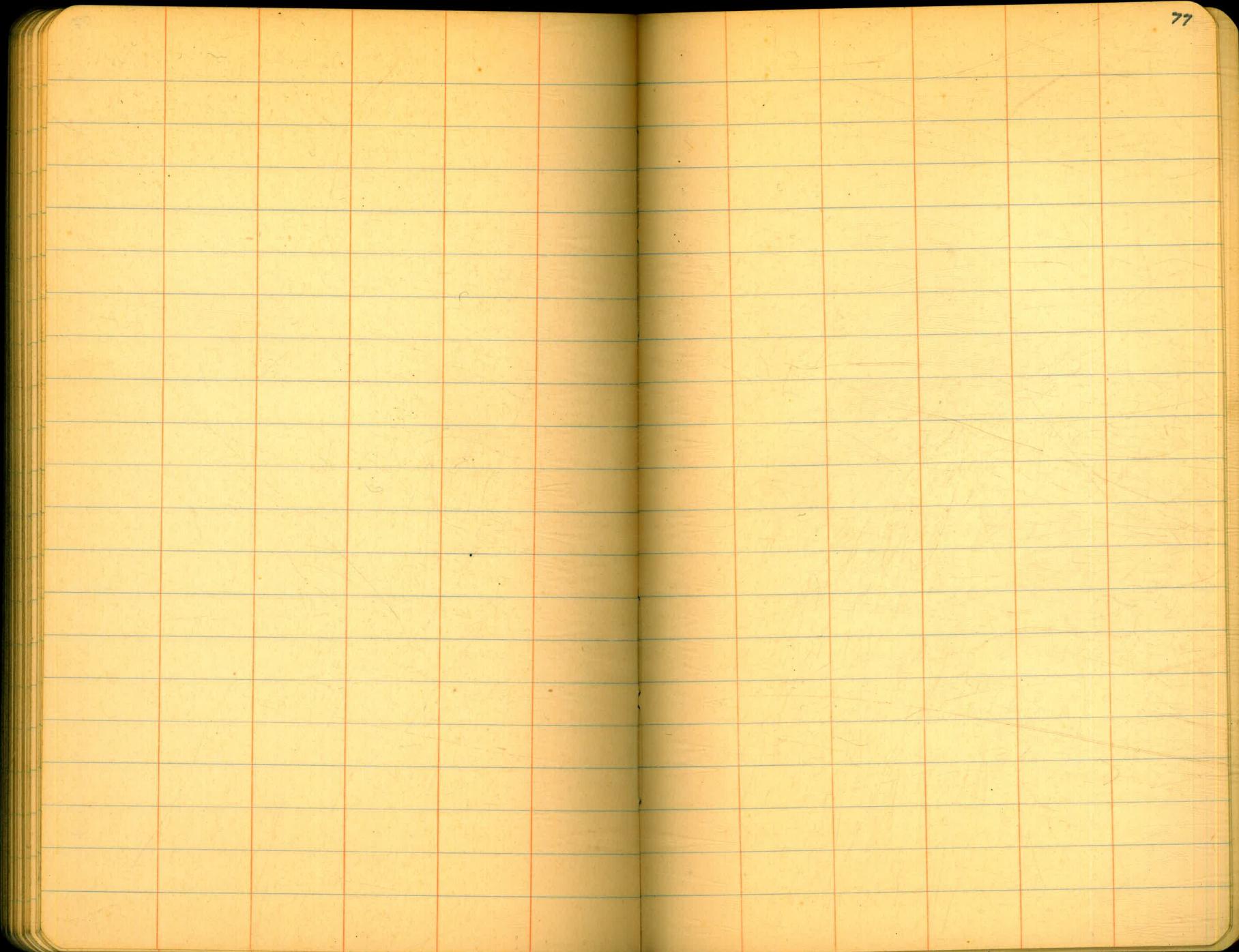














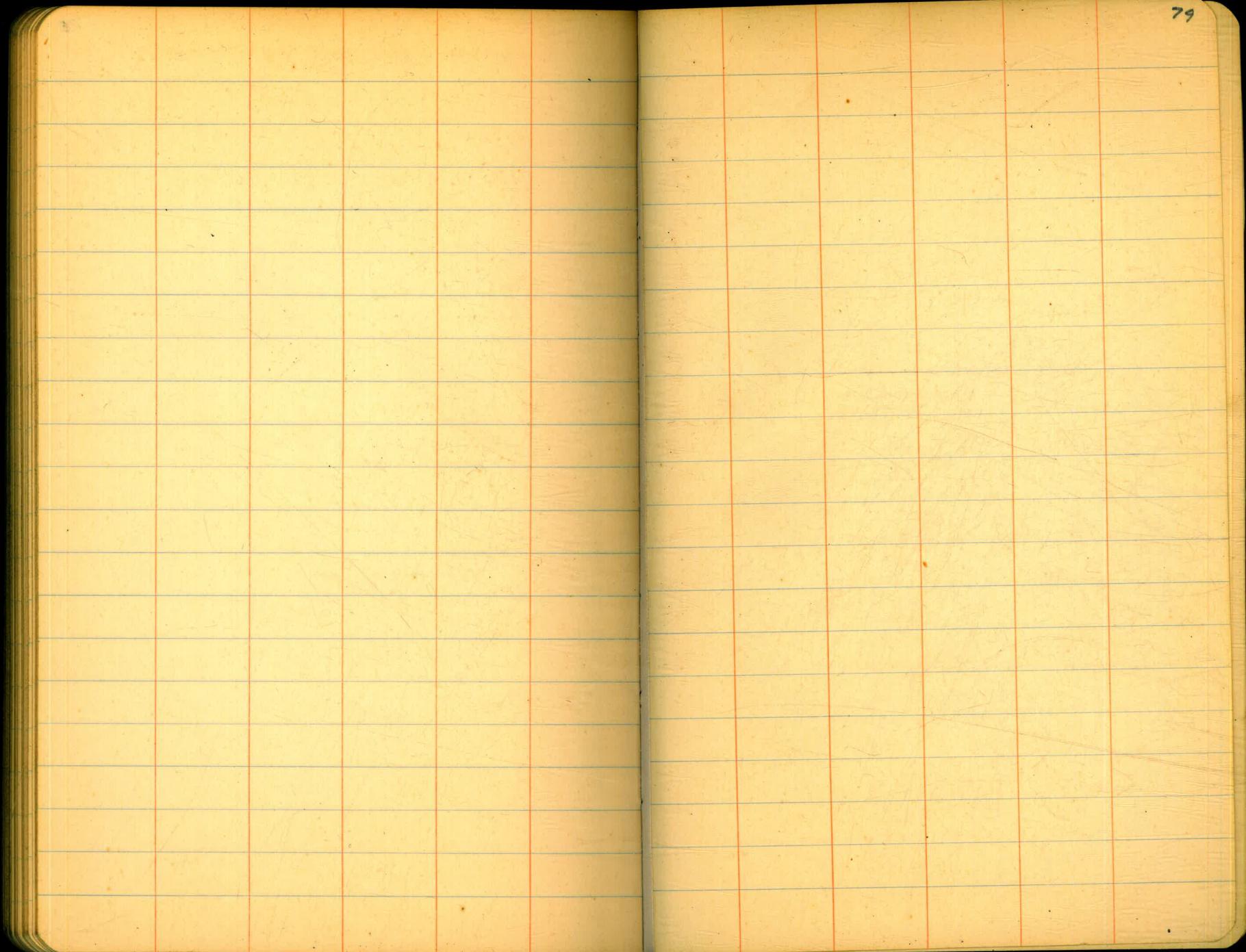


TABLE IX.—CALCULATION OF EARTHWORK.

Width	HEIGHT														
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.02	.04	.06	.07	.09	.11	.13	.15	.17	.18	.20	.22	.24	.26	.28
2	.04	.07	.11	.15	.18	.22	.26	.30	.33	.37	.41	.44	.48	.52	.56
3	.06	.11	.17	.22	.28	.33	.39	.44	.50	.56	.61	.67	.72	.78	.83
4	.07	.15	.22	.30	.37	.44	.52	.59	.67	.74	.81	.89	.96	1.04	1.11
5	.09	.19	.28	.37	.46	.56	.65	.74	.83	.93	1.02	1.11	1.20	1.30	1.39
6	.11	.22	.33	.44	.56	.67	.78	.89	1.00	1.11	1.22	1.33	1.44	1.55	1.67
7	.13	.26	.39	.52	.65	.78	.91	1.04	1.16	1.30	1.42	1.55	1.68	1.81	1.94
8	.15	.30	.44	.59	.74	.89	1.04	1.19	1.35	1.48	1.63	1.78	1.92	2.08	2.22
9	.17	.33	.50	.67	.88	1.00	1.17	1.33	1.50	1.67	1.83	2.00	2.17	2.33	2.50
10	.18	.37	.56	.74	.93	1.11	1.30	1.48	1.67	1.85	2.04	2.22	2.41	2.59	2.78
11	.20	.41	.61	.82	1.02	1.22	1.43	1.63	1.83	2.04	2.24	2.44	2.65	2.85	3.06
12	.22	.44	.67	.89	1.11	1.33	1.56	1.78	2.00	2.22	2.44	2.67	2.89	3.11	3.33
13	.24	.48	.72	.96	1.20	1.44	1.68	1.92	2.16	2.41	2.65	2.89	3.13	3.37	3.61
14	.26	.52	.78	1.04	1.30	1.55	1.81	2.08	2.32	2.58	2.85	3.11	3.37	3.63	3.89
15	.28	.56	.83	1.11	1.39	1.67	1.94	2.22	2.50	2.78	3.06	3.33	3.61	3.89	4.17
16	.30	.59	.89	1.18	1.48	1.78	2.07	2.37	2.67	2.96	3.26	3.56	3.85	4.15	4.44
17	.31	.63	.94	1.26	1.57	1.89	2.20	2.52	2.83	3.15	3.46	3.78	4.09	4.41	4.72
18	.33	.67	1.00	1.33	1.67	2.00	2.33	2.67	3.00	3.33	3.67	4.00	4.33	4.67	5.00
19	.35	.70	1.06	1.41	1.76	2.11	2.46	2.82	3.17	3.52	3.87	4.22	4.57	4.92	5.28
20	.37	.74	1.11	1.48	1.85	2.22	2.59	2.96	3.33	3.70	4.07	4.44	4.81	5.18	5.56
21	.39	.78	1.17	1.55	1.94	2.33	2.72	3.11	3.50	3.89	4.28	4.67	5.06	5.44	5.83
22	.41	.81	1.22	1.63	2.04	2.44	2.85	3.26	3.67	4.07	4.48	4.89	5.30	5.70	6.11
23	.43	.85	1.28	1.70	2.13	2.56	2.98	3.41	3.83	4.24	4.68	5.11	5.54	5.96	6.39
24	.44	.89	1.33	1.78	2.22	2.67	3.11	3.56	4.00	4.44	4.89	5.33	5.78	6.22	6.67
25	.46	.92	1.39	1.85	2.31	2.78	3.24	3.70	4.17	4.63	5.09	5.56	6.02	6.48	6.94
26	.48	.96	1.44	1.92	2.41	2.89	3.37	3.85	4.33	4.82	5.30	5.78	6.26	6.74	7.24
27	.50	1.00	1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50
28	.52	1.04	1.55	2.07	2.59	3.11	3.63	4.15	4.67	5.18	5.70	6.22	6.74	7.26	7.78
29	.54	1.07	1.61	2.15	2.68	3.22	3.76	4.30	4.83	5.37	5.91	6.44	6.98	7.52	8.06
30	.56	1.11	1.67	2.22	2.78	3.33	3.89	4.44	5.00	5.55	6.11	6.67	7.22	7.78	8.33
31	.57	1.15	1.72	2.30	2.87	3.44	4.02	4.59	5.17	5.74	6.32	6.89	7.46	8.04	8.61
32	.59	1.18	1.78	2.37	2.92	3.56	4.15	4.74	5.33	5.92	6.52	7.11	7.70	8.30	8.89
33	.61	1.22	1.83	2.44	3.05	3.67	4.28	4.89	5.50	6.11	6.72	7.33	7.94	8.55	9.17
34	.63	1.26	1.89	2.52	3.15	3.78	4.40	5.04	5.67	6.29	6.93	7.56	8.18	8.81	9.44
35	.65	1.30	1.94	2.59	3.24	3.89	4.53	5.18	5.83	6.48	7.13	7.78	8.42	9.08	9.72
36	.67	1.33	2.00	2.67	3.33	4.00	4.66	5.33	6.00	6.67	7.33	8.00	8.67	9.33	10.00
37	.68	1.37	2.06	2.74	3.42	4.11	4.79	5.48	6.17	6.85	7.54	8.22	8.91	9.59	10.28
38	.70	1.41	2.11	2.82	3.52	4.22	4.92	5.63	6.33	7.03	7.74	8.44	9.15	9.85	10.56
39	.72	1.44	2.17	2.89	3.61	4.33	5.05	5.78	6.50	7.22	7.95	8.67	9.39	10.11	10.83
40	.74	1.48	2.22	2.96	3.70	4.44	5.18	5.92	6.67	7.41	8.15	8.89	9.63	10.37	11.11

Table gives cu. yds. in 1 ft. of a triangle of given width and height. Corrections for tenths of width are one tenth the values found under each height considering the widths from 1 to 9 as tenths and similarly the corrections for tenths of height are one tenth the figures opposite width considering the heights from 1 to 9 as tenths. Thus if  $w=16.2$  and  $h=5.3$ , cu. yds. =  $1.48 + 0.028 + 0.089 = 1.597$  cu. yds. or practically 160 cu. yds. per 100 ft. If  $w$  exceeds 40 ft., use one half and multiply result by 2, if both  $w$  and  $h$  are large use one half of each and multiply result by 4. Any cross-section may be divided into triangles by the following rule. To the triangle of the sum of the outside cuts (or fills) =  $h$ , and  $\frac{1}{2}$  the roadbed =  $w$ , add the triangles formed by taking the distance out to each break in turn (=  $w$ 's) by the difference between the cuts (or fills) on each side of it (=  $h$ 's) always subtracting the outer from the inner.

### DISTANCES FROM CENTER OF ROADWAY FOR CROSS-SECTIONING.

Roadway 16 feet wide. Side Slopes 1 on  $1\frac{1}{2}$ .  
For Single Track Embankment.

H	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	II
0	8.0	8.2	8.3	8.5	8.6	8.8	8.9	9.1	9.2	9.4	0
1	9.5	9.7	9.8	10.0	10.1	10.3	10.4	10.6	10.7	10.9	1
2	11.0	11.2	11.3	11.5	11.6	11.8	11.9	12.1	12.2	12.4	2
3	12.5	12.7	12.8	13.0	13.1	13.3	13.4	13.6	13.7	13.9	3
4	14.0	14.2	14.3	14.5	14.6	14.8	14.9	15.1	15.2	15.4	4
5	15.5	15.7	15.8	16.0	16.1	16.3	16.4	16.6	16.7	16.9	5
6	17.0	17.2	17.3	17.5	17.6	17.8	17.9	18.1	18.2	18.4	6
7	18.5	18.7	18.8	19.0	19.1	19.3	19.4	19.6	19.7	19.9	7
8	20.0	20.2	20.3	20.5	20.6	20.8	20.9	21.1	21.2	21.4	8
9	21.5	21.7	21.8	22.0	22.1	22.3	22.4	22.6	22.7	22.9	9
10	23.0	23.2	23.3	23.5	23.6	23.8	23.9	24.1	24.2	24.4	10
11	24.5	24.7	24.8	25.0	25.1	25.3	25.4	25.6	25.7	25.9	11
12	26.0	26.2	26.3	26.5	26.6	26.8	26.9	27.1	27.2	27.4	12
13	27.5	27.7	27.8	28.0	28.1	28.3	28.4	28.6	28.7	28.9	13
14	29.0	29.2	29.3	29.5	29.6	29.8	29.9	30.1	30.2	30.4	14
15	30.5	30.7	30.8	31.0	31.1	31.3	31.4	31.6	31.7	31.9	15
16	32.0	32.2	32.3	32.5	32.6	32.8	32.9	33.1	33.2	33.4	16
17	33.5	33.7	33.8	34.0	34.1	34.3	34.4	34.6	34.7	34.9	17
18	35.0	35.2	35.3	35.5	35.6	35.8	35.9	36.1	36.2	36.4	18
19	36.5	36.7	36.8	37.0	37.1	37.3	37.4	37.6	37.7	37.9	19
20	38.0	38.2	38.3	38.5	38.6	38.8	38.9	39.1	39.2	39.4	20
21	39.5	39.7	39.8	40.0	40.1	40.3	40.4	40.6	40.7	40.9	21
22	41.0	41.2	41.3	41.5	41.6	41.8	41.9	42.1	42.2	42.4	22
23	42.5	42.7	42.8	43.0	43.1	43.3	43.4	43.6	43.7	43.9	23
24	44.0	44.2	44.3	44.5	44.6	44.8	44.9	45.1	45.2	45.4	24
25	45.5	45.7	45.8	46.0	46.1	46.3	46.4	46.6	46.7	46.9	25
26	47.0	47.2	47.3	47.5	47.6	47.8	47.9	48.1	48.2	48.4	26
27	48.5	48.7	48.8	49.0	49.1	49.3	49.4	49.6	49.7	49.9	27
28	50.0	50.2	50.3	50.5	50.6	50.8	50.9	51.1	51.2	51.4	28
29	51.5	51.7	51.8	52.0	52.1	52.3	52.4	52.6	52.7	52.9	29
30	53.0	53.2	53.3	53.5	53.6	53.8	53.9	54.1	54.2	54.4	30
31	54.5	54.7	54.8	55.0	55.1	55.3	55.4	55.6	55.7	55.9	31
32	56.0	56.2	56.3	56.5	56.6	56.8	56.9	57.1	57.2	57.4	32
33	57.5	57.7	57.8	58.0	58.1	58.3	58.4	58.6	58.7	58.9	33
34	59.0	59.2	59.3	59.5	59.6	59.8	59.9	60.1	60.2	60.4	34
35	60.5	60.7	60.8	61.0	61.1	61.3	61.4	61.6	61.7	61.9	35
36	62.0	62.2	62.3	62.5	62.6	62.8	62.9	63.1	63.2	63.4	36
37	63.5	63.7	63.8	64.0	64.1	64.3	64.4	64.6	64.7	64.9	37
38	65.0	65.2	65.3	65.5	65.6	65.8	65.9	66.1	66.2	66.4	38
39	66.5	66.7	66.8	67.0	67.1	67.3	67.4	67.6	67.7	67.9	39
40	68.0	68.2	68.3	68.5	68.6	68					